



SITE SELECTION STUDY FOR
THE PROPOSED NEW TRANSMISSION SUBSTATION
NEAR MAKOPANE IN THE NORTHERN REGION

GEOTECHNICAL REPORT

*PRELIMINARY GEOTECHNICAL INVESTIGATION
FOR THE SELECTION OF A SITE FOR THE
PROPOSED NEW BURUTHO 400/132kV SUBSTATION*

May 2010



GEOTECHNICAL INVESTIGATIONS
TRANSMISSION TECHNOLOGY

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TITLE :

GEOTECHNICAL REPORT

PRELIMINARY GEOTECHNICAL INVESTIGATION
FOR THE SELECTION OF A SITE FOR THE
PROPOSED NEW BURUTHO 400/132kV SUBSTATION

PREPARED BY : *Specialist Consultant Geotechnical Investigations
Transmission Technology*

CLIENT : *Land and Rights
Transmission*

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Soillab*

REPORT NO : *GR03-08*

DATE : *May 2010*

Approved :

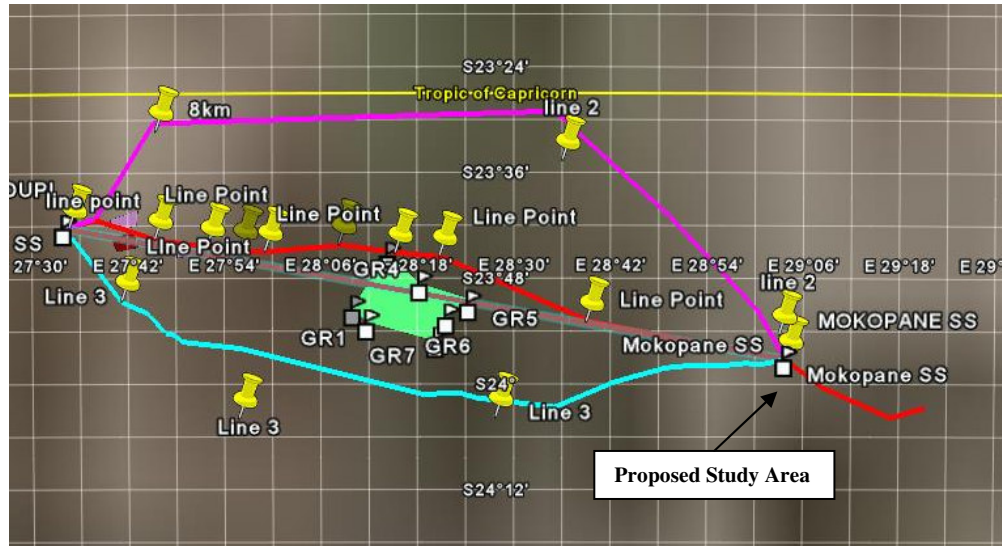
F A Grové

**Senior Specialist Consultant
Geotechnical Investigations**

1.0 INTRODUCTION

Eskom plans to construct a new 400/132 kV Substation near Makopane in the Northern Region.

It was requested by Transmission System Planning to carry out a site selection study for the proposed new Burutho, 400/132kV Substation, near Makopane in the Northern Region.



PROPOSED STUDY AREA FOR THE NEW BURUTHO 400/132kV SUBSTATION
(Previously referred to Makopane Substation)

Figure-I

A Site for the planned development is required in the area near Makopane. The **first phase** of this investigation included extensive desk studies, where a total number of three most suitable study areas were identified for the substation. For the selection of these study areas during the desk studies, factors such as the geology, topography, farming activities, mining activities, Eskom installations and power line servitude routes were considered.

Information collected during this investigation is suitable for site selection purposes, and once the final design is required, a Detailed Geotechnical Investigation will be required to provide design parameters and confirm findings of this investigation..

2.0 THE NATURE OF THE PROJECT

The project comprises the construction of a new Substation in the Mashashane area, approximately 35km to the north of Mokopane town in the Limpopo Province.

The development would include the installation of typical equipment such as:

- Electrical transformers
- Circuit breakers or line termination structures
- High-voltage switchgear
- Low voltage switchgear
- Surge & lightning protection equipment
- Control and metering equipment
- Access roads and buildings

3.0 OBJECTIVES OF THE INVESTIGATION

The objectives of this investigation were defined as follows:

- Conduct intensive desk studies of the area to identify a total number of three most suitable sites for the proposed new substation
- Obtain geotechnical information during a Preliminary Geotechnical Investigation of selected sites to confirm findings of the desk study for suitability evaluation of sites

4.0 DESK STUDY

4.1 Maps Consulted

An attempt was made to collect as much information as possible of the study area demarcated by Transmission System Planning, for the proposed new substation during the desk study. For this purpose the following maps were consulted:

- i) Topographical Maps – 2328DD LIMBURG; 2329CC MASHASANE
- ii) Ortho Photos – Covering the area
- iii) Geological Map – Geological Map 2328 Pietersburg
- iv) Mining - Mineral Map of the Bushveld Complex – South Africa
With Special reference to Platinum and Chrome Simplified Geology,
Selected Mines and Mineral Deposits – South Africa, Lesotho and
Swaziland

4.2 Aerial Photographic Interpretation

In principle the following features are being studied, information being obtained and interpreted when studying aerial photographs for geotechnical purposes:

- Reflection of the action of nature in creating the existing conditions
- Grouping of materials according to certain patterns
- Definition of various boundaries and linear features of significance
- Field checking by visual inspection

Stereo-interpretation has a great advantage over interpretation of a single photograph, because it is better able to identify topographical and erosion features, grey tones, and textures have greater requisite clarity contrasts. Basically, two aspects of the air photo image are revealed in the stereo-model of a given area, and these are surface form and grey tone, which could be subdivided as:

- a) Elements of Surface Form
 - Topographic form
 - Drainage form
 - Erosion form
- b) Elements of Grey tone and Texture of :
 - vegetation
 - due to land use
 - soil and rock material

Geotechnical information obtained in this way was correlated with Geological Map data.

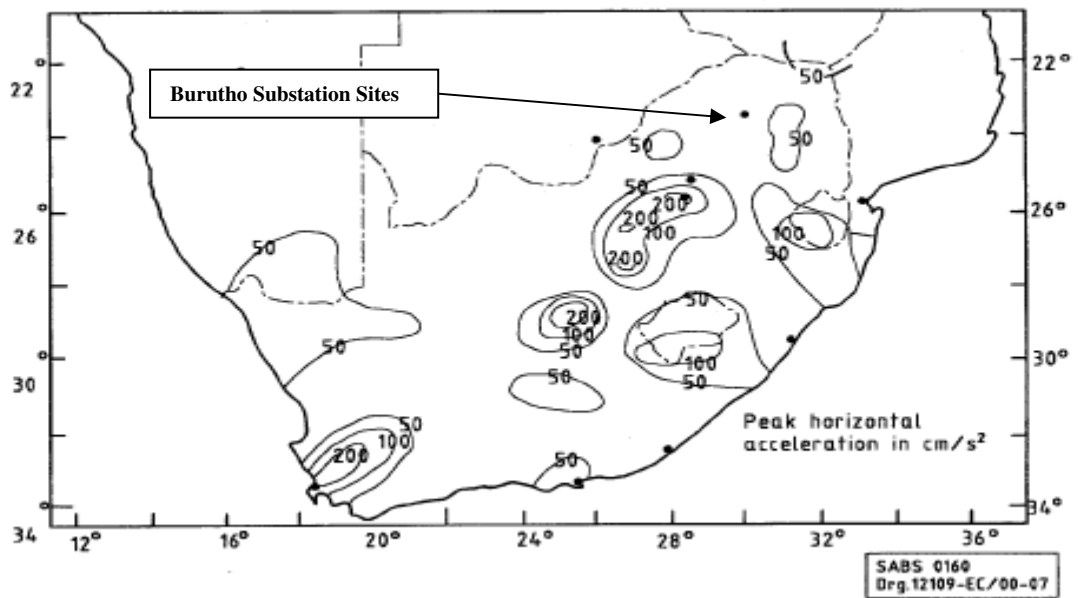
4.3 Review of Desk Study and Selection of Sites

The size of the substation platform is 338m x 354,2m. For the purpose of flexibility site areas selected are of the order of 700mx700m. The region is known for its platinum and chrome mining activities. It has been confirmed by the EIA studies that

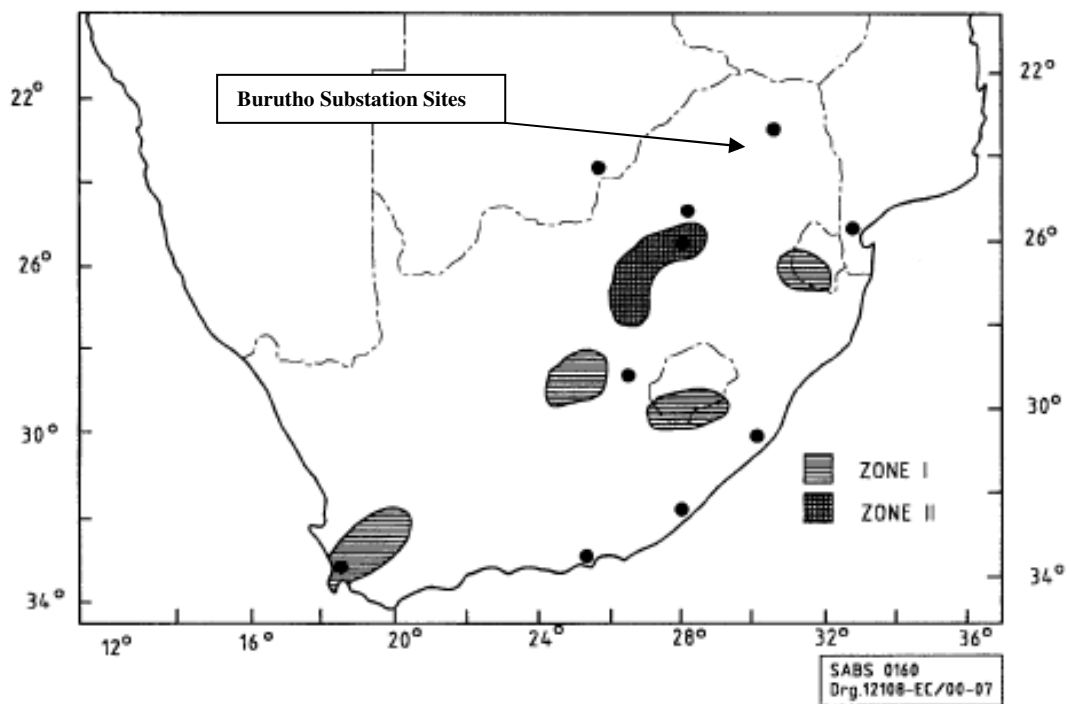
the sites selected for this study are not located in future mining fields. It should be noticed that the load centre area is limited in site options due to extreme topographical constraints.

4.4 Seismic Zoning

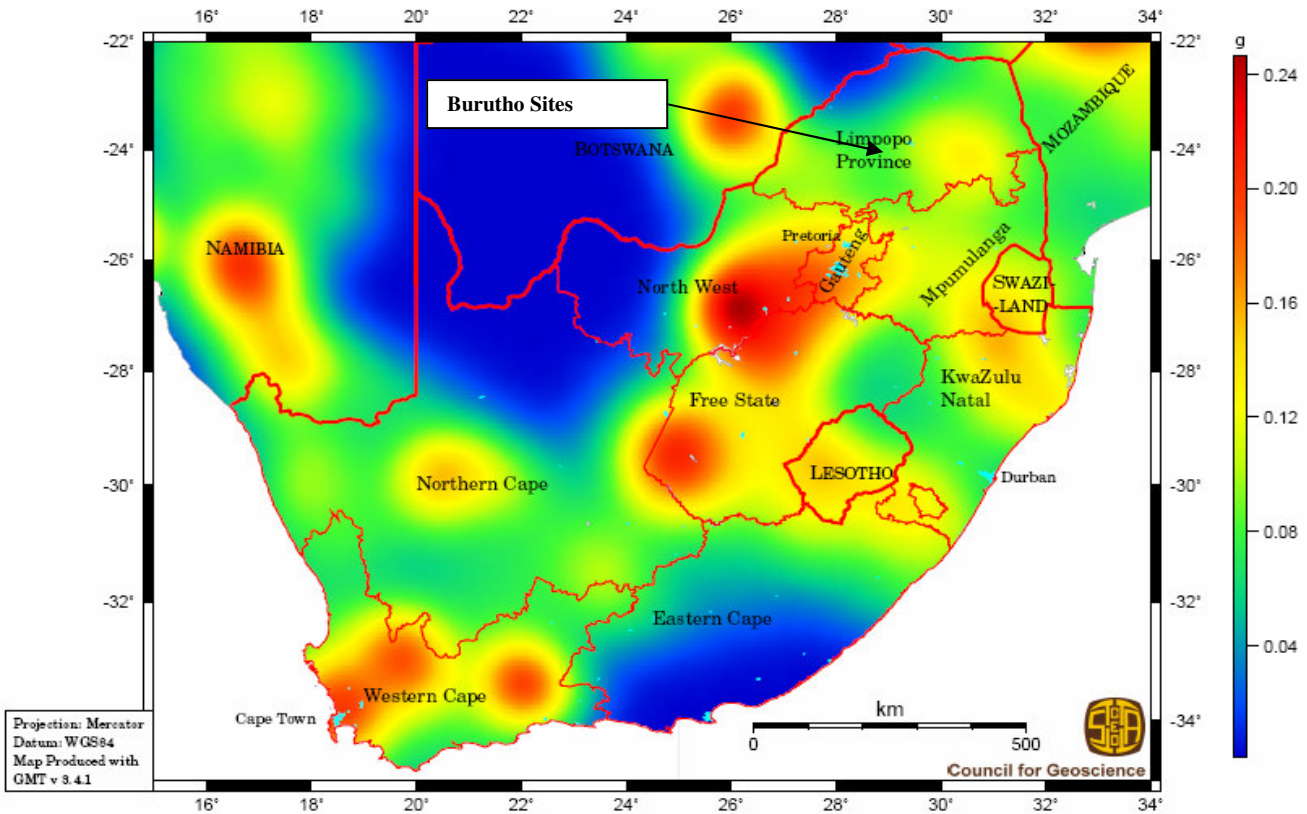
The South African loading code, SANS 10160 – 1969 (Figure-IIa and IIb) , shows that the proposed sites for this study are situated in an area where the peak ground acceleration with a 10% probability of being exceeded in a 50 year period is between 50cm/sec² and 100cm/sec². Figure-IIb also show the zones where detailed seismic design (Zone-I) and compliance with minimum requirements (Zone-II) are specified by the code. The proposed Burutho sites fall outside of these Zones.



SEISMIC HAZARD MAP OF SA *Figure-IIa*



SEISMIC HAZARD ZONES OF SA *Figure-IIb*



4.1 MAP 1: SEISMIC HAZARD MAP OF SOUTH AFRICA
 (INCLUDING THE KINGDOMS OF LESOTHO AND SWAZILAND)
 Peak ground acceleration (g) with a 10% probability of being exceeded in a 50 year period

Figure-III

More recent data produced by the Council of Geoscience place the sites within the zone where the minimum seismic event, with a 10% probability of being exceeded in a 50 year period, falls in the range of 0,07g to 0,10g (Figure-III). This data however, still needs to be verified.

5.0 LOCALITY OF SITES INITIALLY SELECTED

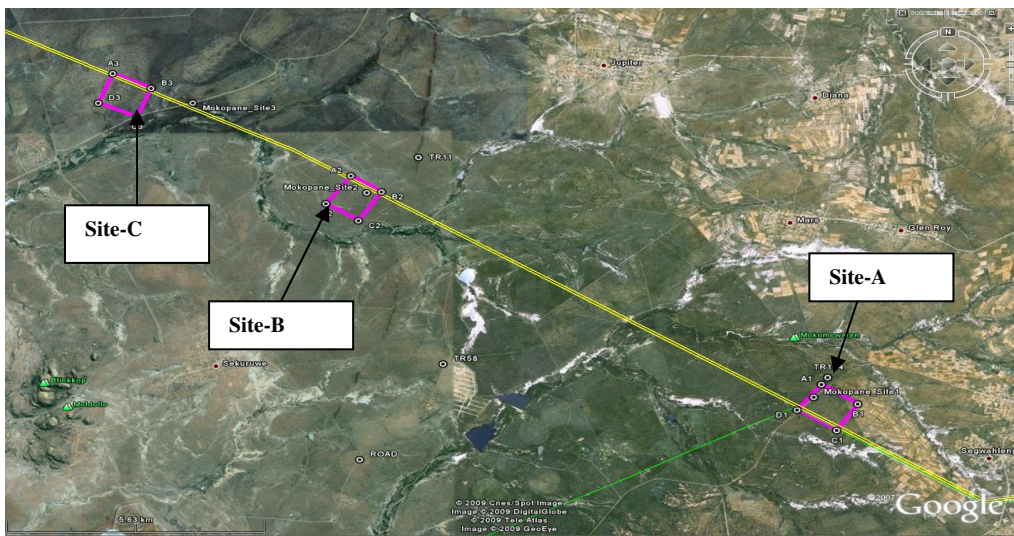


Figure-IV

It should be noted that options for suitable sites are limited in the Study Area due to extreme topographical constraints, which entails hillocks, water ways, perennial and non-perennial water streams and no road infrastructure.

For this reason, considering costs to develop **Site-A**, **Site-A** was rejected at an early stage due to extreme access and topographical difficulties and related costs. It was obvious to have geotechnical studies carried out for only Site-B and Site-C.

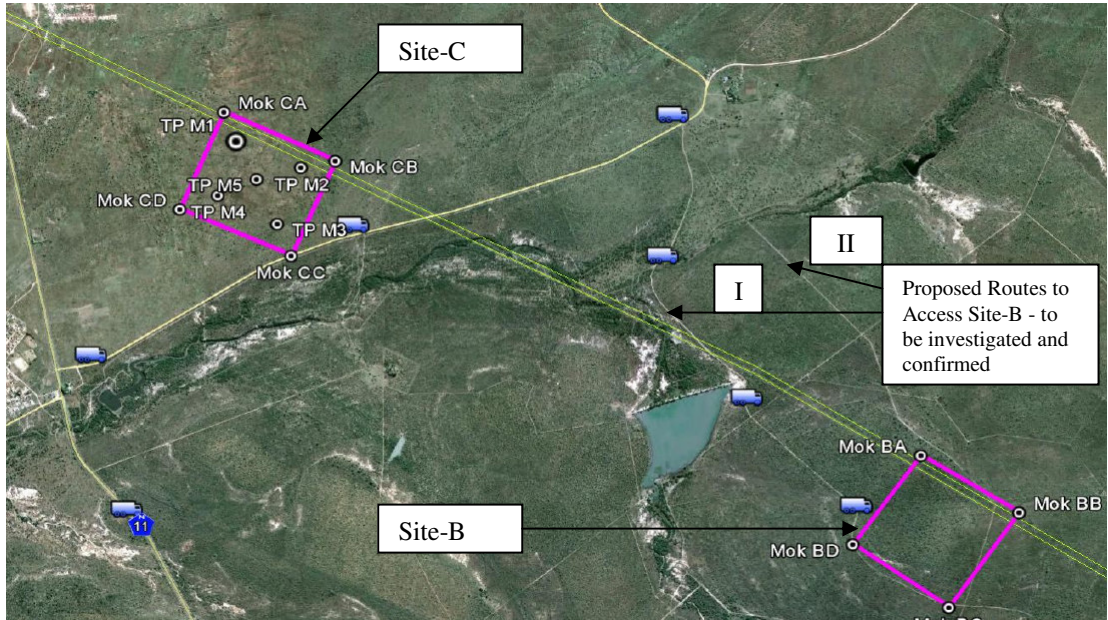


Figure-V

**POSITIONING OF SITE-B AND SITE-C
SHOWING ACCESS TO SITES FROM THE N11**

Site-C can easily be accessed from the N11 as indicated in *Figure-V*. To access **Site-B** will require a further investigation to obtain approval for road servitude options as indicated (**Option-I and Option-II**). Both these route options should be topographically surveyed and be evaluated in terms of flood lines and construction costs.

The identification and evaluation of the preferred access route does not form part of this preliminary Geotechnical Investigation.

Site-B and Site-C are located on the farms Noord Braband 774 LR and Zuid Holland 773 LR respectively. Site coordinates are:

Site-B S 23° 53' 42" E 28° 58' 30.4 "
 Site-C S 23° 52' 21.8" E 28° 55' 34 "

6.0 REGIONAL ENVIRONMENT

6.1 Geography

6.1.1 Topography and Drainage

Waving topography with the presence of hillocks, hills and water ways forms features of the region. The two sites selected for final evaluation purposes are located within a 6 km radius. **Site-B** is located on a high, well drained and has no risk of flooding. **Site-C** is partly located within the 1:50 year flood line, and subject to a risk of being flooded. Access to Site-B is problematic since a road servitude needs to be located without a risk of flooding. This access road will cross a non-perennial water way which will require careful design.

The **slope** traversing **Site-B** in a north to south direction away from the hillock is $\pm 2,5\%$ and **Site-C** is located on a gentle, south western slope, of $\pm 2,0\%$.

6.1.2 Flood –line

Flood line studies carried out indicate Site-C to have a risk of being flooded with a 1:50 year flood. Site-B has no risk of flooding. (See Appendix-A)

6.1.3 Climate

N-Value

The "Weinert N-Value", that describes the climatic environment, is approximately 4 for the area. Where "N" is more than "5", disintegration is the prominent form of weathering, and where "N" is less than "5", decomposition affects those rocks whose minerals are liable to change chemically under atmospheric conditions.

Rainfall

The average annual rainfall of the study areas is 505.3mm

Vegetation

Both sites are covered with typical indigenous bush of the area, re grass, bush and trees.

6.2 Geology

The regional geology comprises Gneiss, Migmatite and Leucogranite. The solid geology of the sites investigated is masked by transported silty sands at shallow depths. The solid geology is considered decomposed to highly weathered at shallow depths, Gneiss. (Geological Map 2328 Pietersburg)

6.3 Water Table

No evidence of a shallow water table was observed on any of the sites.

7.0 THE INVESTIGATION

7.1 Topographical Survey

Topographical surveys of sites were carried out to enable preliminary geometric designs. This forms an important part of the geotechnical evaluation of sites, since ground elevations will be altered, due to the cut to fill operations during construction of the platform.

7.2 Geometric Design

Preliminary geometric designs were carried out for all three sites, at positions considering the optimisation of the topography and existing and future power line routes. These designs expose valuable information in terms of construction costs.

7.3 Exploratory Work

Test pits were excavated randomly to maximum reach or refusal, to confirm findings of the desk study, in terms of geotechnical properties.

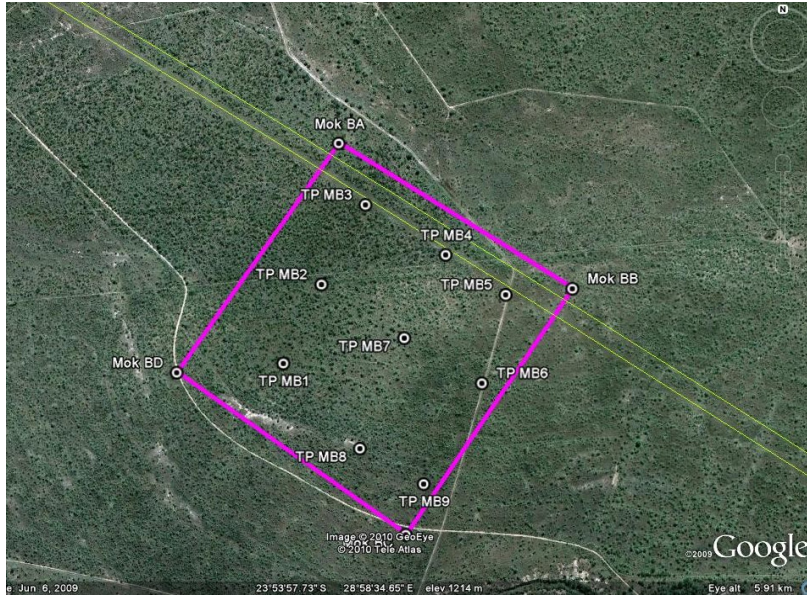


Figure-VI

SITE-B TEST PIT POSITIONING

The average soil profile was found uniformly present over the sites, with discrepancies regarding the material occurrences and physical properties with depth.

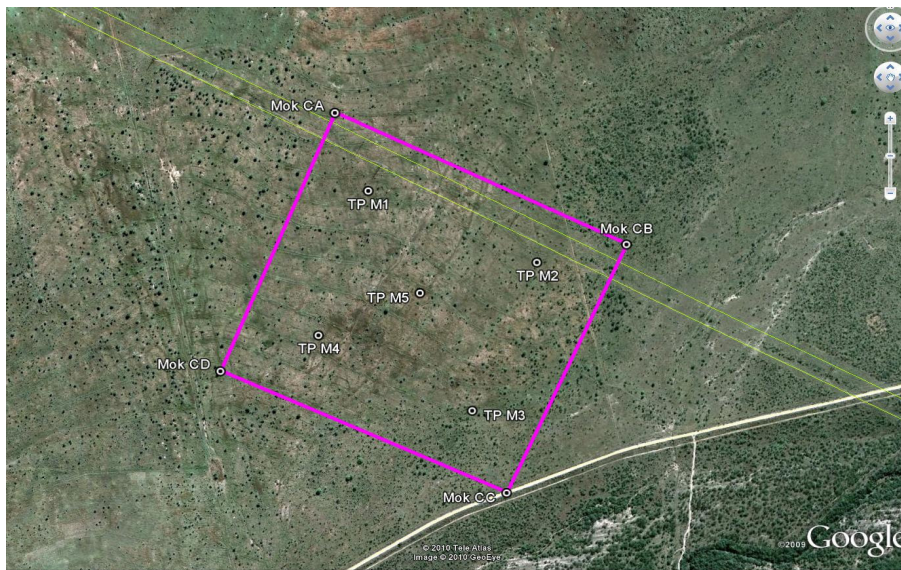


Figure-VII

SITE-C TEST PIT POSITIONING

The exception was that the profile of Site-B was found less weathered with depth than Site-C, with rock outcropping in places.

7.4 Laboratory Testing

Laboratory tests on soil samples were primarily conducted to determine Geotechnical properties of the soil, confirming findings during profiling.

The following tests were carried out:

- Grading (Mechanical and Hydrometer)
- Atterberg Limits

- Moisture Content
- Electrical Conductivity

8.0 GEOTECHNICAL EVALUATION

8.1 Soil Profile

The soil profiles of both sites investigated (**Site-B & Site-C**) are similar, with shallowly weathered rock near surface. However the difference in topography will have a remarkable influence on the cost of the construction of the substation platform on each site.

TABLE - I GEOTECHNICAL PROPERTIES

PROJECT: BURUTHO SUBSTATION - SITE SELECTION

Page No. _____
_____ Pages

TP BH NO	DEPTH (m)	ATTERBERG LIMITS			GM	PI OF WS	-2 µm %	PE	INSITU PROPERTIES							SOIL CHEMISTRY			COMPACTION					SOIL CLASSIFCT							
		LL	PI	LS					Soil Prof Hor izon	-% -4S	MC	Gra- vel	Sand	Silt	Clay	Per meab ility cm/s	pH	Con- ducti- vity S/m	Lange- lier Index	Insitu BDD	MDD	CBR % Mod AASHTO				Pra	U				
																						90	93	95	100						
TP-M2	0-0.2	20	6	3	0.96	<4	8L	A			11	53	29	5				0.0456												A4(3)	C
TP-M2	0.8-1.3	20	6	3	1.8	<4	1L	C			26	59	14	1				0.0275												A1-b(0)	S
TP-M3	1.1-0.40	19	8	4	1.96	<5	4L	A			57	23	17	4				0.0322												A2-4(0)	C
TP-M3	0.4-1.40	SP	SP	0.5	2.08	NP	4L	B			55	31	14	0				0.668												A1-b(0)	S
TP-MB4	0-0.3	NP	NP	0	1.1	NP	0L	A			12	57	7	1				0.0208												A4(3)	S
TP-MB4	0.3-0.8	SP	SP	0.5	1.1	SP	0L	B			77	20	3	0				0.019												A1-a(0)	C
TP-MB4	0.8-1.1	NP	NP	0	2.19	NP	0L	B			56	37	7	0				0.0187												A1-a(0)	S

LEGEND

TP - Test Pit
BH - Borehole
LL - Liquid Limit
PI - Plastic Index
PIWS - PI of Whole Sample

e - Void Ratio
LS - Linear Shrinkage
GM - Grading Modul
µm - Clay Fraction
PE - Potential Expansiveness
MDD - Maximum Dry Density kg/m³
W - Weston (Heave Prediction)

T - TRANSPORTED MATERIAL
P-1 ALLUVIUM (First Horizon)
P-2 ALLUVIUM (Second Horizon)

Laboratory testing confirmed geotechnical properties and findings during profiling of test pits.

8.2 Geology Influences on Construction

It is believed that the combination of ground conditions and topography of **Site-B and C** will have a **great affect** on conventional construction methods. Both sites are shallowly underlain by **SOFT ROCK**. However, the steeper sloping of the **topography of Site-B** will have a vast influence on the difference in construction costs of the platform. In addition the construction of an access road of at least 5km is required to access Site-B.

8.2 Water Table

No evidence of a shallow **perched water table** was found during the field investigation.

8.3 Topography - Preliminary Substation Positioning and Geometry (Earthworks Volumes)

An attempt was made to optimise the positioning of the substation platform on both sites, considering topography and power line locality (*See Appendix-D*)

Topographical features and present soil information of each site were taken into account during this exercise. The following volumes for earthworks were obtained during this exercise for each site:

<u>Site</u>	<u>Strip m³</u>	<u>Cut m³</u>	<u>Fill m³</u>	<u>Special Measures</u>
Site-A	15 570	168 370	151 135	Construction of Access Road ± 10km
Site-B	13 655	42635	33 305	Borrow material required to Construct the Substation Platform + 5km Access Road
Site-C	13 165	14 555	11 455	None – Access Road 600m

From the preliminary quantities it is clear that **Site-C will be the most cost effective** site to develop, ignoring the risk of flooding and the costs to develop **Site-A** is abnormally high. Considering all costs to develop sites and risks involved **Site-B** would be the most suitable site to develop.

Attached to this report are preliminary geometric designs, indicating optimised platform positioning for each site (**Appendix-D**). Combining the geometry of the topography with the ground conditions of each site (**only Site-B & Site-C**) it is clear that a vast quantity of rock will be excavated from **Site-B**, during a cut to fill operation. This will result in the need to import borrow material to replace unsuitable rock from cut. It is estimated with information at hand that a minimum of 15 000m³ of rock from cut will be unsuitable for the construction of the fill.

8.4 Flood Line Studies

Flood line studies have revealed that Site-A and Site-B have no risk of flooding but Site-C has an apparent risk of being flooded during a 1:50 year flood.

9.0 CONCLUSIONS AND RECOMMENDATIONS

Considering the results of all the studies carried out during this investigation it is clear that Site-C is the most cost effective site to develop, not considering the risk of flooding. Though Site-B will be more costly site to develop it has no risk of being flooded.

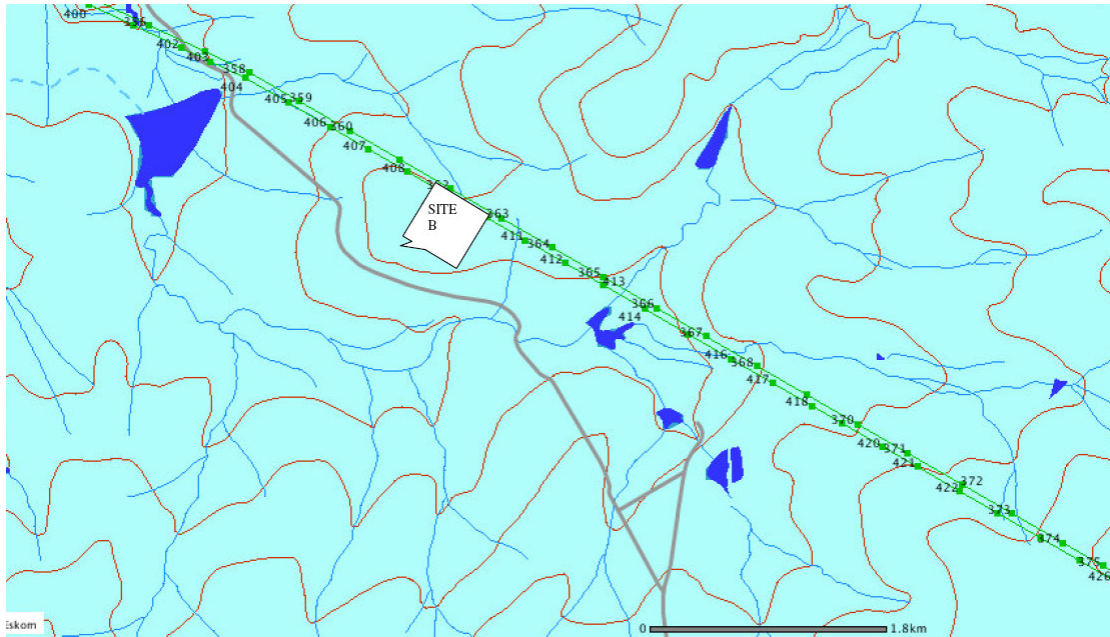
9.0 REFERENCES

- 9.1** Jennings Brink & Williams (1973). Revised Guide to Soil Profiling for Civil Engineering purposes in South Africa. The civil Engineer in S.A. Jan. 1973.

APPENDIX-A

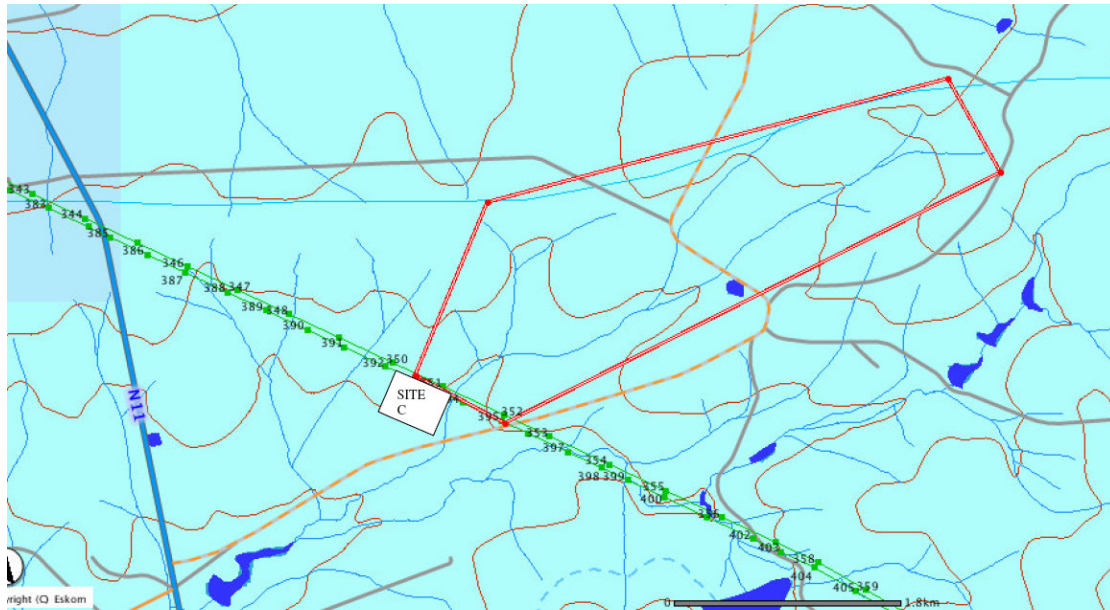
Flood line Studies

Burotho Sites



MOKOPANE SITE-B

SITE B IS SITUATED CLOSE TO A RIDGE AND THEREFORE HAS A VERY SMALL CATCHMENT WHICH DELIVERS A MAXIMUM FLOW OF 7.10 CUBIC METERS PER SECOND IN A 1:50 FLOOD. THE NEAREST WATER COURSE IS 120 METERS AWAY FROM THE SITE AND THE EXPECTED WIDTH A FLOW IS ONLY 27.5 METERS WHICH MEANS THAT THIS SITE HAS NO DANGER OF FLOODING UNDER A 1:50 YEAR FLOOD.



MOKOPANE SITE-C











SITE C IS SITUATED CLOSE TO A TRIBUTARY WHICH HAS A CATCHMENT OF 4.73 SQUARE KILOMETERS WHICH DELIVERS A MAXIMUM FLOW OF 42.3 CUBIC METERS PER SECOND IN A 1:50 FLOOD. THE NEAREST WATER COURSE IS 20.4 METERS AWAY FROM THE SITE AND THE EXPECTED WIDTH A FLOW IS 51.5 METERS WHICH MEANS THAT THIS SITE HAS AN APPARENT POSSIBILITY OF FLOODING UNDER A 1:50 YEAR FLOOD.

APPENDIX - B
SOIL PROFILES

PROJECT: BURUTHO SUBSTATION SITE SELECTION-SITE-C
 Done by: Specialist Consultant
 Geotechnical Investigations
TRANSMISSION TECHNOLOGY

LEGEND
 Sheet 1 of 1

JOB NUMBER: GR03-08

	BOULDERS	{SA01}
	GRAVEL	{SA02}
	SAND	{SA04}
	SANDY	{SA05}
	SILT	{SA06}
	SILTY	{SA07}
	CLAY	{SA08}
	CLAYEY	{SA09}
	GRANITE	{SA17}{SA44}
	DISTURBED SAMPLE	{SA38}

Name ●

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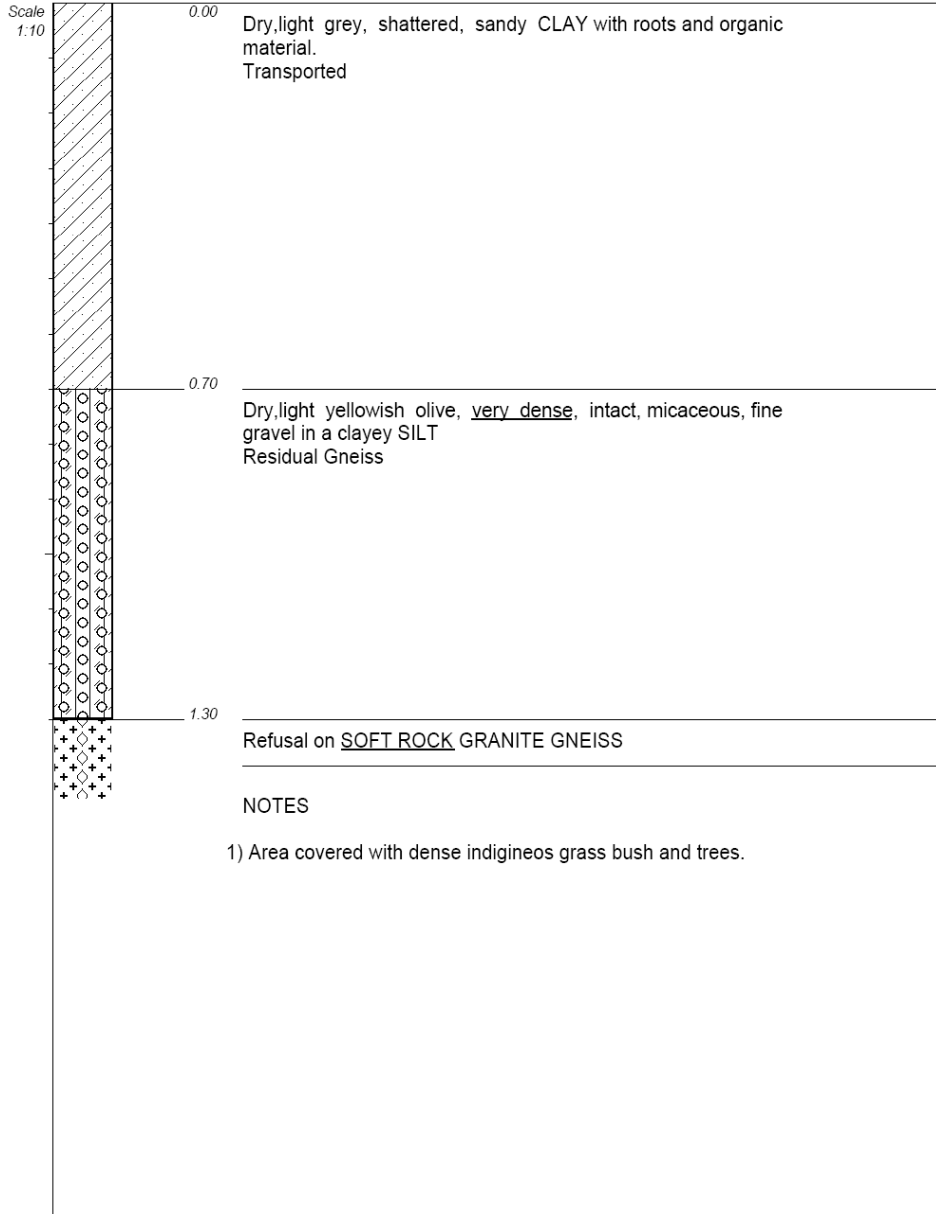
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 Y-COORD :

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 SUMMARY OF SYMBOLS

PROJECT: BURUTHO SUBSTATION SITE SELECTION-SITE-C
Done by: Specialist Consultant
Geotechnical Investigations
TRANSMISSION TECHNOLOGY

HOLE No: TP-M1
 Sheet 1 of 1

JOB NUMBER: GR03-08



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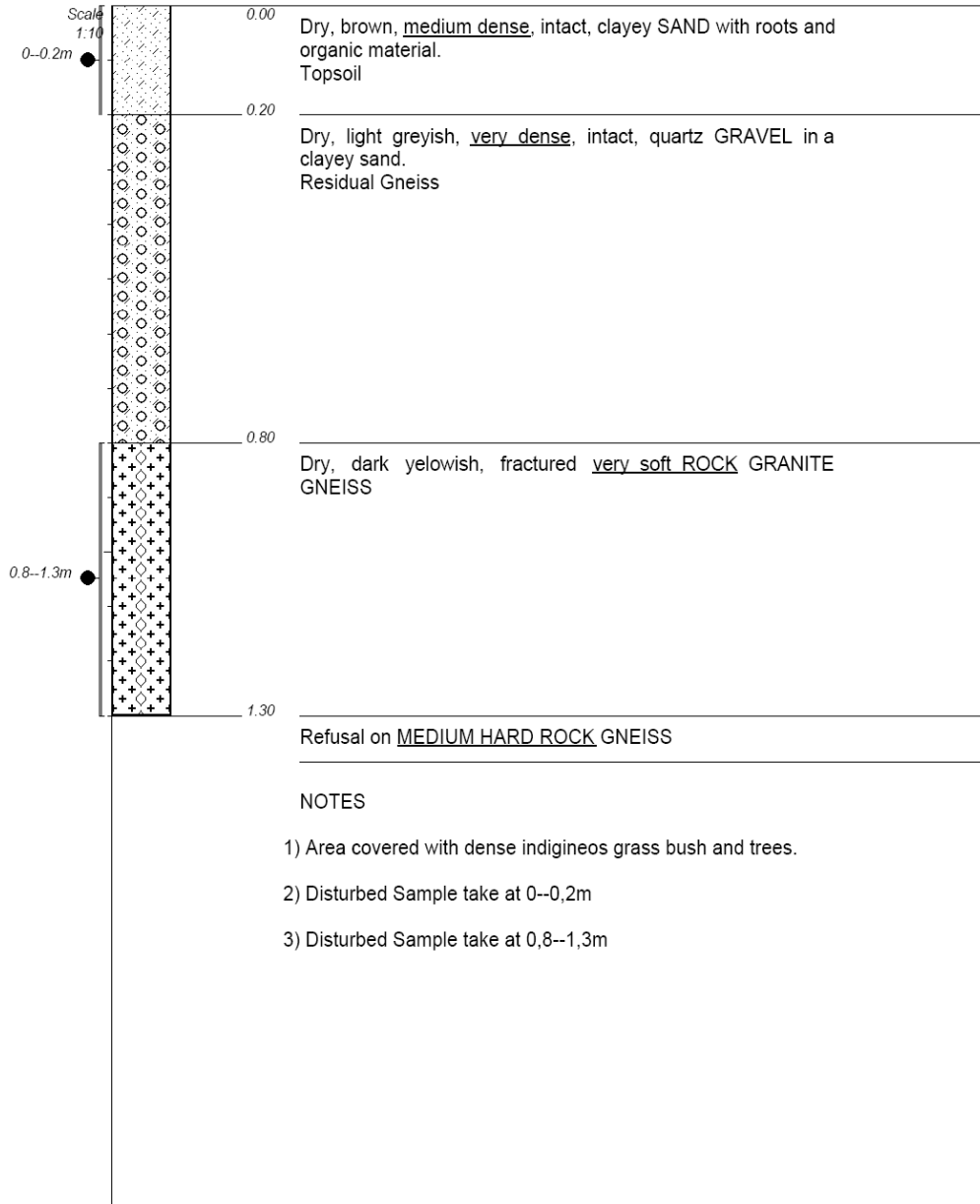
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Done by: Specialist Consultant
Geotechnical Investigations
TRANSMISSION TECHNOLOGY

HOLE No: TP-M2
 Sheet 1 of 1

JOB NUMBER: GR03-08



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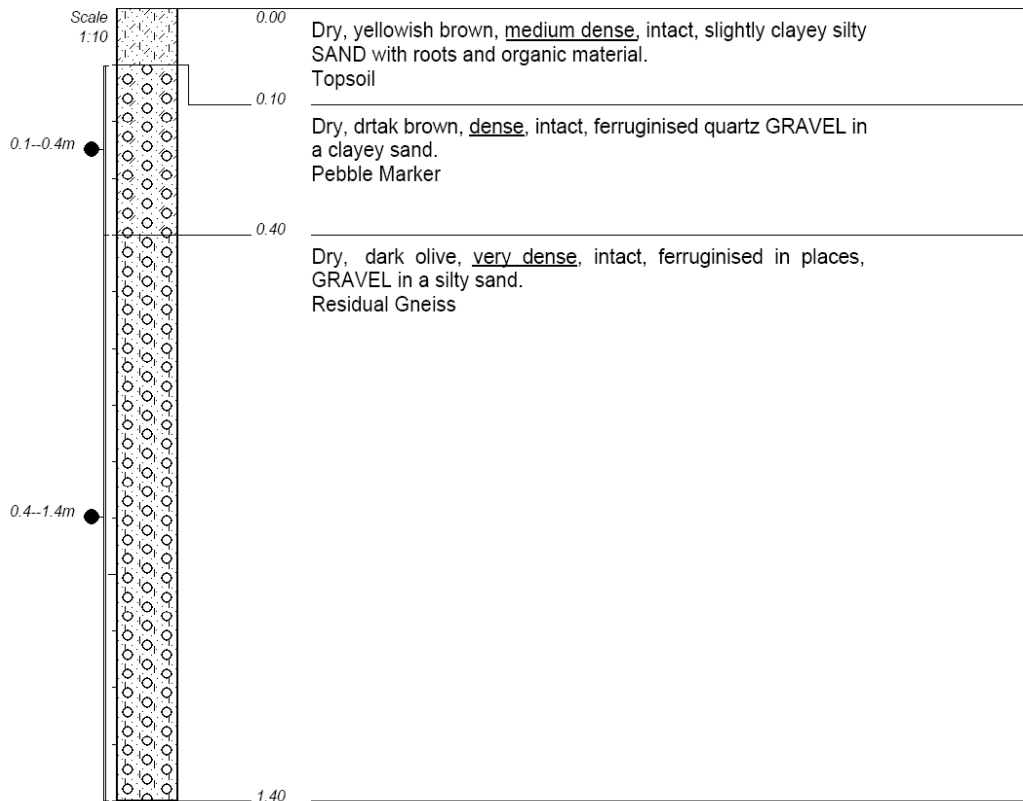
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HOLE No: TP-M2

PROJECT: BURUTHO SUBSTATION SITE SELECTION-SITE-C
Done by: Specialist Consultant
Geotechnical Investigations
TRANSMISSION TECHNOLOGY

HOLE No: TP-M3
 Sheet 1 of 1

JOB NUMBER: GR03-08



Refusal on MEDIUM HARD ROCK GRANITE GNEISS

NOTES

- 1) Area covered with dense indigineos grass bush and trees
- 2) Disturbed Sample take at 0,1--0,4m
- 3) Disturbed Sample take at 0,4--1,4m

CONTRACTOR : Tyris Plant Hire
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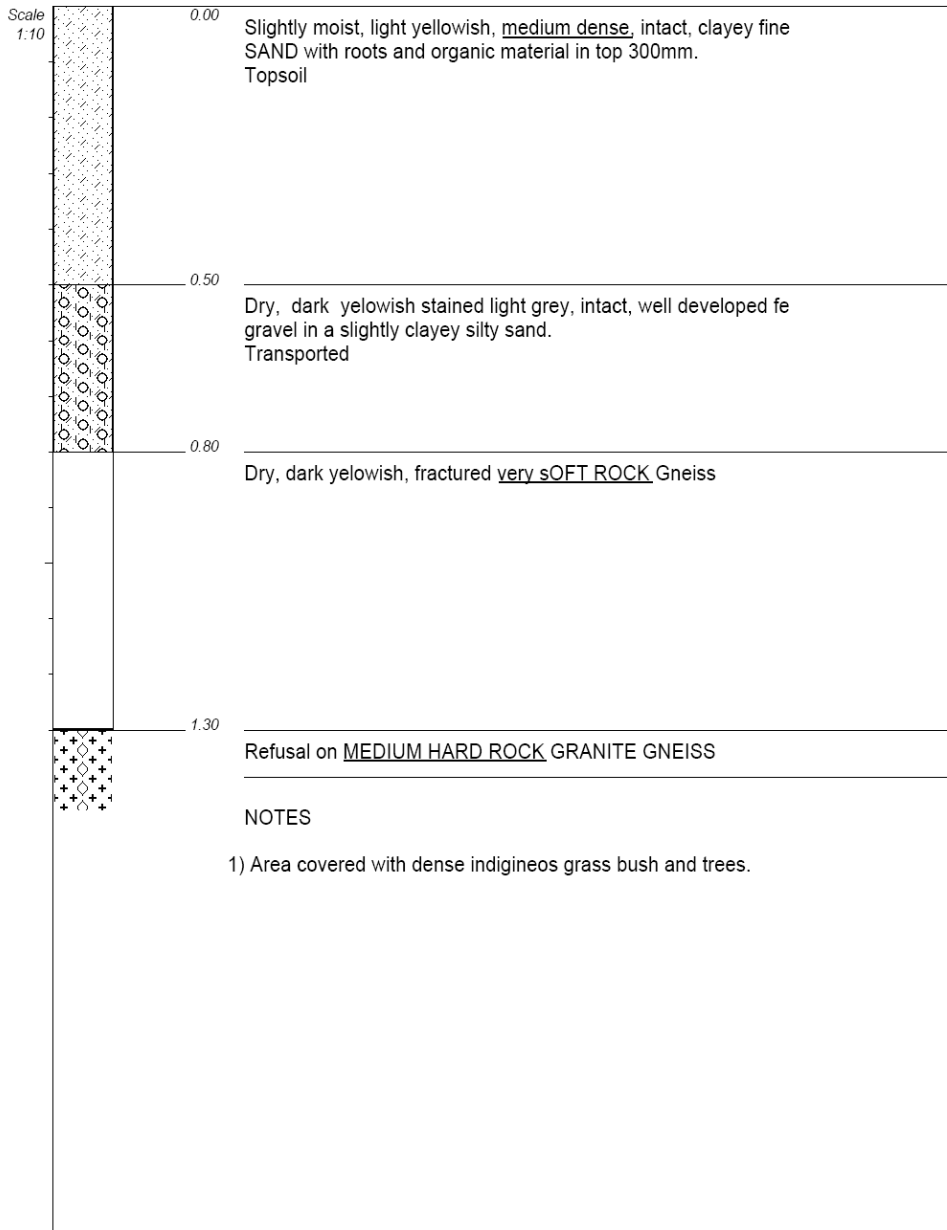
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PROJECT: BURUTHO SUBSTATION SITE SELECTION-SITE-C
Done by: Specialist Consultant
Geotechnical Investigations
TRANSMISSION TECHNOLOGY

HOLE No: TP-M4
 Sheet 1 of 1

JOB NUMBER: GR03-08



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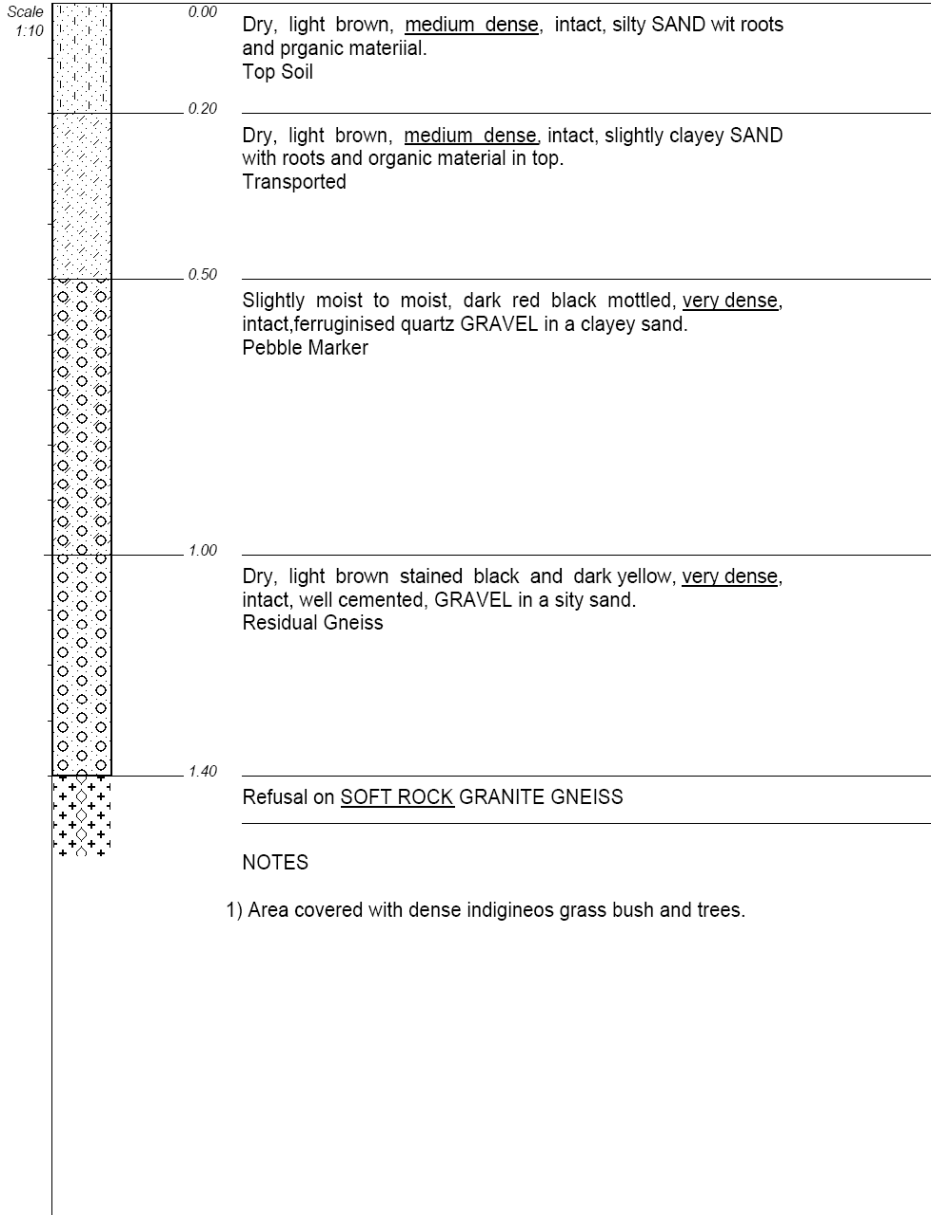
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PROJECT: BURUTHO SUBSTATION SITE SELECTION-SITE-C
Done by: Specialist Consultant
Geotechnical Investigations
TRANSMISSION TECHNOLOGY

HOLE No: TP-M5
 Sheet 1 of 1

JOB NUMBER: GR03-08



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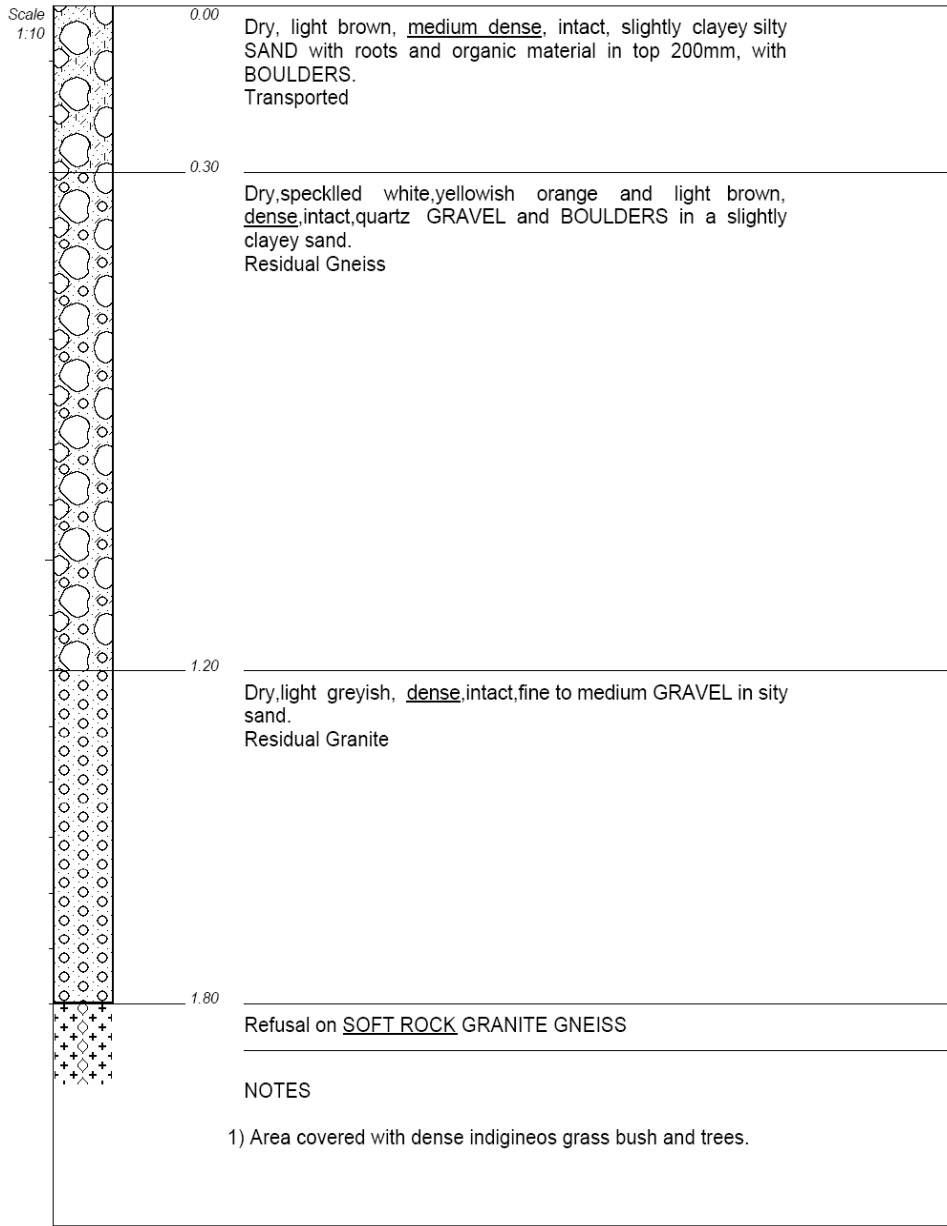
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HOLE No: TP-M5

PROJECT: BURUTHO SUBSTATION SITE SELECTION-SITE-B
Done by: Specialist Consultant
Geotechnical Investigations
TRANSMISSION TECHNOLOGY

HOLE No: TP-MB1
Sheet 1 of 1

JOB NUMBER: GR03-08



CONTRACTOR : Tyris Plant Hire
MACHINE : CAT 428C
DRILLED BY :
PROFIED BY : FA GrovÈ
TYPE SET BY :
SETUP FILE : PROF-1.SET

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DIAM : Trench Pit
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DATE : 15/03/10
DATE : 08/06/10 13:05
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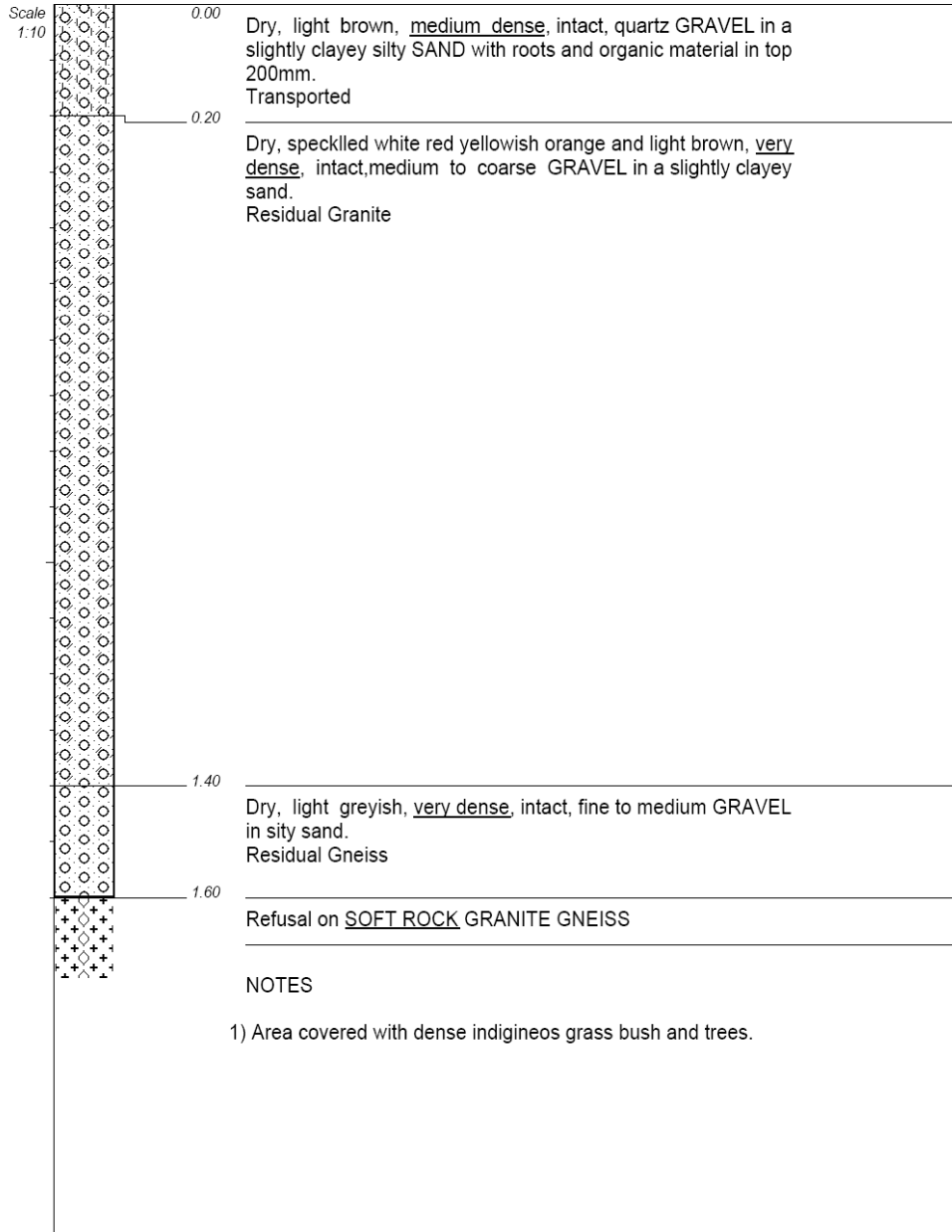
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Y-COORD : 2641929

HOLE No: TP-MB1

PROJECT: BURUTHO SUBSTATION SITE SELECTION-SITE-B
Done by: Specialist Consultant
Geotechnical Investigations
TRANSMISSION TECHNOLOGY

HOLE No: TP-MB2
 Sheet 1 of 1

JOB NUMBER: GR03-08



CONTRACTOR : Tyris Plant Hire
 MACHINE : CAT 428C
 DRILLED BY :
 PROFILED BY : FA GrovÉ
 TYPE SET BY :
 SETUP FILE : PROF-1.SET

INCLINATION : VERTICAL
 DIAM : Trench Pit
 DATE :
 DATE : 15/03/10
 DATE : 08/06/10 13:05
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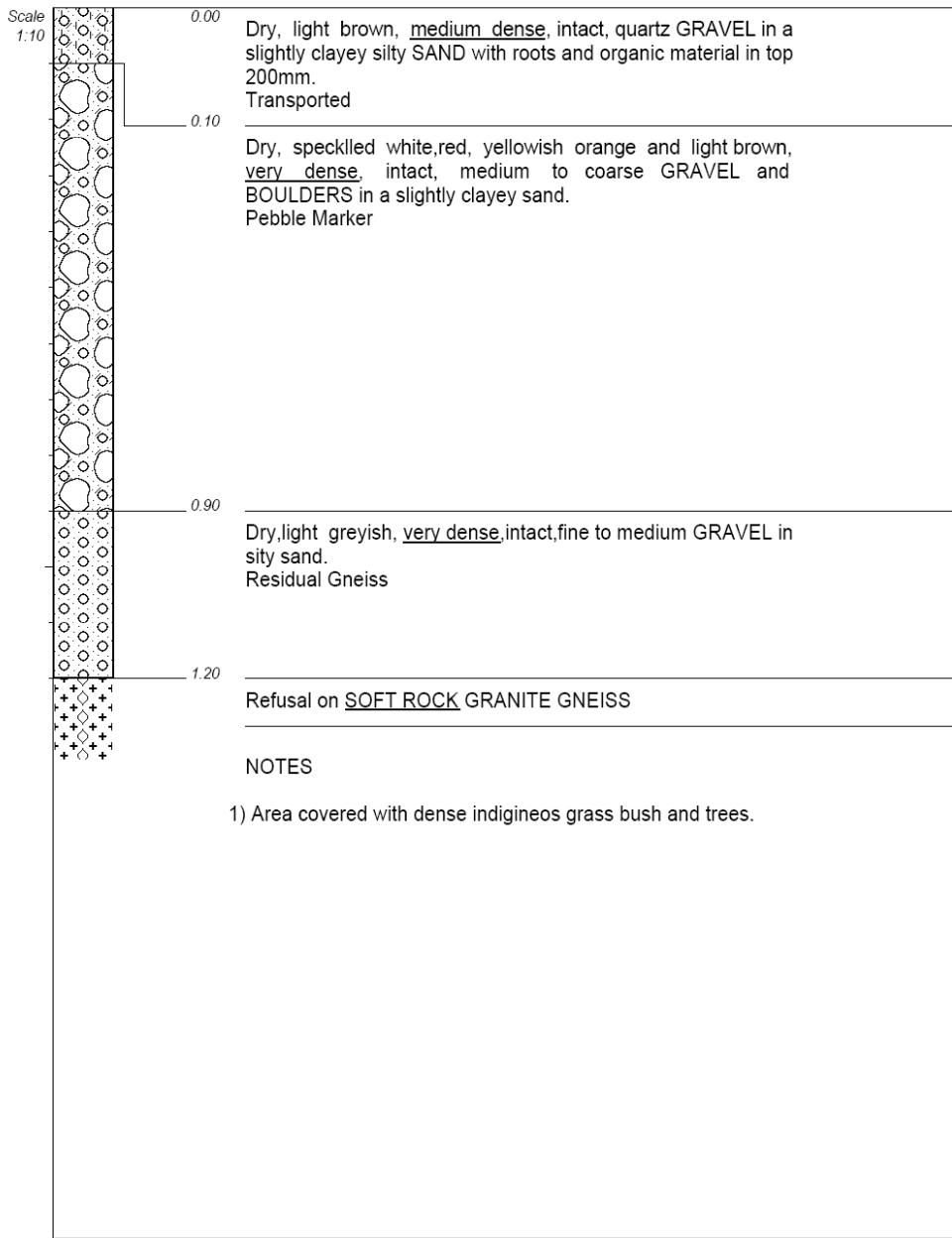
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 X-COORD : 2586
 Y-COORD : 2644106

HOLE No: TP-MB2

PROJECT: BURUTHO SUBSTATION SITE SELECTION-SITE-B
Done by: Specialist Consultant
Geotechnical Investigations
TRANSMISSION TECHNOLOGY

HOLE No: TP-MB3
 Sheet 1 of 1

JOB NUMBER: GR03-08



CONTRACTOR : Tyris Plant Hire
 MACHINE : CAT 428C
 DRILLED BY :
 PROFILED BY : FA GrovÉ
 TYPE SET BY :
 SETUP FILE : PROF-1.SET

INCLINATION : VERTICAL
 DIAM : Trench Pit
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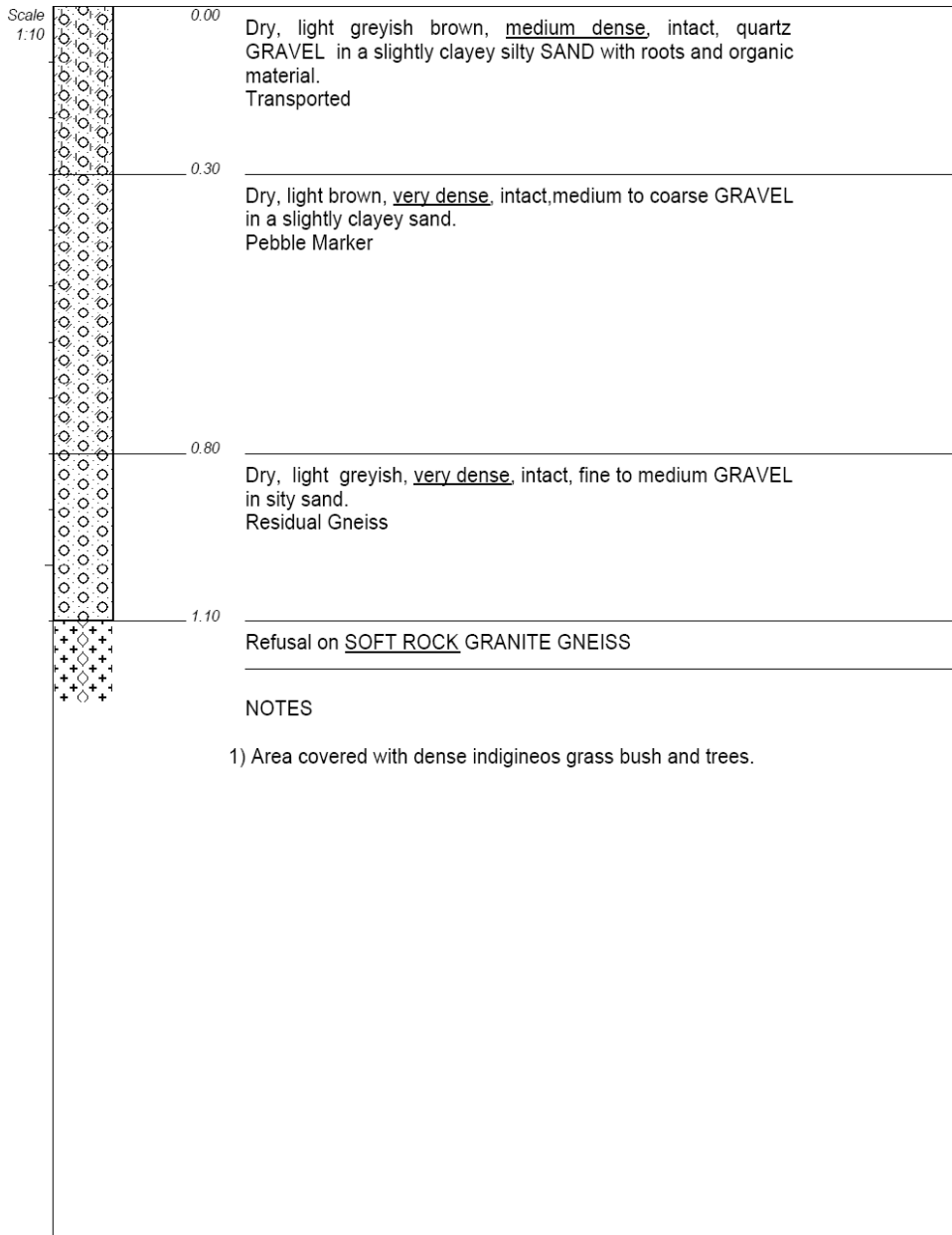
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 Y-COORD : 2643866

HOLE No: TP-MB3

PROJECT: BURUTHO SUBSTATION SITE SELECTION-SITE-B
Done by: Specialist Consultant
Geotechnical Investigations
TRANSMISSION TECHNOLOGY

HOLE No: TP-MB4
 Sheet 1 of 1

JOB NUMBER: GR03-08



CONTRACTOR : Tyris Plant Hire
 MACHINE : CAT 428C
 DRILLED BY :
 PROFILED BY : FA GrovÈ
 TYPE SET BY :
 SETUP FILE : PROF-1.SET

INCLINATION : VERTICAL
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 DATE :
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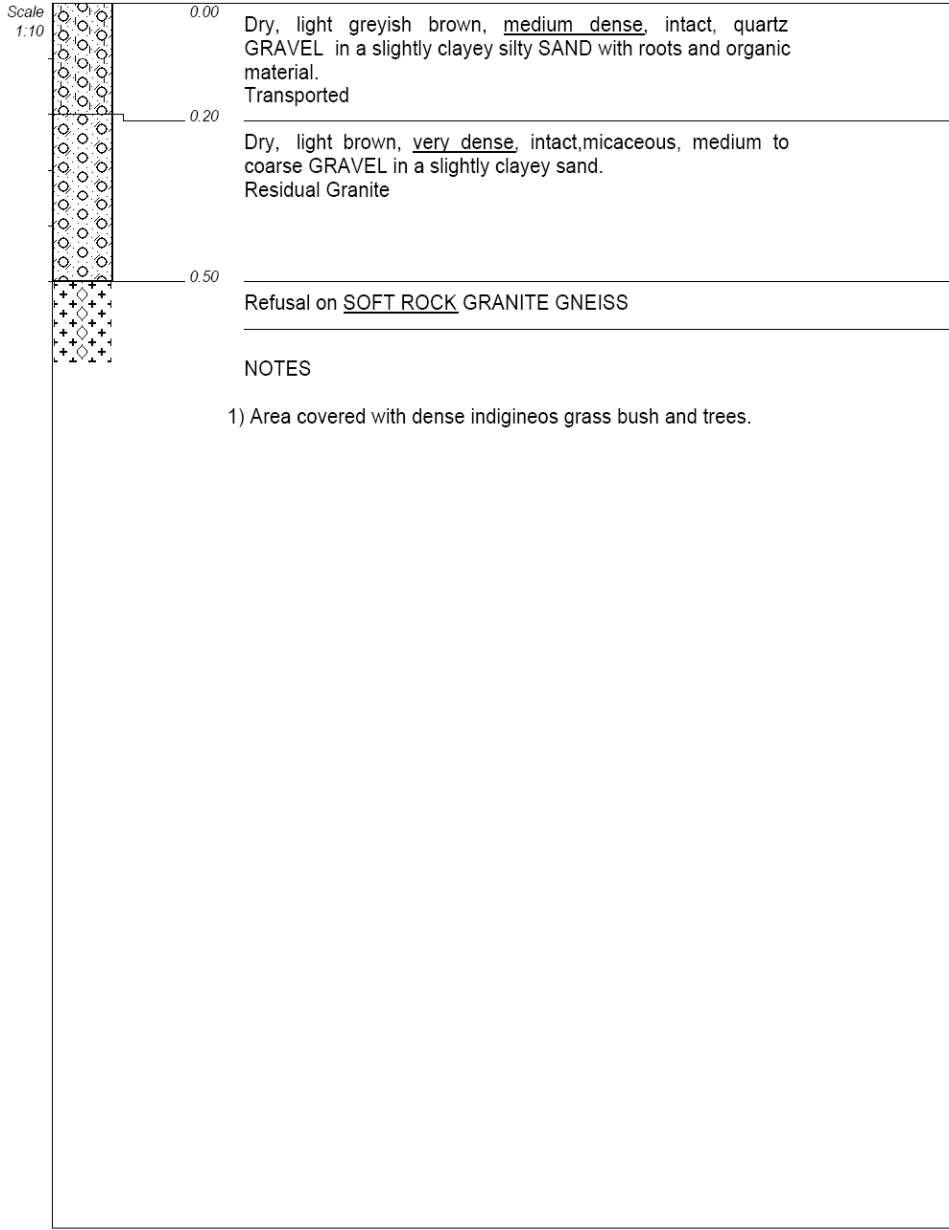
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 Y-COORD : 2644016

HOLE No: TP-MB4

PROJECT: BURUTHO SUBSTATION SITE SELECTION-SITE-B
Done by: Specialist Consultant
Geotechnical Investigations
TRANSMISSION TECHNOLOGY

HOLE No: TP-MB5
Sheet 1 of 1

JOB NUMBER: GR03-08



CONTRACTOR: Tyris Plant Hire
MACHINE: CAT 428C
DRILLED BY:
PROFILED BY: FA GrovÈ
TYPE SET BY:
SETUP FILE: PROF-1.SET

INCLINATION: VERTICAL
DIAM: Trench Pit
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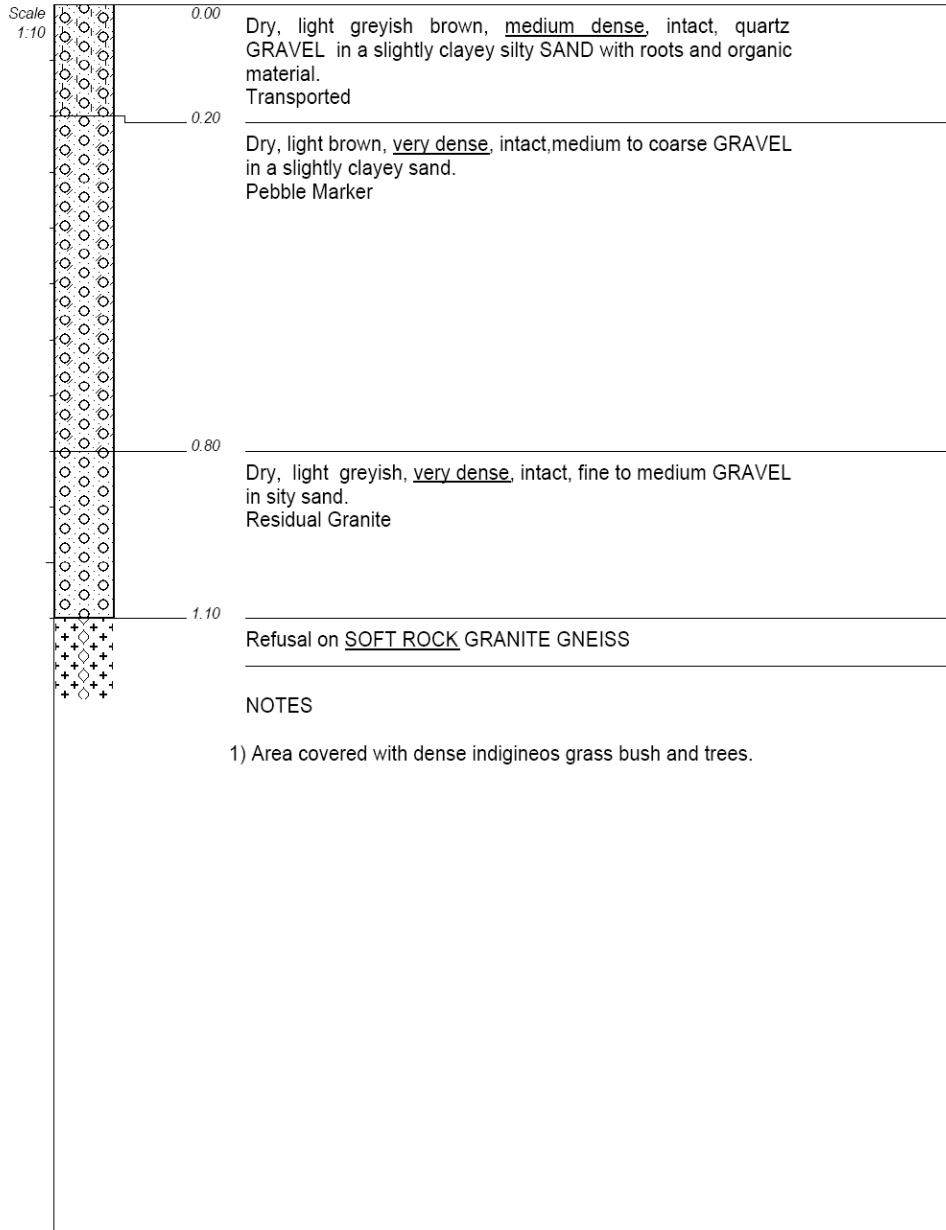
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X-COORD: 2036
Y-COORD: 2644135

HOLE No: TP-MB5

PROJECT: BURUTHO SUBSTATION SITE SELECTION-SITE-B
Done by: Specialist Consultant
Geotechnical Investigations
TRANSMISSION TECHNOLOGY

HOLE No: TP-MB6
 Sheet 1 of 1

JOB NUMBER: GR03-08



CONTRACTOR: Tyris Plant Hire
 MACHINE: CAT 428C
 DRILLED BY:
 PROFILED BY: FA GrovĚ
 TYPE SET BY:
 SETUP FILE: PROF-1.SET

INCLINATION: VERTICAL
 DIAM: Trench Pit
 DATE:
 DATE: 15/03/10
 DATE: 08/06/10 13:05
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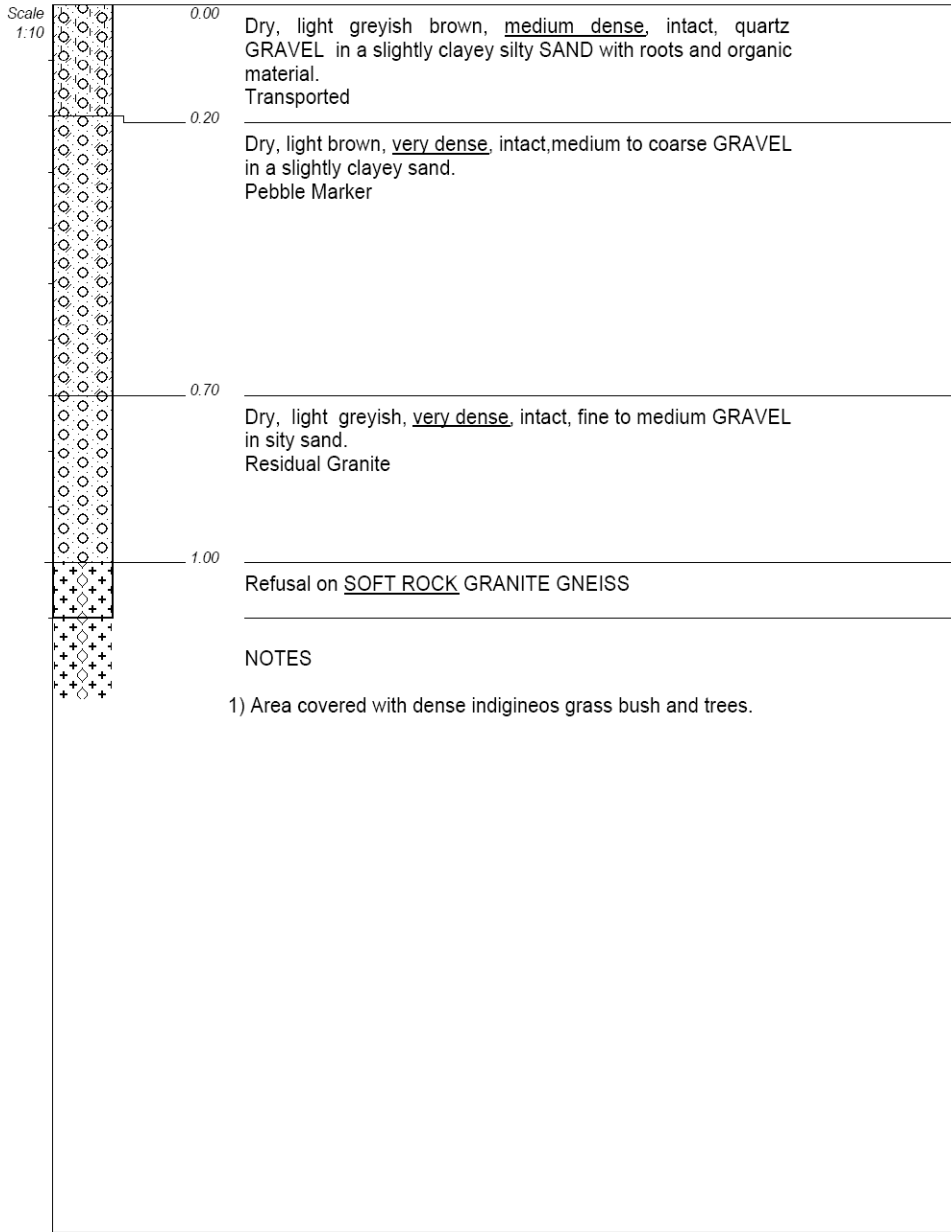
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 X-COORD: 2107
 Y-COORD: 2644401

HOLE No: TP-MB6

PROJECT: BURUTHO SUBSTATION SITE SELECTION-SITE-B
Done by: Specialist Consultant
Geotechnical Investigations
TRANSMISSION TECHNOLOGY

HOLE No: TP-MB7
 Sheet 1 of 1

JOB NUMBER: GR03-08



CONTRACTOR: Tyriss Plant Hire
 MACHINE: CAT 428C
 DRILLED BY:
 PROFILED BY: FA GrovĚ
 TYPE SET BY:
 SETUP FILE: PROF-1.SET

INCLINATION: VERTICAL
 DIAM: Trench Pit
 DATE:
 DATE: 15/03/10
 DATE: 08/06/10 13:05
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ELEVATION: NGL
 X-COORD: 2340
 Y-COORD: 2644264

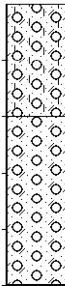
HOLE No: TP-MB7

PROJECT: BURUTHO SUBSTATION SITE SELECTION-SITE-B
Done by: Specialist Consultant
Geotechnical Investigations
TRANSMISSION TECHNOLOGY

HOLE No: TP-MB8
Sheet 1 of 1

JOB NUMBER: GR03-08

Scale
1:10



0.00 Dry, light greyish brown, medium dense, intact, quartz GRAVEL in a slightly clayey silty SAND with roots and organic material.
Transported

0.20 Dry, light brown, very dense, intact, medium to coarse GRAVEL in a slightly clayey sand.
Residual Granite

0.50 Refusal on HARD ROCK GNEISS

NOTES

- 1) Area covered with dense indigineous grass bush and trees.
- 2) Trees 100mm to 300mm

CONTRACTOR : Tyris Plant Hire
MACHINE : CAT 428C
DRILLED BY :
PROFILED BY : FA GrovÉ
TYPE SET BY :
SETUP FILE : PROF-1.SET

INCLINATION : VERTICAL
DIAM : Trench Pit
DATE :
DATE : 15/03/10

ELEVATION : NGL
X-COORD : 2340
Y-COORD : 2644264

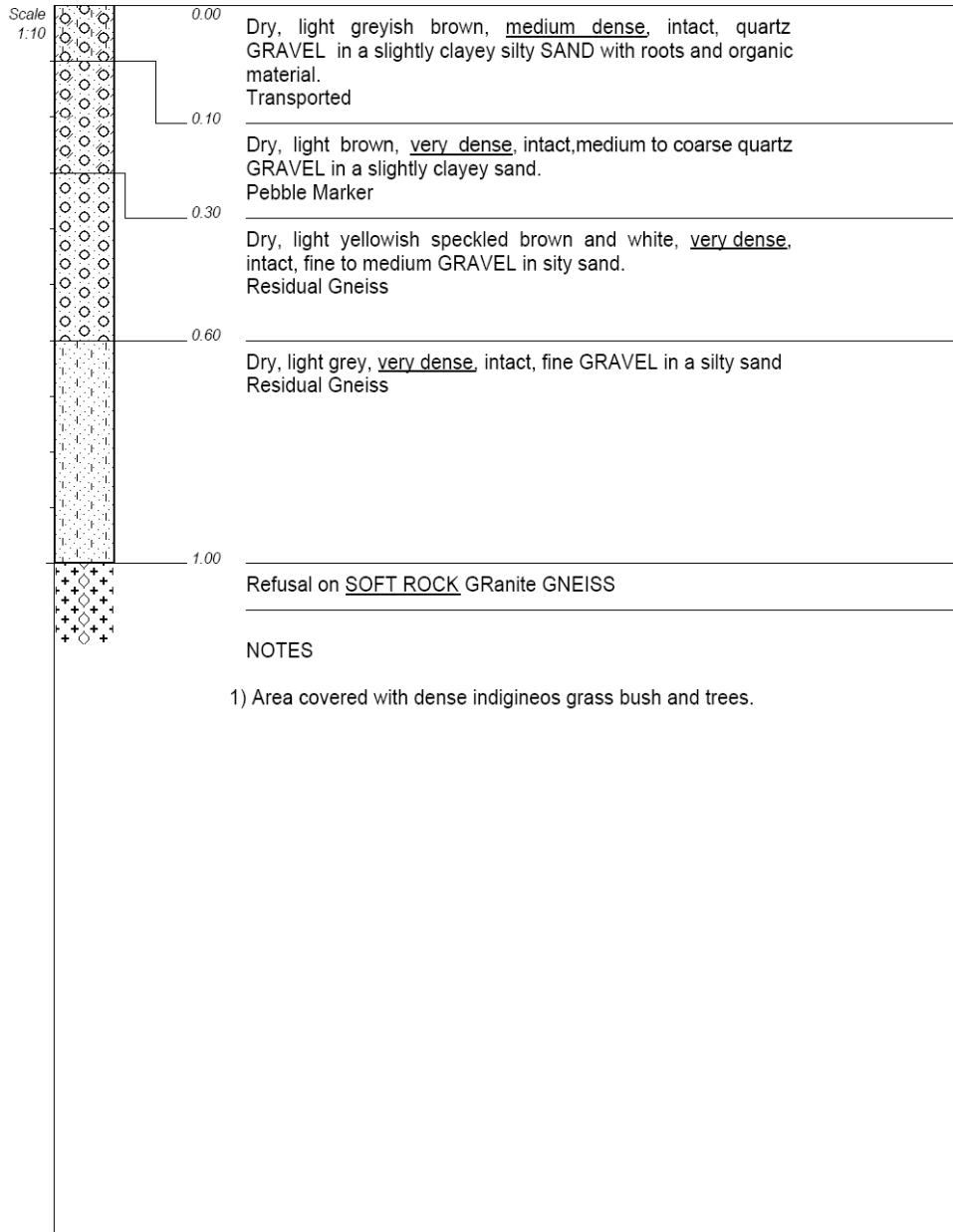
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PROJECT: BURUTHO SUBSTATION SITE SELECTION-SITE-B
Done by: Specialist Consultant
Geotechnical Investigations
TRANSMISSION TECHNOLOGY

HOLE No: TP-MB9
 Sheet 1 of 1

JOB NUMBER: GR03-08



CONTRACTOR: Tyriss Plant Hire
 MACHINE: CAT 428C
 DRILLED BY: FA GrovÈ
 PROFILED BY: FA GrovÈ
 TYPE SET BY:
 SETUP FILE: PROF-1.SET

INCLINATION: VERTICAL
 DIAM: Trench Pit
 DATE: 15/03/10
 DATE: 08/06/10 13:05
 TEXT: ..C:\GEOFRED\MAKTPPF.TXT

ELEVATION: NGL
 X-COORD: 2281
 Y-COORD: 2644707

HOLE No: TP-MB9

APPENDIX - C
Laboratory Test Results

CLIENT : ESKOM TRANSMISSION
PROJECT : BUROTHO (MAKOPANE) SUBSTATION
PROJECT NO. : S10-0401
DATE : 2010-04-01

CONDUCTIVITY

Soillab No	Sample Position	Sample Depth (m)	Electrical Conductivity S/m
S10-0401-01	TP-M2	0-0.2	0.0456
S10-0401-02	TP-M2	0.8-1.3	0.0257
S10-0401-03	TP-M3	0.1-0.4	0.0311
S10-0401-04	TP-M3	0.4-1.4	0.0668
S10-0401-05	TP-MB4	0-0.3	0.0208
S10-0401-06	TP-MB4	0.3-0.8	0.0190
S10-0401-07	TP-MB4	0.8-1.1	0.0187

0401-01.doc

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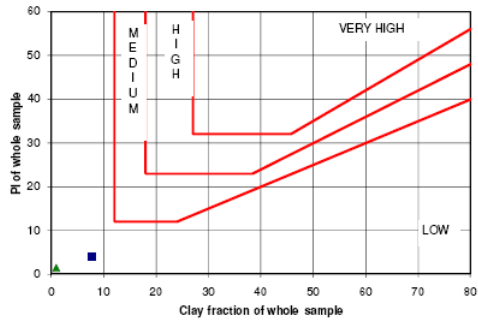
P O Box 72928
Lynnwood Ridge 0040
Fax (012) 481-3812

PARTICLE SIZE ANALYSIS

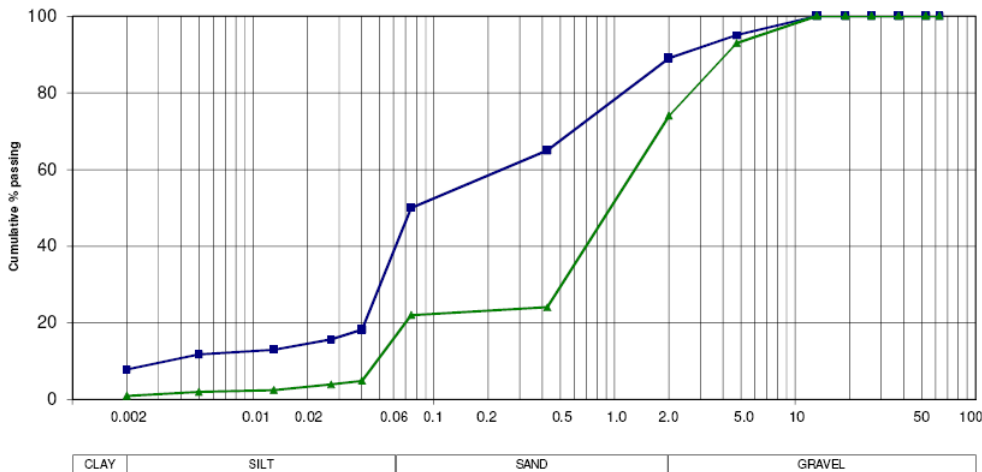
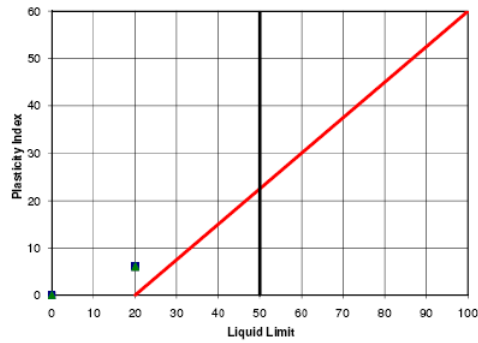
Sample No.	44362	44363
Soillab sample no.	S10-0401-01	S10-0401-02
Depth (m)	0-0.2	0.8-1.3
Position	TP-M2	TP-M2
Material Description	DARK BROWN QUARTZ	DARK OLIVE DOLERITE
	SLTY SAND	GRAVELLY SAND
Moisture (%)	4.2	3.4
SG		
SCREEN ANALYSIS (% PASSING) (TMH 1 A1(a) & A5)		
63.0 mm	100	100
53.0 mm	100	100
37.5 mm	100	100
26.5 mm	100	100
19.0 mm	100	100
13.2 mm	100	100
4.75 mm	95	93
2.00 mm	89	74
0.425 mm	65	24
0.075 mm	50	22
HYDROMETER ANALYSIS (% PASSING) (TMH 1 A6)		
0.040 mm	18	5
0.027 mm	16	4
0.013 mm	13	2
0.005 mm	12	2
0.002 mm	8	1
% Clay	8	1
% Silt	29	14
% Sand	53	59
% Gravel	11	26
ATTERBERG LIMITS (TMH 1 A2 - A4)		
Liquid Limit	20	20
Plasticity Index	6	6
Linear Shrinkage (%)	3.0	3.0
Grading Modulus	0.96	1.80
Classification	A-4 (3)	A-1-b (0)
Unified Classification	CL-ML	SC & SM
Chart Reference	—■—	—▲—

PROJECT : BUROTHO (MAKOPANE) SUBSTATION
 JOB No. : S10-0401
 DATE : 2010-04-22

POTENTIAL EXPANSIVENESS



PLASTICITY CHART



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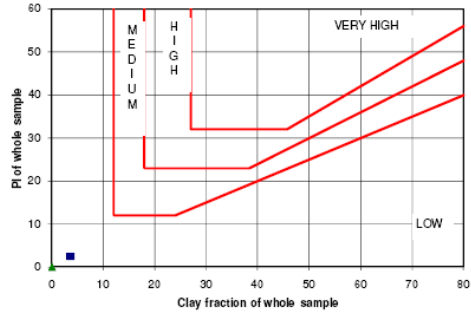
P O Box 72928
 Lynnwood Ridge 0040
 Fax (012) 481-3812

PARTICLE SIZE ANALYSIS

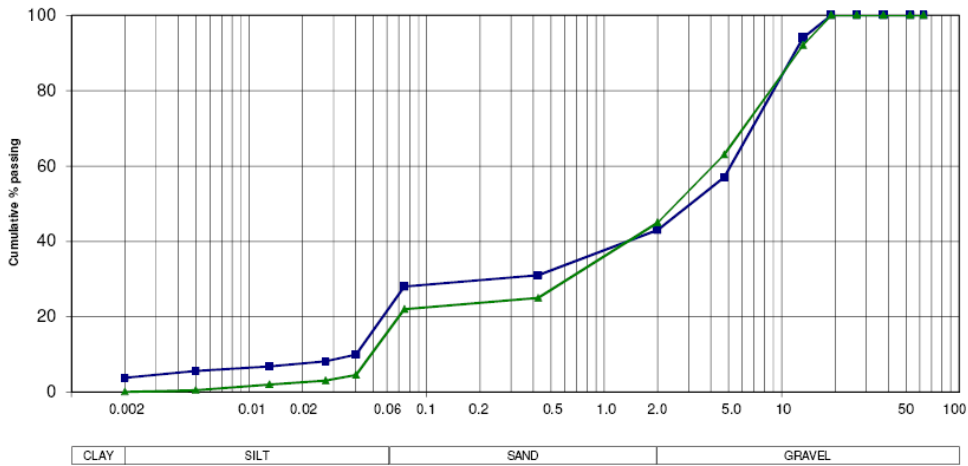
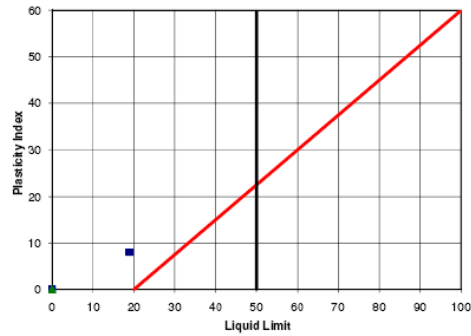
Sample No.	44364	44365
Soillab sample no.	S10-0401-03	S10-0401-04
Depth (m)	0.1-0.4	0.4-1.4
Position	TP-M3	TP-M3
Material Description	DARK BROWN QUARTZ	DARK BROWN RHYOLITE
	SANDY GRAVEL	SANDY GRAVEL
Moisture (%)	3.0	6.6
SG		
SCREEN ANALYSIS (% PASSING) (TMH 1 A1(a) & A5)		
63.0 mm	100	100
53.0 mm	100	100
37.5 mm	100	100
26.5 mm	100	100
19.0 mm	100	100
13.2 mm	94	92
4.75 mm	57	63
2.00 mm	43	45
0.425 mm	31	25
0.075 mm	28	22
HYDROMETER ANALYSIS (% PASSING) (TMH 1 A6)		
0.040 mm	10	5
0.027 mm	8	3
0.013 mm	7	2
0.005 mm	6	1
0.002 mm	4	0
% Clay	4	0
% Silt	17	14
% Sand	23	31
% Gravel	57	55
ATTERBERG LIMITS (TMH 1 A2 - A4)		
Liquid Limit	19	
Plasticity Index	8	SP
Linear Shrinkage (%)	4.0	0.5
Grading Modulus	1.98	2.08
Classification	A-2-4 (0)	A-1-b (0)
Unified Classification	GC	SM
Chart Reference■.....▲.....

PROJECT : BUROTHO (MAKOPANE) SUBSTATION
 JOB No. : S10-0401
 DATE : 2010-04-22

POTENTIAL EXPANSIVENESS



PLASTICITY CHART



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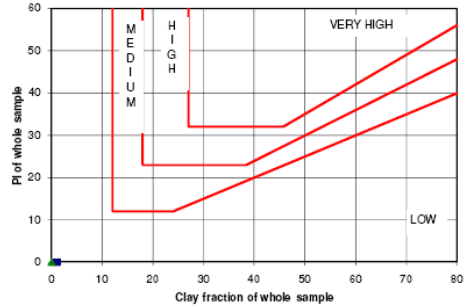
P O Box 72928
 Lynnwood Ridge 0040
 Fax (012) 481-3812

PARTICLE SIZE ANALYSIS

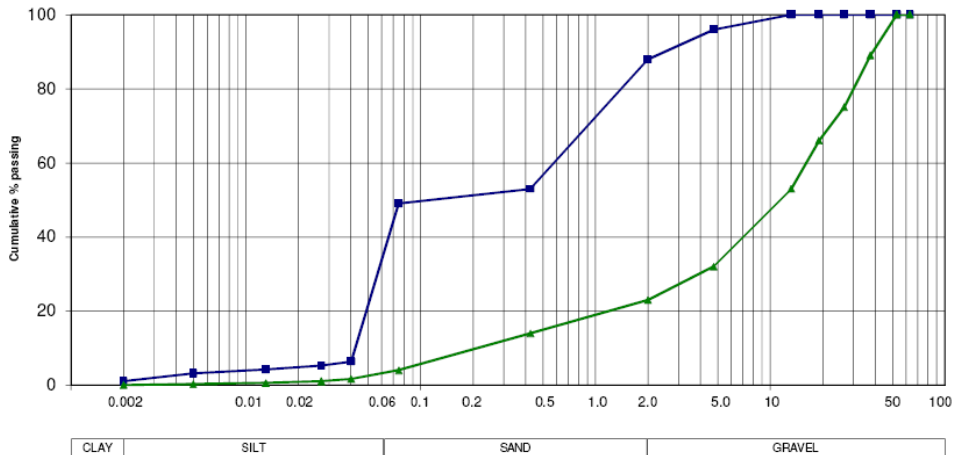
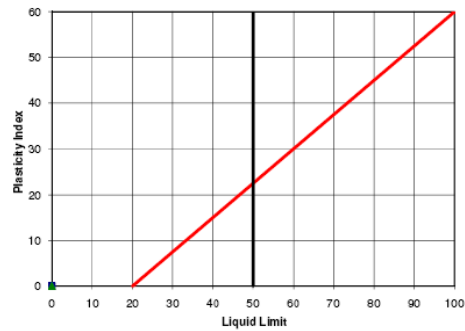
Sample No.	44366	44367
Soillab sample no.	S10-0401-05	S10-0401-06
Depth (m)	0-0.3	0.3-0.8
Position	TP-MB4	TP-MB4
Material Description	DARK BROWN QUARTZ	DARK BROWN QUARTZ
	SILTY SAND	SANDY GRAVEL
Moisture (%)	1.1	1.0
SG		
SCREEN ANALYSIS (% PASSING) (TMH 1 A1(a) & A5)		
63.0 mm	100	100
53.0 mm	100	100
37.5 mm	100	89
26.5 mm	100	75
19.0 mm	100	66
13.2 mm	100	53
4.75 mm	96	32
2.00 mm	88	23
0.425 mm	53	14
0.075 mm	49	4
HYDROMETER ANALYSIS (% PASSING) (TMH 1 A6)		
0.040 mm	6	2
0.027 mm	5	1
0.013 mm	4	1
0.005 mm	3	0
0.002 mm	1	0
% Clay	1	0
% Silt	30	3
% Sand	57	20
% Gravel	12	77
ATTERBERG LIMITS (TMH 1 A2 - A4)		
Liquid Limit		
Plasticity Index	NP	SP
Linear Shrinkage (%)	0.0	0.5
Grading Modulus	1.10	2.59
Classification	A-4 (3)	A-1-a (0)
Unified Classification	SM	GP
Chart Reference	-----■-----	-----▲-----

PROJECT : BUROTHO (MAKOPANE) SUBSTATION
 JOB No. : S10-0401
 DATE : 2010-04-22

POTENTIAL EXPANSIVENESS



PLASTICITY CHART



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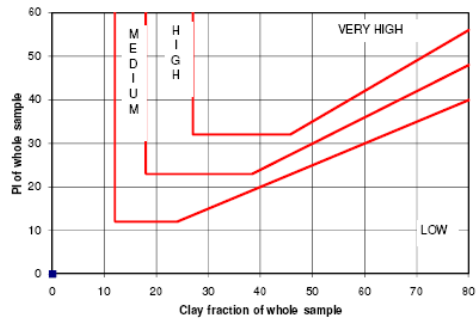
P O Box 72928
 Lynnwood Ridge 0040
 Fax (012) 481-3812

PARTICLE SIZE ANALYSIS

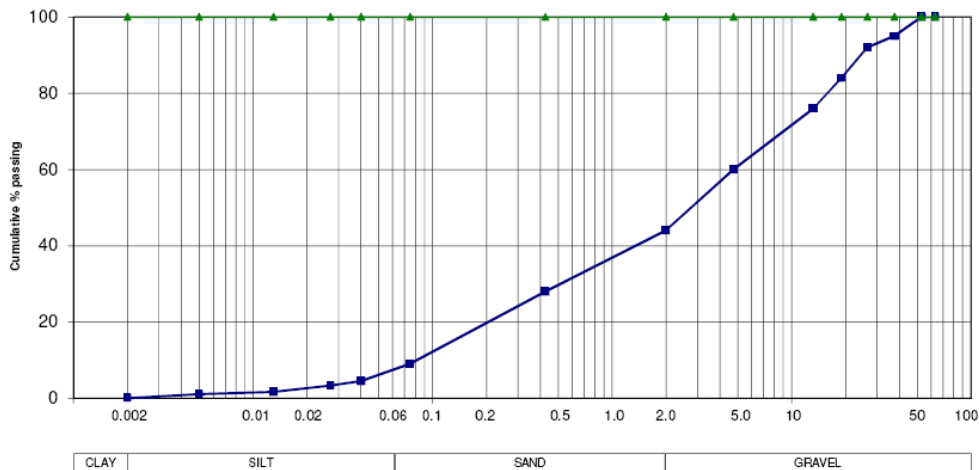
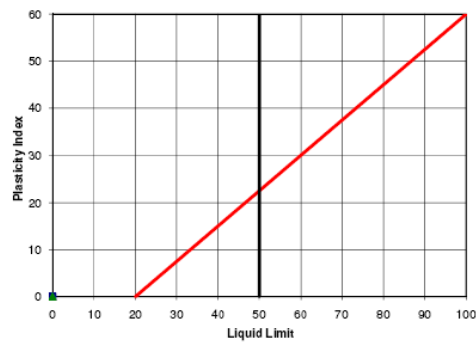
Sample No.	44368	
Soillab sample no.	S10-0401-07	
Depth (m)	0.8-1.1	
Position	TP-MB4	
Material Description	DARK BROWN QUARTZ SANDY GRAVEL	
Moisture (%)	1.0	
SG		
SCREEN ANALYSIS (% PASSING) (TMH 1 A1(a) & A5)		
63.0 mm	100	
53.0 mm	100	
37.5 mm	95	
26.5 mm	92	
19.0 mm	84	
13.2 mm	76	
4.75 mm	60	
2.00 mm	44	
0.425 mm	28	
0.075 mm	9	
HYDROMETER ANALYSIS (% PASSING) (TMH 1 A6)		
0.040 mm	4	
0.027 mm	3	
0.013 mm	2	
0.005 mm	1	
0.002 mm	0	
% Clay	0	
% Silt	7	
% Sand	37	
% Gravel	56	
ATTERBERG LIMITS (TMH 1 A2 - A4)		
Liquid Limit		
Plasticity Index	NP	
Linear Shrinkage (%)	0.0	
Grading Modulus	2.19	
Classification	A-1-a (0)	
Unified Classification	SW & SC	
Chart Reference	—■—	—▲—

PROJECT : BUROTHO (MAKOPANE) SUBSTATION
 JOB No. : S10-0401
 DATE : 2010-04-22

POTENTIAL EXPANSIVENESS



PLASTICITY CHART



SOILLAB

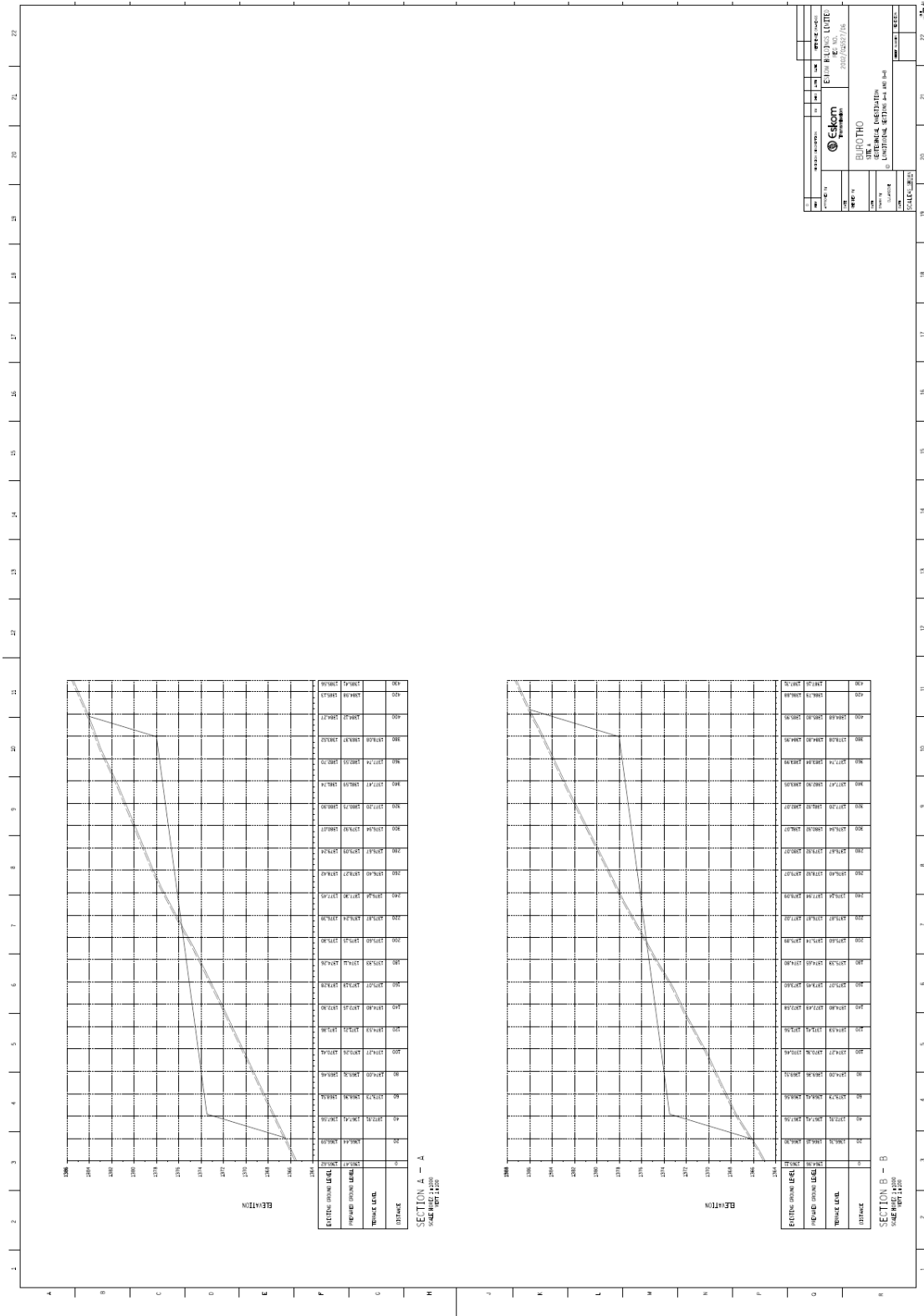
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APPENDIX - D

Contour Plans, Preliminary Geometric Designs and Sections



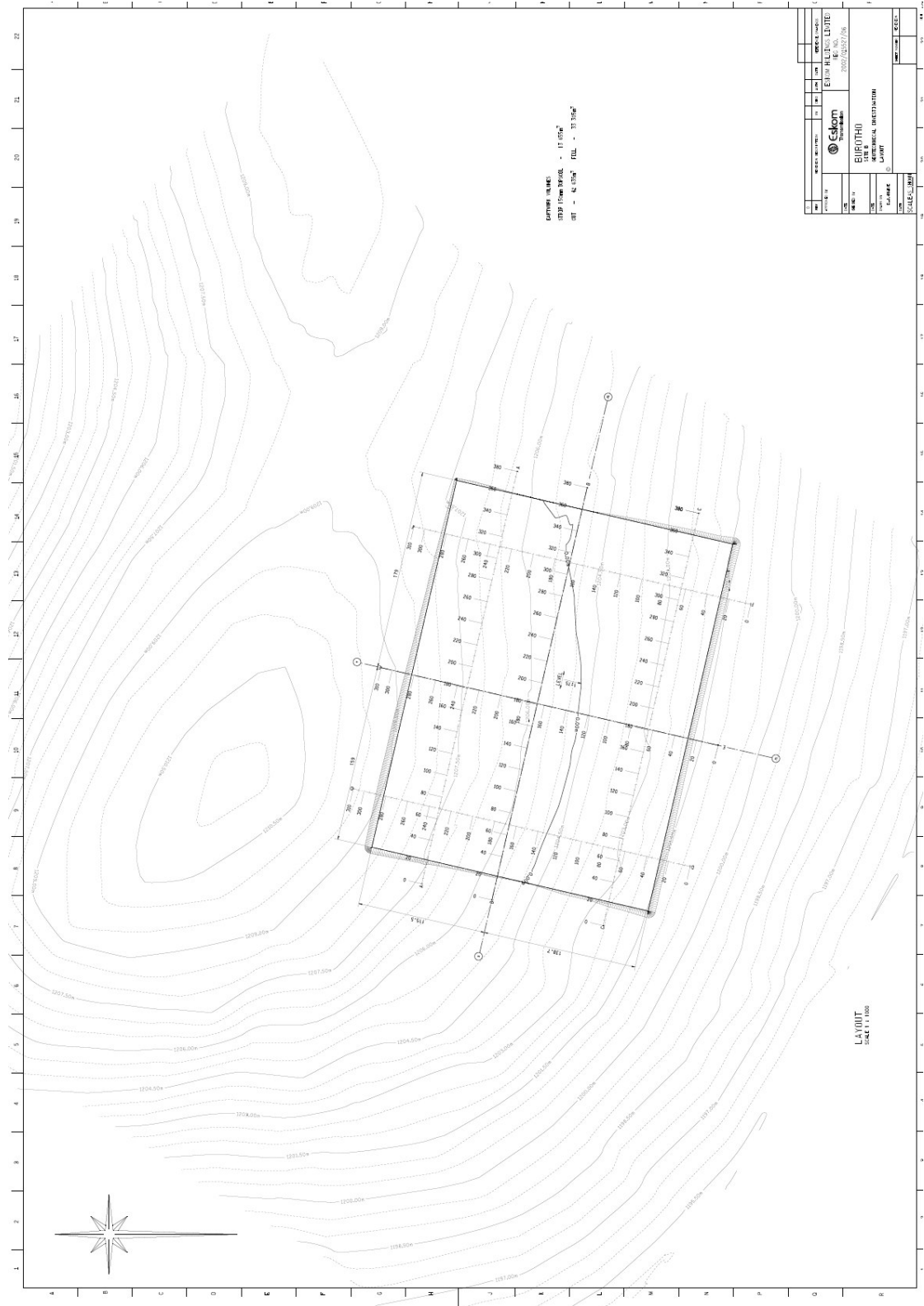
SECTION A - A
 SCALE: HORIZ. 1"=20' VERT. 1"=4'
 DATE: 02/20/2020

STATION	EXISTING GROUND ELEV.	FINISHED GROUND ELEV.	FINISH
0+00	1000.00	1000.00	
0+10	1000.00	1000.00	
0+20	1000.00	1000.00	
0+30	1000.00	1000.00	
0+40	1000.00	1000.00	
0+50	1000.00	1000.00	
0+60	1000.00	1000.00	
0+70	1000.00	1000.00	
0+80	1000.00	1000.00	
0+90	1000.00	1000.00	
1+00	1000.00	1000.00	
1+10	1000.00	1000.00	
1+20	1000.00	1000.00	
1+30	1000.00	1000.00	
1+40	1000.00	1000.00	
1+50	1000.00	1000.00	
1+60	1000.00	1000.00	
1+70	1000.00	1000.00	
1+80	1000.00	1000.00	
1+90	1000.00	1000.00	
2+00	1000.00	1000.00	
2+10	1000.00	1000.00	
2+20	1000.00	1000.00	
2+30	1000.00	1000.00	
2+40	1000.00	1000.00	
2+50	1000.00	1000.00	
2+60	1000.00	1000.00	
2+70	1000.00	1000.00	
2+80	1000.00	1000.00	
2+90	1000.00	1000.00	
3+00	1000.00	1000.00	
3+10	1000.00	1000.00	
3+20	1000.00	1000.00	
3+30	1000.00	1000.00	
3+40	1000.00	1000.00	
3+50	1000.00	1000.00	
3+60	1000.00	1000.00	
3+70	1000.00	1000.00	
3+80	1000.00	1000.00	
3+90	1000.00	1000.00	
4+00	1000.00	1000.00	

SECTION B - B
 SCALE: HORIZ. 1"=20' VERT. 1"=4'
 DATE: 02/20/2020

STATION	EXISTING GROUND ELEV.	FINISHED GROUND ELEV.	FINISH
0+00	1000.00	1000.00	
0+10	1000.00	1000.00	
0+20	1000.00	1000.00	
0+30	1000.00	1000.00	
0+40	1000.00	1000.00	
0+50	1000.00	1000.00	
0+60	1000.00	1000.00	
0+70	1000.00	1000.00	
0+80	1000.00	1000.00	
0+90	1000.00	1000.00	
1+00	1000.00	1000.00	
1+10	1000.00	1000.00	
1+20	1000.00	1000.00	
1+30	1000.00	1000.00	
1+40	1000.00	1000.00	
1+50	1000.00	1000.00	
1+60	1000.00	1000.00	
1+70	1000.00	1000.00	
1+80	1000.00	1000.00	
1+90	1000.00	1000.00	
2+00	1000.00	1000.00	
2+10	1000.00	1000.00	
2+20	1000.00	1000.00	
2+30	1000.00	1000.00	
2+40	1000.00	1000.00	
2+50	1000.00	1000.00	
2+60	1000.00	1000.00	
2+70	1000.00	1000.00	
2+80	1000.00	1000.00	
2+90	1000.00	1000.00	
3+00	1000.00	1000.00	
3+10	1000.00	1000.00	
3+20	1000.00	1000.00	
3+30	1000.00	1000.00	
3+40	1000.00	1000.00	
3+50	1000.00	1000.00	
3+60	1000.00	1000.00	
3+70	1000.00	1000.00	
3+80	1000.00	1000.00	
3+90	1000.00	1000.00	
4+00	1000.00	1000.00	

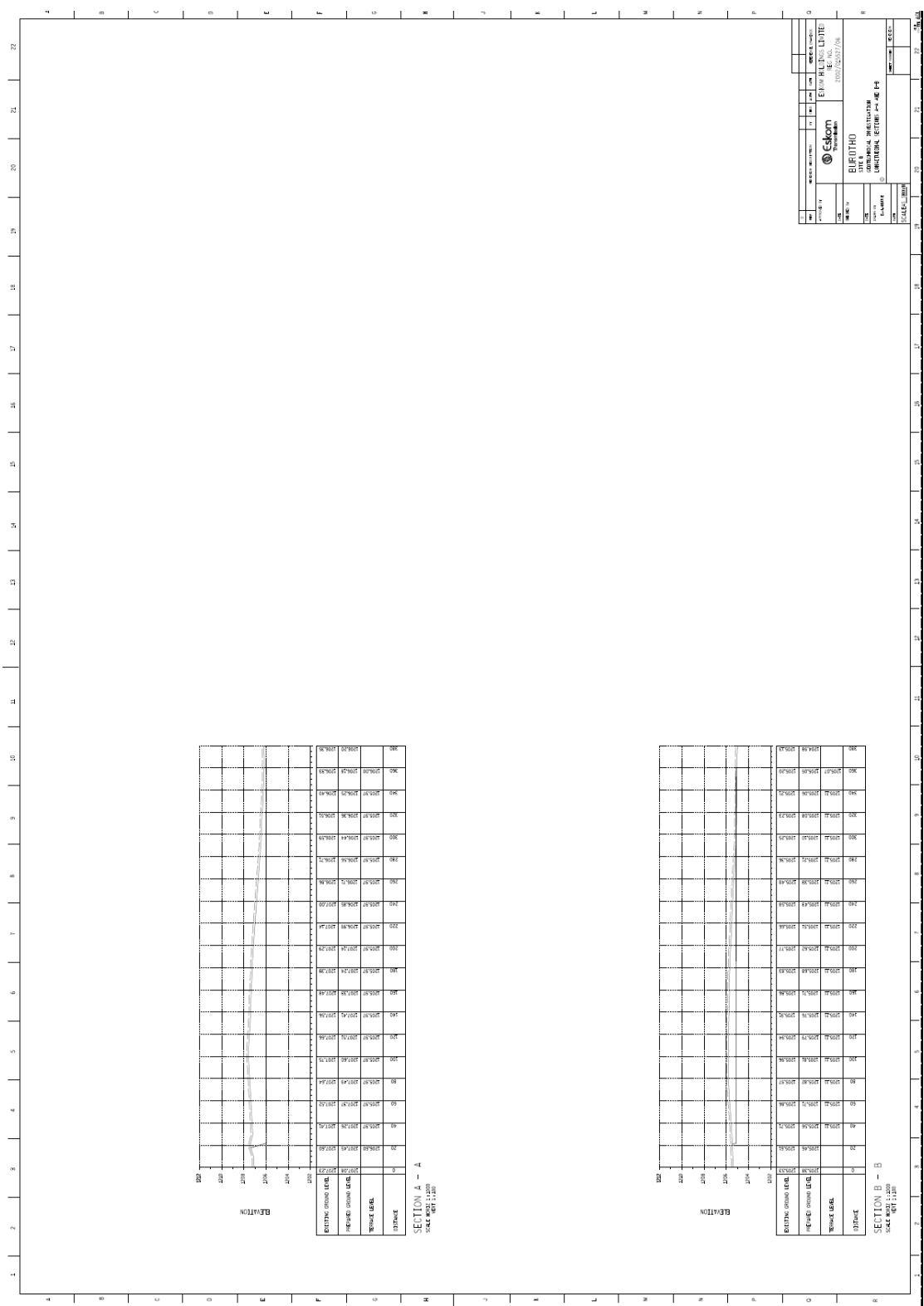
PROJECT NO.	10000000	DATE	02/20/2020
DRAWN BY	J. L. L...	CHECKED BY	J. L. L...
SCALE	HORIZ. 1"=20' VERT. 1"=4'	PROJECT	ROADWAY
		EVANS & LLOYD CONSULTING ENGINEERS 2007/24827736	
BUROTHO INC. 10000000		PROJECT NO. 10000000 DATE 02/20/2020	



CURVE RADIUS
 STIP FROM SURFACE - 11.00m
 CUT - 4.00m FILL - 31.50m

NO.	REVISION	DATE	BY	CHECKED
1				
Colson CONSULTANTS 6100 HILLVIEW DRIVE SUITE 100 RICHMOND, BC V6X 4R7 TEL: (604) 273-8888 FAX: (604) 273-8889 WWW.COLSONCONSULTANTS.COM				
PROJECT 6100 HILLVIEW DRIVE CIVIL ENGINEERING LAYOUT				
DATE 2023/12/27/26				
SCALE 1:1000				

LAYOUT
 SCALE 1:1000



SECTION A - A
SCALE: HORIZONTAL 1" = 100' VERTICAL 1" = 10'

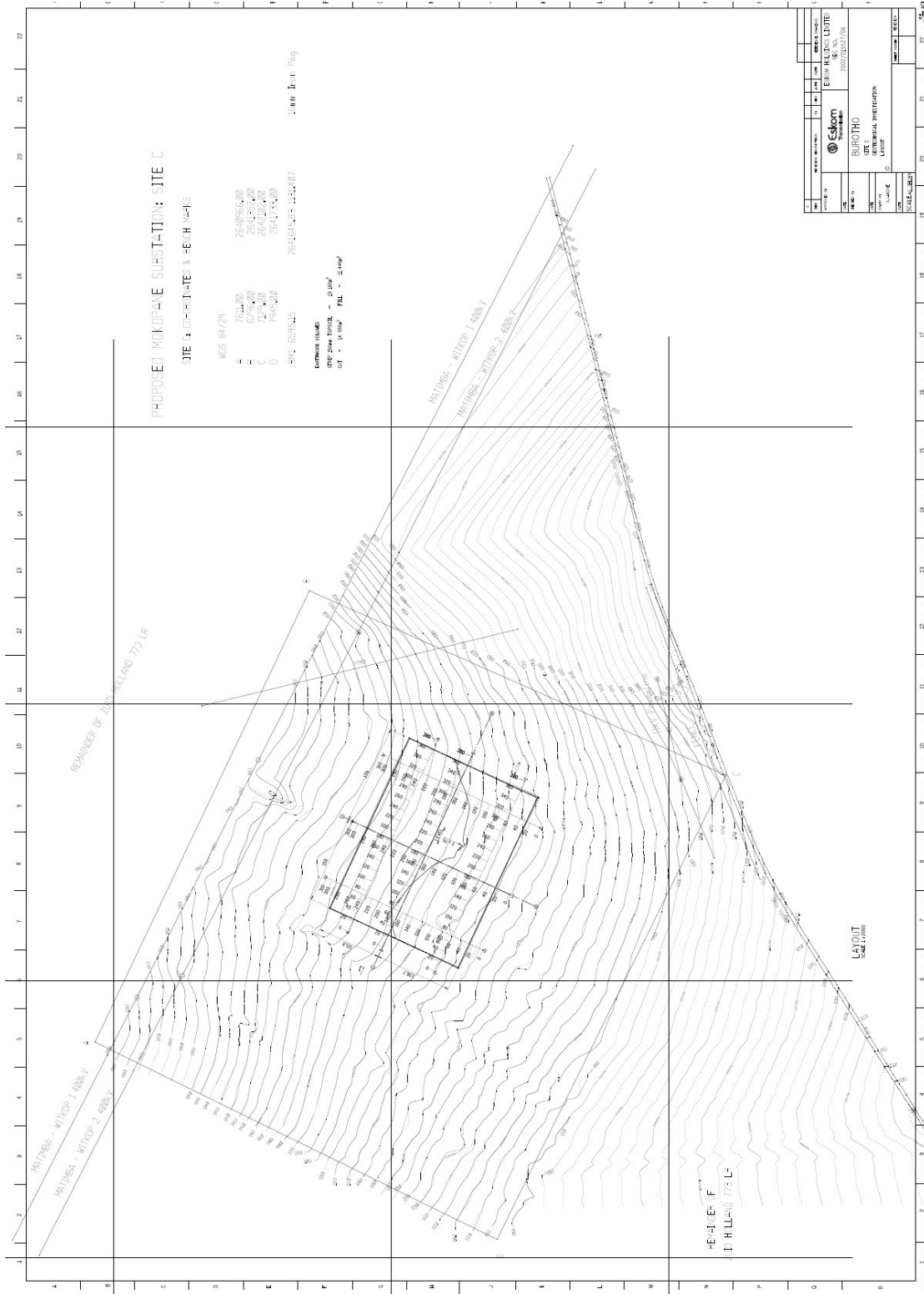
STATION	EXISTING GROUND ELEVATION	PROPOSED GRADE ELEVATION	GROUND LEVEL ELEVATION
1+00	203.70	203.70	203.70
2+00	203.60	203.60	203.60
3+00	203.50	203.50	203.50
4+00	203.40	203.40	203.40
5+00	203.30	203.30	203.30
6+00	203.20	203.20	203.20
7+00	203.10	203.10	203.10
8+00	203.00	203.00	203.00
9+00	202.90	202.90	202.90
10+00	202.80	202.80	202.80
11+00	202.70	202.70	202.70
12+00	202.60	202.60	202.60
13+00	202.50	202.50	202.50
14+00	202.40	202.40	202.40
15+00	202.30	202.30	202.30
16+00	202.20	202.20	202.20
17+00	202.10	202.10	202.10
18+00	202.00	202.00	202.00
19+00	201.90	201.90	201.90
20+00	201.80	201.80	201.80
21+00	201.70	201.70	201.70
22+00	201.60	201.60	201.60

SECTION B - B
SCALE: HORIZONTAL 1" = 100' VERTICAL 1" = 10'

STATION	EXISTING GROUND ELEVATION	PROPOSED GRADE ELEVATION	GROUND LEVEL ELEVATION
0+00	203.70	203.70	203.70
1+00	203.60	203.60	203.60
2+00	203.50	203.50	203.50
3+00	203.40	203.40	203.40
4+00	203.30	203.30	203.30
5+00	203.20	203.20	203.20
6+00	203.10	203.10	203.10
7+00	203.00	203.00	203.00
8+00	202.90	202.90	202.90
9+00	202.80	202.80	202.80
10+00	202.70	202.70	202.70
11+00	202.60	202.60	202.60
12+00	202.50	202.50	202.50
13+00	202.40	202.40	202.40
14+00	202.30	202.30	202.30
15+00	202.20	202.20	202.20
16+00	202.10	202.10	202.10
17+00	202.00	202.00	202.00
18+00	201.90	201.90	201.90
19+00	201.80	201.80	201.80
20+00	201.70	201.70	201.70
21+00	201.60	201.60	201.60
22+00	201.50	201.50	201.50

PROJECT NO.	1000000000	DATE	01/20/2024
PROJECT NAME	BRIDGE RECONSTRUCTION	SCALE	AS SHOWN
CLIENT	STATE OF CALIFORNIA	DRAWN BY	JK
DESIGNED BY	JK	CHECKED BY	JK
APPROVED BY	JK	DATE	01/20/2024

Caltrans
CALIFORNIA HIGHWAY PATROL
CALIFORNIA DEPARTMENT OF TRANSPORTATION



PROPOSED MOKOPANE SUBSTATION: SITE C

SITE: 00-011-01-TE & ECH 1-6-15

MCS 84/29

A	753.00	2640966.00
B	675.00	2641514.00
C	727.00	2642302.00
D	794.00	2641352.00
RM	6595.00	2640654.00

CONTAIN VOLUME
 270 200 2000L - 2.100 m³
 100 - 10 000 m³ FILL - 1.100 m³

John J. van Pong

NO.	DATE	BY	REVISION
1	15/01/2015	JJP	ISSUED FOR TENDER
2	15/01/2015	JJP	REVISED
3	15/01/2015	JJP	REVISED
4	15/01/2015	JJP	REVISED
5	15/01/2015	JJP	REVISED
6	15/01/2015	JJP	REVISED
7	15/01/2015	JJP	REVISED
8	15/01/2015	JJP	REVISED
9	15/01/2015	JJP	REVISED
10	15/01/2015	JJP	REVISED
11	15/01/2015	JJP	REVISED
12	15/01/2015	JJP	REVISED
13	15/01/2015	JJP	REVISED
14	15/01/2015	JJP	REVISED
15	15/01/2015	JJP	REVISED
16	15/01/2015	JJP	REVISED
17	15/01/2015	JJP	REVISED
18	15/01/2015	JJP	REVISED
19	15/01/2015	JJP	REVISED
20	15/01/2015	JJP	REVISED
21	15/01/2015	JJP	REVISED
22	15/01/2015	JJP	REVISED
23	15/01/2015	JJP	REVISED
24	15/01/2015	JJP	REVISED
25	15/01/2015	JJP	REVISED
26	15/01/2015	JJP	REVISED
27	15/01/2015	JJP	REVISED

ESKON

 ENGINEERING & CONSTRUCTION

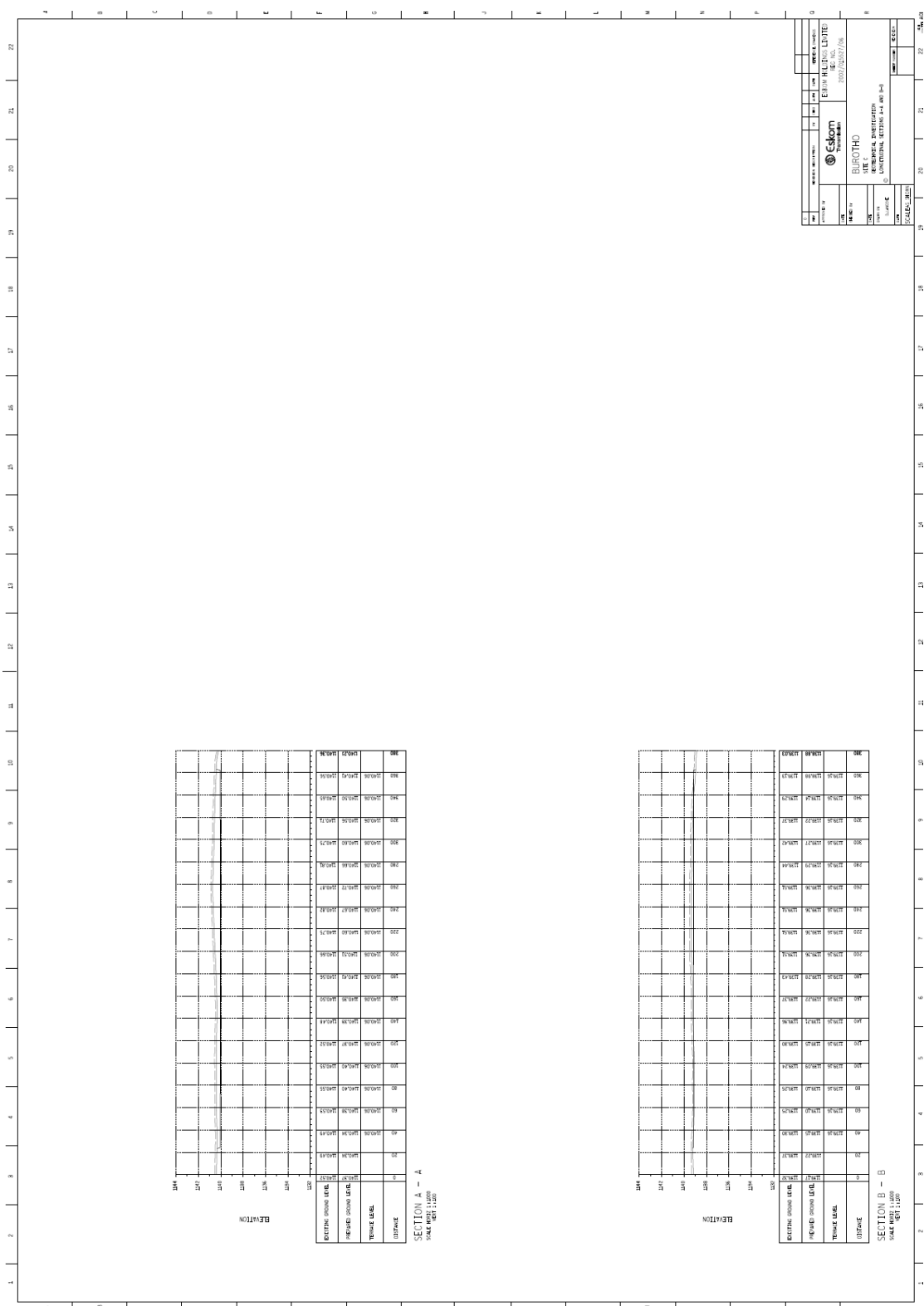
BURCHIO

 ELECTRICAL ENGINEERING

PROJECT NO: 15/01/2015/01/01

DRAWING NO: 15/01/2015/01/01/01

SCALE: 1:1000



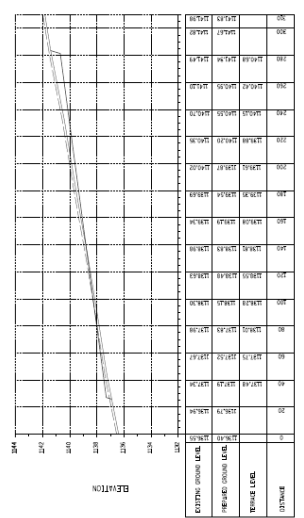
SECTION A - A
SCALE HORIZ: 1"=200'
SCALE VERT: 1"=20'

STATION	EXISTING GROUND ELEVATION	PROPOSED GROUND ELEVATION	FINISH ELEVATION
1	1.00	1.00	1.00
2	2.00	2.00	2.00
3	3.00	3.00	3.00
4	4.00	4.00	4.00
5	5.00	5.00	5.00
6	6.00	6.00	6.00
7	7.00	7.00	7.00
8	8.00	8.00	8.00
9	9.00	9.00	9.00
10	10.00	10.00	10.00
11	11.00	11.00	11.00
12	12.00	12.00	12.00
13	13.00	13.00	13.00
14	14.00	14.00	14.00
15	15.00	15.00	15.00
16	16.00	16.00	16.00
17	17.00	17.00	17.00
18	18.00	18.00	18.00
19	19.00	19.00	19.00
20	20.00	20.00	20.00
21	21.00	21.00	21.00
22	22.00	22.00	22.00

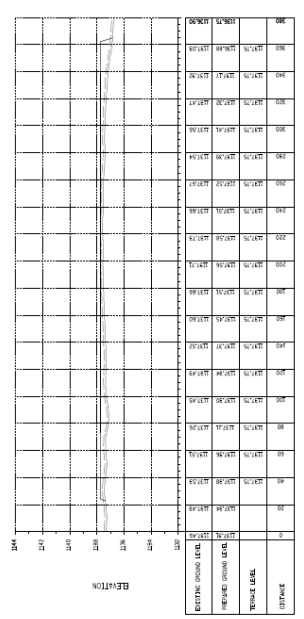
SECTION B - B
SCALE HORIZ: 1"=200'
SCALE VERT: 1"=20'

STATION	EXISTING GROUND ELEVATION	PROPOSED GROUND ELEVATION	FINISH ELEVATION
1	1.00	1.00	1.00
2	2.00	2.00	2.00
3	3.00	3.00	3.00
4	4.00	4.00	4.00
5	5.00	5.00	5.00
6	6.00	6.00	6.00
7	7.00	7.00	7.00
8	8.00	8.00	8.00
9	9.00	9.00	9.00
10	10.00	10.00	10.00
11	11.00	11.00	11.00
12	12.00	12.00	12.00
13	13.00	13.00	13.00
14	14.00	14.00	14.00
15	15.00	15.00	15.00
16	16.00	16.00	16.00
17	17.00	17.00	17.00
18	18.00	18.00	18.00
19	19.00	19.00	19.00
20	20.00	20.00	20.00
21	21.00	21.00	21.00
22	22.00	22.00	22.00

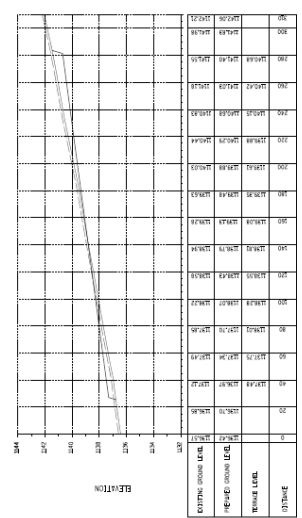
DATE	2024/02/28	PROJECT	ERLON ALLEY LANE
DRAWN BY		CHECKED BY	
SCALE		DATE	
BIRLOTHO INC. 100 INDUSTRIAL PARKWAY #400 CHICAGO, ILLINOIS 60642			
PROJECT NO. 2024/02/28			
SHEET NO. 1 OF 1			



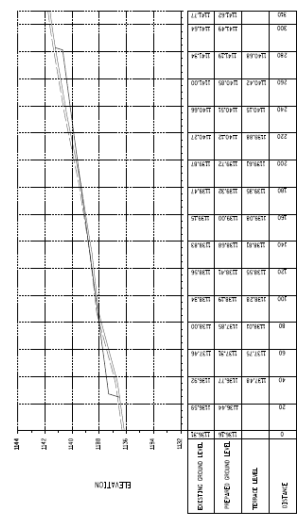
SECTION E - E
SCALE: HORIZ. 1:1000
VERT. 1:100



SECTION C - C
SCALE: HORIZ. 1:1000
VERT. 1:100



SECTION F - F
SCALE: HORIZ. 1:1000
VERT. 1:100



SECTION D - D
SCALE: HORIZ. 1:1000
VERT. 1:100

PROJECT NO.	11	DATE	2022/02/27/20
SCALE	1:1000	DESIGNER	CSKORN
DATE	2022/02/27/20	CLIENT	BLUOTHO
PROJECT NAME	RECONSTRUCTION OF THE RAILWAY TRACKS AND PLATFORMS		
PROJECT LOCATION	LUNDAK, KUALA LUMPUR, MALAYSIA		
PROJECT NO.	11	DATE	2022/02/27/20
SCALE	1:1000	DESIGNER	CSKORN
DATE	2022/02/27/20	CLIENT	BLUOTHO
PROJECT NAME	RECONSTRUCTION OF THE RAILWAY TRACKS AND PLATFORMS		
PROJECT LOCATION	LUNDAK, KUALA LUMPUR, MALAYSIA		