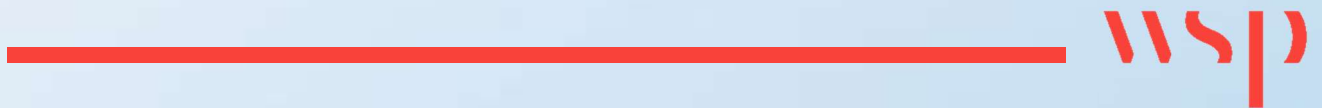


Appendix F.1B

PRELIMINARY GEOTECHNICAL SCOPE





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FINAL REPORT

PRELIMINARY GEOTECHNICAL INVESTIGATIONS SCOPE & SPECIFICATION FOR THE PROPOSED KOMATI REPOWERING & REPURPOSING PROJECT

Report no.: BAV152.12.23 Status: **Final** Date: Feb 2024



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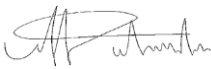
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EXECUTIVE SUMMARY

BAV Consulting Pty Ltd was appointed by **Eskom Holdings SOC Limited** to conduct a preliminary geotechnical investigation in order to determine and assess the subsurface soil conditions at selected sites for the renewable energy project. The sites of investigation are situated within and/or near Komati Power Station on Eskom owned land within Steve Tswhe Local Municipality.

The proposed Eskom sites are situated adjacent to several operational coal mines currently exploited for coal seams in excess of 45 meters by either bord and pillar or open-pit mining. The Blinkpan underground operation located just west of the Komati area which utilises a mechanised bord-and-pillar underground mining method confirmed that there's no past or on-going mining operation underneath the proposed site.

Komati Village and its neighbouring areas are regionally characterized by rocks of the Karoo Supergroup covered by Quaternary deposits. The Karoo Supergroup rocks in the area can be further classified as the Vryheid Formation of the Ecca Group. The rocks of the Vryheid Formation typically include sandstone, siltstone, shale and coal beds. These were later intruded by Jurassic dolerite dykes and sills.

There are no soluble rocks such as dolomite or limestone underlying the sites, therefore, the presence of karst-related subsurface topography leading to the formation of sinkholes and subsidence features is unlikely. However, the area is known to have been previously undermined, therefore further studies should be undertaken to assess possible land-subsidence events resulting from the mobilisation of overburden material into underlying abandoned mines.

Additional percussion drilling is recommended at Solar PV Site 1 and 2 in order to accurately confirm and delineate areas affected by previous mining activities. Furthermore, rotary core drilling and associated strength tests such as SPT, UCS, pile tests, etc. are recommended at the footprint of the proposed structures.

Conditions prevailing on site suggest that no problems are foreseen for the development of agrivoltaics, wind turbines and a BESS system, provided that the contents of this report are acknowledged, and recommendations, as outlined in the report, are adhered to. Note that heavy loaded structures such as the proposed wind turbines are not recommended at Solar PV Site 1 due to the presence of cavities that can manifest into subsidence or sinkholes triggered by continued weathered overburden material, groundwater level fluctuations, and collapse of underground mine pillars.

1 INTRODUCTION

1.1 Terms of Reference

Komati Power Station is planning to develop and implement a renewable energy project through the use of Solar PV Agrivoltaics, Wind turbines and a BESS system to generate and store power; thus, a geotechnical investigation is required to determine the subsurface soil conditions.

The proposed Solar PV plant, Battery Storages and Wind turbines for Komati Power Station is located within the boundary of Eskom-owned land. The area is in Mpumalanga Province between Middleburg and Bethal. Suitable areas for renewable energy project were identified considering the wetlands, ash dams, existing underground and above ground services (electrical cables and overhead lines).

BAV Consulting Pty Ltd was appointed by **Eskom Holdings SOC Limited** to conduct a preliminary geotechnical investigation in order to determine and assess the subsurface soil conditions at selected sites for the renewable energy project. The sites of investigation are situated within and/or near Komati Power Station on Eskom owned land within Steve Tshwete Local Municipality, Nkangala District Municipality, Mpumalanga Province.

1.2 Proposed Developments

Eskom Generation is planning to implement the BESS in phase 1 of solar PV at the Komati Power Station in Mpumalanga. Suitable areas for renewable energy project were identified considering the wetlands, ash dams, existing underground and above ground services (electrical cables and overhead lines). The proposed development would include the installation of the following typical equipment:

- Solar PV
- Battery Storage (BESS Unit)
- Wind Turbines
- Power Transformers
- Electrical Transformer
- Shunt Reactors
- Low Voltage Switchgear
- Instrument Transformers
- Surge Arrestors
- Control and Ancillary Buildings

1.3 Scope of Work

The geotechnical investigation included the following key components:

- Health, Safety and Environment
- Desktop studies to review of existing regional, site and surface information.
- Services detection by means of Ground Penetration Radar (GPR)
- Resistivity survey for near-surface soil conductivity
- Excavation of test pits, soil profiling and sampling
- Dynamic probing by DPSH Testing
- Geotechnical core drilling including SPT tests at intervals of 1.5 m
- Investigate potential undermined ground at Solar PV site 1 by percussion drilling
- Laboratory testing by a SANAS accredited civil engineering laboratory
- Prepare a factual report
- Compile an interpretive report

1.4 Report Provisions and Exclusions

This report is specifically suitable for use in preliminary design purposes for structures imposing pressures equal to or less than 300 kPa and the planning of additional geotechnical investigations. It is meant solely for use in the above manner. Other infrastructure and service developments on this site, such as high-rise buildings, bridges, underground works etc. fall outside the scope of this report.

Any form of development, outside the boundaries of the investigated areas as per the attached site layout plan, is not covered by this report.

1.5 Approach

The approach in respect of the fieldwork phase is in conformance with the Site Investigation Code of Practice, published by The Geotechnical Division of the South African Institution of Civil Engineering (SAICE), 2008. A systematic approach was followed consisting of:

- Desktop studies and information gathering
- Site visit, area identification and field reconnaissance survey
- Geophysical surveys including GPR and resistivity survey
- Excavation of test pits, soil profiling and sampling
- In-situ testing such as DCP and DPSH tests
- Percussion drilling and sampling
- Core drilling including SPT tests

1.6 Sources of Information

The following were studied prior to the investigation taking place:

- 1: 250 000 Geological map sheet 2628 East Rand, Copyright Geological Survey of South Africa (Council for Geosciences)
- **SANS 633**: Soil profiling and rotary percussion borehole logging on dolomite land in Southern Africa for engineering purposes
- **Site Investigation Code of Practice**, 1st Edition, South African Institution of Civil Engineering - Geotechnical Division, January, 2010.
- **Eskom 2023**: Preliminary geotechnical investigations scope & specification for the proposed Komati Repowering & Repurposing Project
- Satellite Imagery (Esri Satellite Imagery/Google earth, 2023)

2 SITE LOCATION AND DESCRIPTION

2.1 Geographic Description

The sites for the proposed development of agrivoltaics, wind turbines and a BESS system are located on portions 10, 11 and the remainder of the farm Komati Power Station 56 IS, near Komati Village. Komati village is situated in between the towns of Middelburg and Bethal within the Steve Tshwete Local Municipality of the Nkangala District, Mpumalanga Province (refer **Figure 2-3**).

2.2 Topographic and Drainage

The area is generally characterised by near-horizontal natural slopes inclined predominantly towards the north. The average slope gradient ranges between 1.2 to 2.0%, hence it is considered to be naturally stable as far as slope instability is concerned. This topographic assessment was based on Google Earth sourced elevations in conjunction with site walk-over survey. It is worth noting that the topography heights are only for relative height differences and cannot be concluded as standard survey heights. **Figure 2-2** below shows the general aerial view of the area.

Surface run-off from the Eskom sites is generally by means of sheetwash towards the north and ultimately into northwest flowing streams traversing the Komati the region. It is the author's view that flood-lines may affect certain sections of the sites, particularly areas adjacent to streams and wetlands. Calculated flood lines for all nearby watercourses and streams should be available from the Local authority's town planning department or landowner. If not, a hydrologist should undertake this determination.

2.3 Climate and Weathering

The climate of Komati is similar to that of Middelburg. It is classified as subtropical highland climate (Cwb) according to Köppen and Geiger classification and the climate is characterised by long, warm and cloudy summers, while the winters are short, cold and clear. Majority of rain falls within the summer months from October until March. The mean precipitation for the area is approximately 650 mm per year. The long-term average high temperatures are in December with an average high of 28°C. The coldest and driest month is June with an average temperature of 8°C and approximately 5 mm precipitation, while the wettest month is January with approximately 170 mm of rainfall.

The site falls within an area with a climatic N-value of between 2 and 5 (Weinert, 1980). This simply means that chemical weathering processes predominate over mechanical weathering, thus the formation of thick layers of residual soil is anticipated. According to Thornthwaite's moisture index, the area is between -20 and 0, which indicates a sub-humid environment (Schultze, 1958).

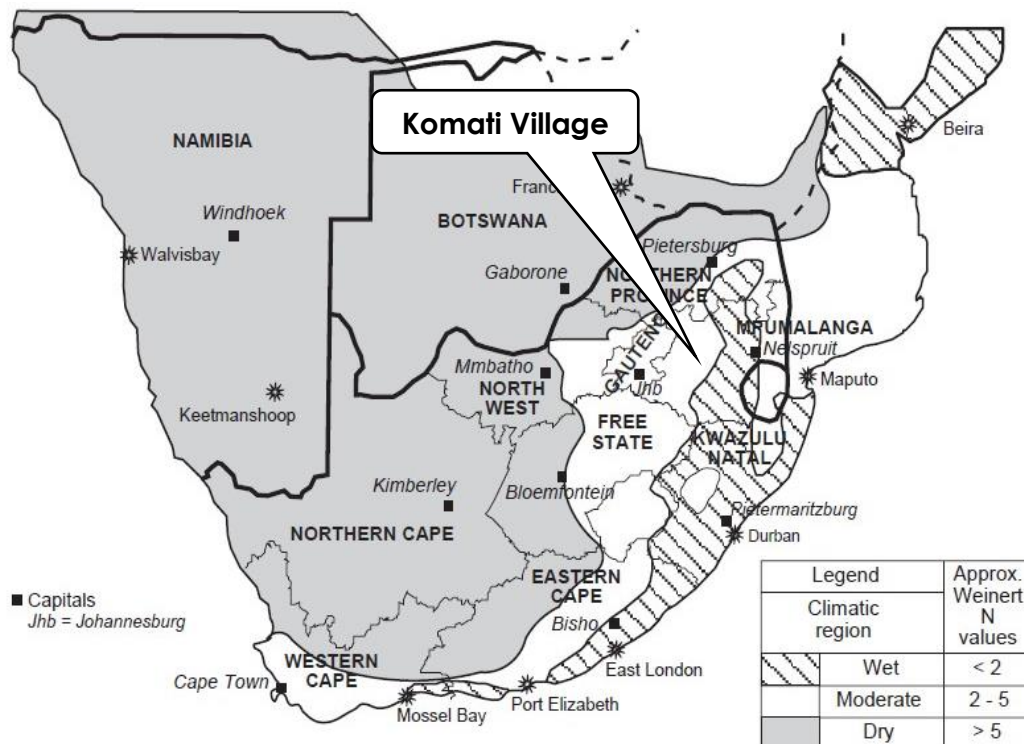


Figure 2-1: Macro Climatic Regions of Southern Africa (Adapt from Weinert, 1980)

2.4 Vegetation

According to Accocs (1980), Komati area falls under the Bankenveld vegetation type. Satellite images and site walkover shows that the area is mainly characterized by grass, weeds, and planted trees. It must be pointed out that this site has been interfered with by anthropogenic activities and therefore, the vegetation noted on site may not represent the genuine site conditions.

2.5 Land-Use

Land use across the project area is predominantly for farming and mining purposes. In terms of this study, it therefore, presents a brownfields type of study necessitating alterations and modification, particularly areas within the main power station.

2.6 Mining Activities

The proposed Eskom sites are situated adjacent to several operational coal mines currently exploited for coal seams in excess of 45 meters by either board and pillar or open-pit mining. The Blinkpan underground operation located just west of the Komati area which utilises a mechanised bord-and-pillar underground mining method confirmed that there's no past or on-going mining operation underneath the proposed site. No credible information was received from the Koorfontein operation located in the north. Nonetheless, the presence of undermined ground at the proposed site cannot be discounted with certainty. Therefore, further studies focusing on undermining are necessary.

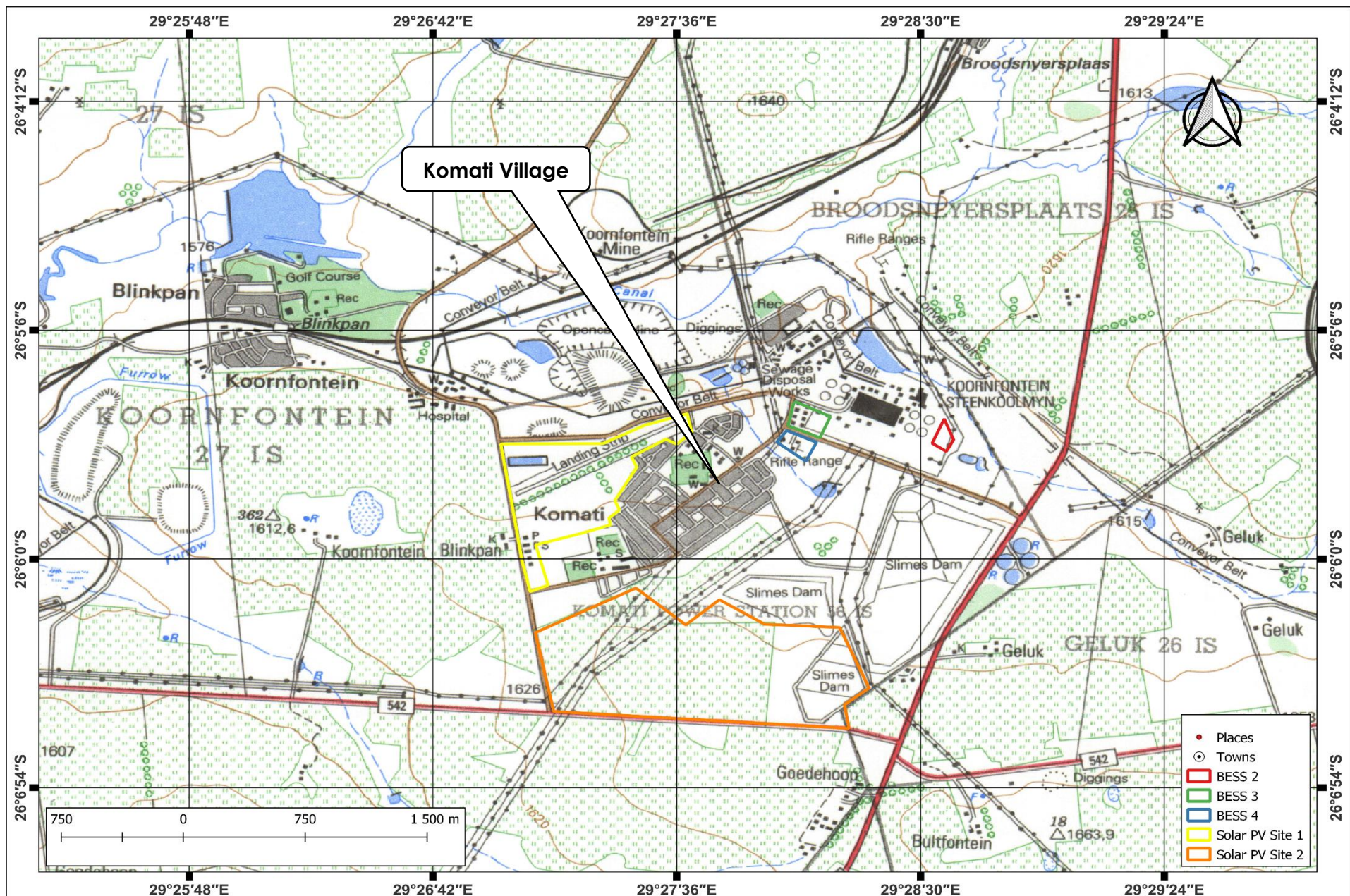
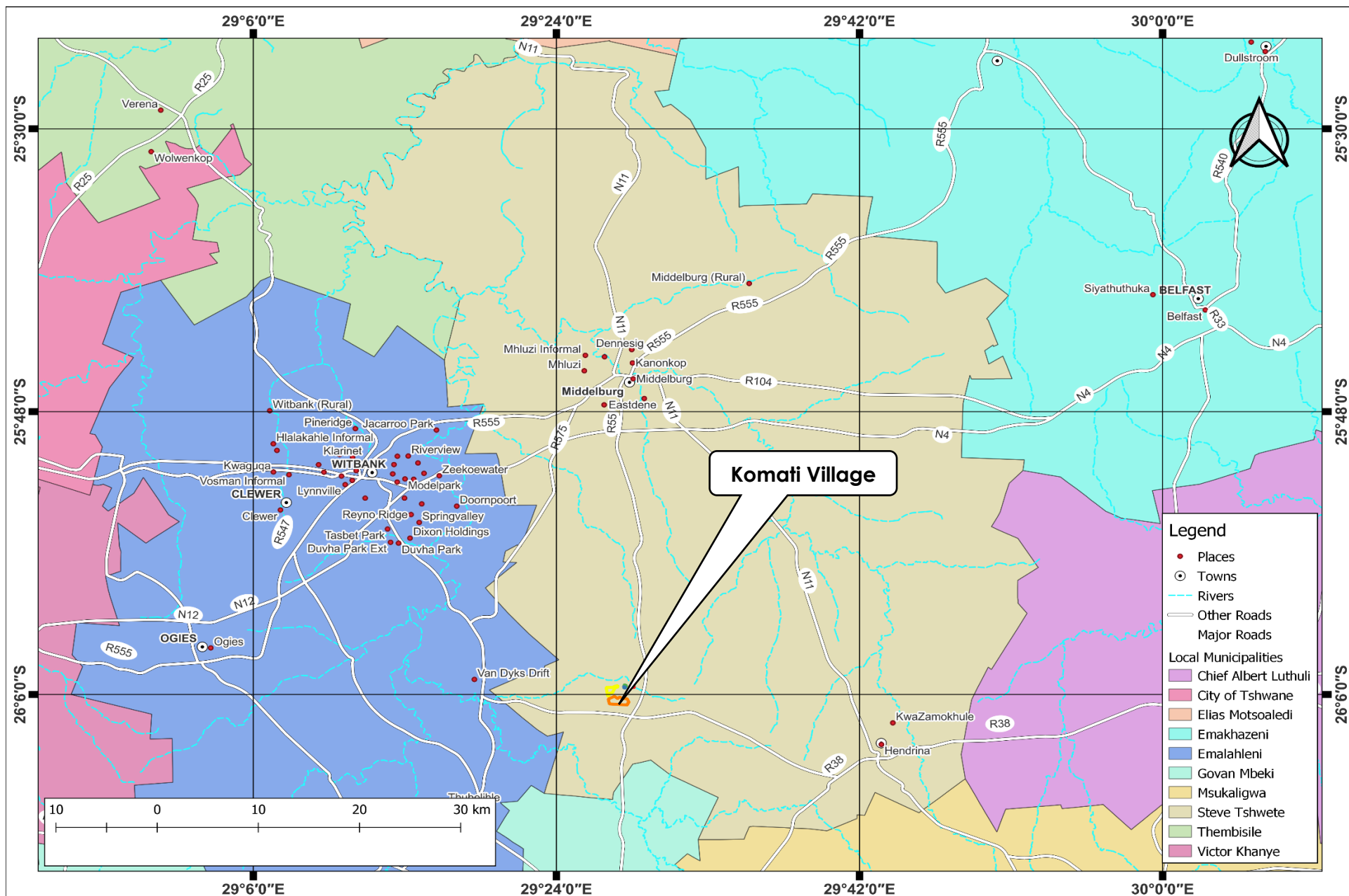


Figure 2-2: Topographic map showing the location of project sites in the vicinity of Komati Village.



3 GEOLOGY SETTING

3.1 Regional Geology

According to the 1:250 000-scale regional geological map sheet 2628 East Rand, Komati Village and its neighbouring areas are regionally characterized by rocks of the Karoo Supergroup covered by Quaternary deposits (refer **Figure 3-3**). The Karoo Supergroup rocks in the area can be further classified as the Vryheid Formation of the Ecca Group. The rocks of the Vryheid Formation typically include sandstone, siltstone, shale and coal beds. These were later intruded by Jurassic dolerite dykes and sills.

There are no soluble rocks such as dolomite or limestone underlying the sites, therefore, the presence of karst-related subsurface topography leading to the formation of sinkholes and subsidence features is unlikely. However, the area is known to have been previously undermined, therefore further studies should be undertaken to assess possible land-subsidence events resulting from the mobilisation of overburden material into underlying abandoned mines.

3.2 Local Geology

The local geological conditions of the Komati area may be interpreted from fieldwork results. The area is generally covered by transported soils, underlain by pedogenic material, residual soils and weathered sedimentary rocks of the Vryheid Formation, respectively (refer **Figure 3-2**).

- **Transported soils** - These are soils that have been transported by a natural agent (imported fill, wind-blown sand, colluvium, hillwash, etc.) during relatively recent geological times and which have not undergone lithification into sedimentary rocks or cementation into a pedogenic material.
- **Residual soils** – Soils derived from the weathering of the underlying rock and have not moved from the place of origin as with transported soils.
- **Weathered rock** – Rock that is/has been altered from its original state through the process of weathering by natural agents.
- **Pedogenic rock** – Weathered rock that is/has been cemented by chemical agent(s).

3.3 Geological Structures

As could further be seen on the geological map and satellite images, there are no geological structures such faults traversing the sites. However, a northeast-southwest trending lineament, possibly a dyke is indicated on the regional geological map. Furthermore, intrusive material interpreted as dolerite was encountered in some of the boreholes drilled as shown in **Figure 3-2** below.

3.4 Regional Geohydrology

The site is located within the Upper Olifants Groundwater Management Unit (GMU) in the B12 Quaternary Catchment Region. According to the regional hydrogeological map Sheet 2526 Johannesburg at a scale of 1: 500 000 published by the Department of Water Affairs (DWA, 1998), the inferred geohydrology of the site is characterized by an intergranular and fractured aquifer system with predominant arenaceous rocks. This aquifer type is characterised by deep weathering of sedimentary rocks imparting intergranular properties of the weathered zones.

The average borehole yield is expected to range from 0.1 to 0.5 l/s indicating that the site is located within a moderate yielding aquifer. The groundwater quality from the contoured geohydrological map suggest that the Electrical Conductivity (EC) range from 0 to 70 mS/m. The mean annual precipitation of the site area ranges from 600 to 800 mm while the groundwater level is expected to range between 10 and 50 mbgl.

Figure 3-1 below shows the local groundwater levels obtained from percussion drilling at Solar PV Site 1.

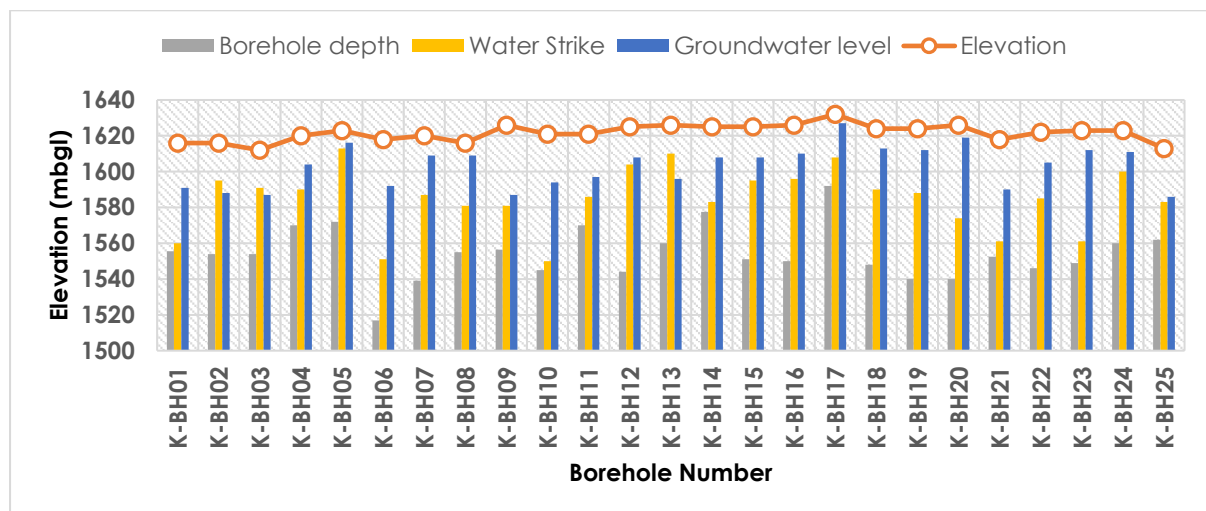


Figure 3-1: Groundwater conditions obtained from boreholes drilled on site.

Table 3-1: The geology of Komati and neighbouring areas

Era/Period	Stratigraphy	Group	Formation	Lithology
Quaternary	-	-	-	(Q) Alluvium
Jurassic	-	-	-	(Jd) Dolerite
Permian	Karoo Supergroup	Ecca Group	Vryheid Formation	(Pv) Sandstone, shale, coal beds
Vaalian	-	-	-	(Vdi) Diabase
	Transvaal Supergroup	Pretoria Group	Selons River Formation	Porphyritic rhyolite, with interbedded mudstone & sandstone



Transported soil, underlain by residual soil and weathered sandstone



Highly weathered sandstone



Highly weathered sandstone



Weathered sandstone with dolerite intrusive



Slightly weathered dolerite

Figure 3-2: The general geology of the sites interpreted from BH6.

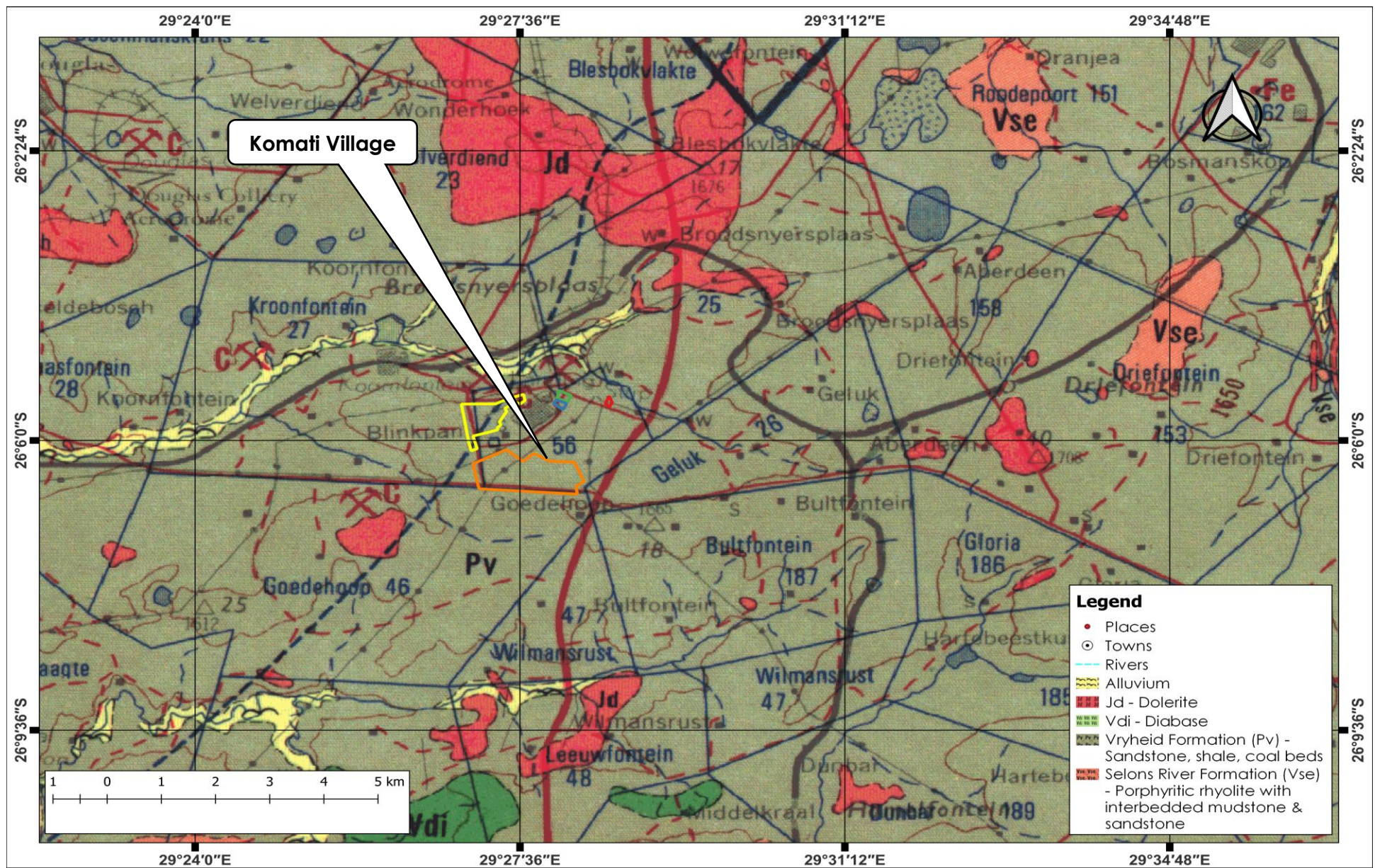


Figure 3-3: Map showing the regional geology of Komati and surrounding areas.

4 GEOPHYSICAL SURVEY

4.1 Resistivity Method

Soil resistivity surveys were carried out in accordance with the 240-96393507: Soil Resistivity Testing Guidelines provided by Eskom as part of the scope. The resistivity survey was undertaken by the ABEM Terrameter SAS1000 system using the Werner protocol (refer **Figure 4-1**). The apparent resistivity data acquired in the field were inverted using the RES2DINV software to provide a true-depth resistivity section.

The resistivity method was used to locate lateral and vertical changes in electrical properties of the sub-surface that may be related to changes in formation properties. The resistivity tomography method provides pseudo-sections of change in electrical properties in the subsurface along a specified line.

The resistivity of a material describes how difficult it is for electrical current to flow through the material. It is a property of the material, independent of the shape (geometry) of the object consisting of the material. This method is extensively used in the exploration of groundwater resources and mineral exploration.

The following geophysical traverses were undertaken with ABEM Terrameter SAS1000 along nine (9) traverse lines at Solar PV Site 1, Solar PV Site 2 and Bess Unit 4 and summarised below. The inverted resistivity data profiles for traverse lines undertaken at the various sites are shown in the figures below. The resistivity values can be interpreted as follows:

- Dark blue to light blue contours is likely indicative of highly saturated sandy material.
- Green to yellow is likely indicative of highly weathered material above bedrock which comprises weathered sedimentary rocks.
- Red to purple or dark red is likely indicative of a fractured to competent bedrock.

The sites are generally blanketed by a laterally persistent layer of sandy clay/clayey sand with low resistivity values and inversely moderate to high conductivity as a result of the moisture content for the upper unconsolidated soils. This is underlain by highly weathered layers of siltstone interlayered with fine sandstone, carbonaceous shale, and shaly coal, all showing moderate to high resistivity values. The weathered layers above were possibly underlain and intruded by slightly weathered to unweathered sandstone and dolerite with high resistivity values. The figures below show resistivity models obtained from field measurements.



Figure 4-1: Geophysical survey for soil resistivity by means of ABEM terrameter SAS1000.

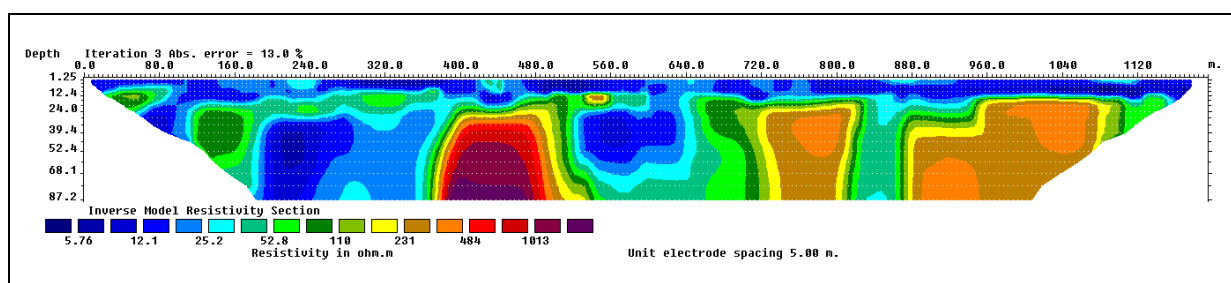


Figure 4-2: Resistivity survey profile at Solar PV Site 1 along a 1.2 km traverse line trending east-west (KOM Line 1).

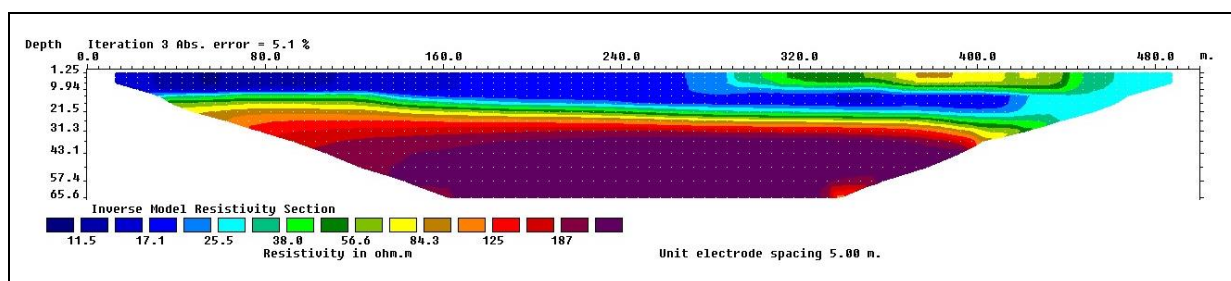


Figure 4-3: Resistivity survey profile at Solar PV Site 1 along a 500 m traverse line trending northwest-southeast (KOM Line 2).

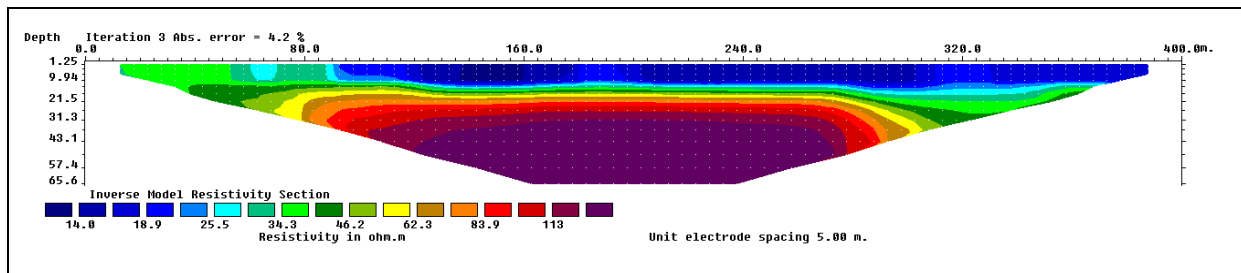


Figure 4-4: Resistivity survey profile at Solar PV Site 1 along a 400 m traverse line trending northwest-southeast (KOM Line 3).

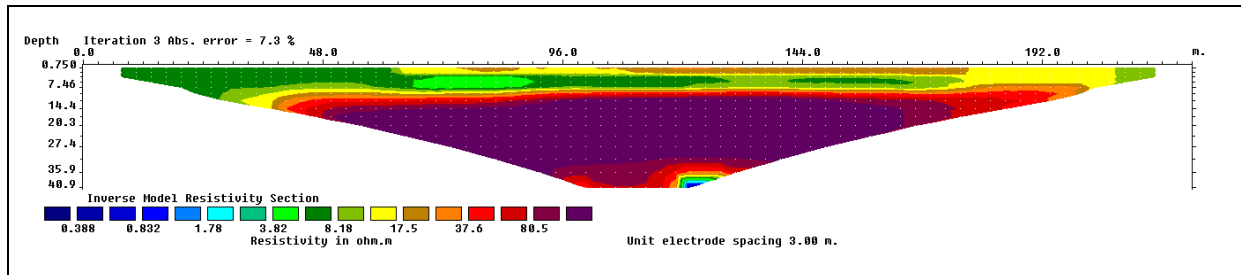


Figure 4-5: Resistivity survey profile at BESS 4 along a 215 m traverse line trending east-west (KOM Line 5).

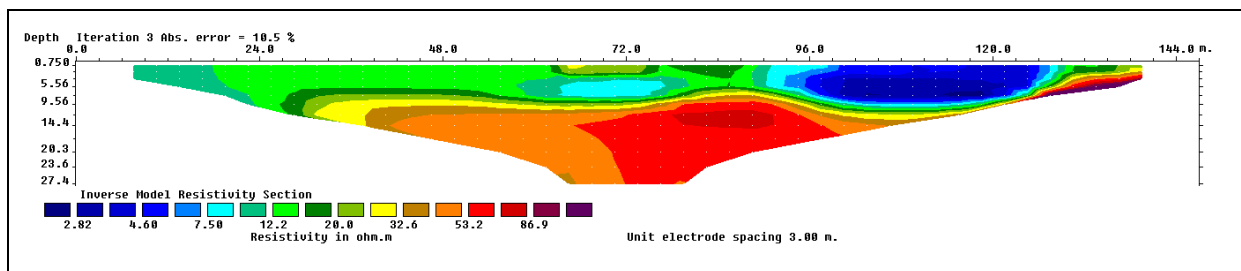


Figure 4-6 Resistivity survey profile at BESS 4 along a 144 m traverse line trending north-south (KOM Line 6).

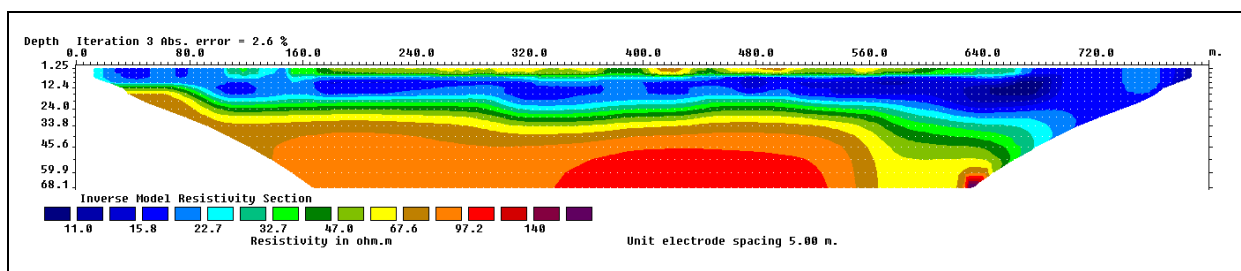


Figure 4-7: Resistivity survey profile at Solar PV Site 2 along an 800 m traverse line trending east-west (KOM Line 7).

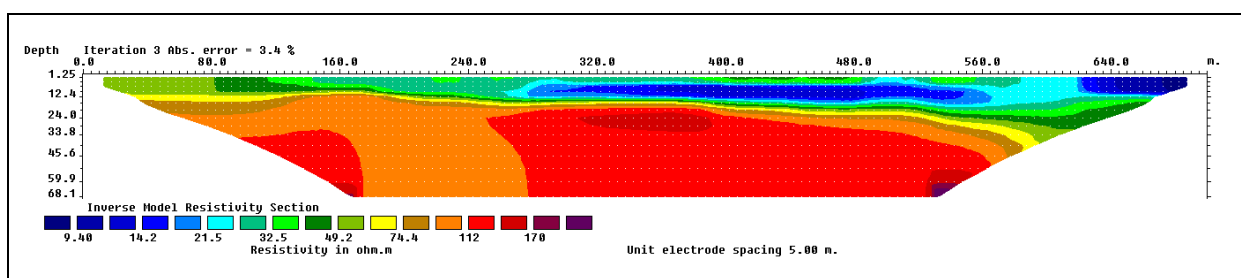


Figure 4-8: Resistivity survey profile at Solar PV Site 2 along a 700 m traverse line trending north-south (KOM Line 8).

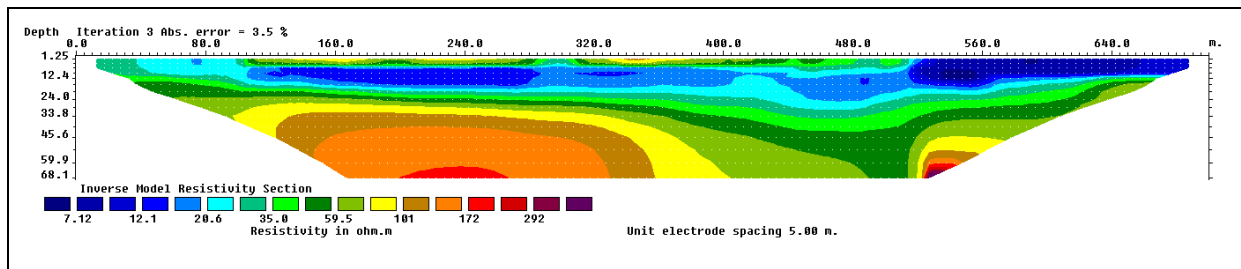


Figure 4-9: Resistivity survey profile at Solar PV Site 2 along a 700 m traverse line trending north-south (KOM Line 9).

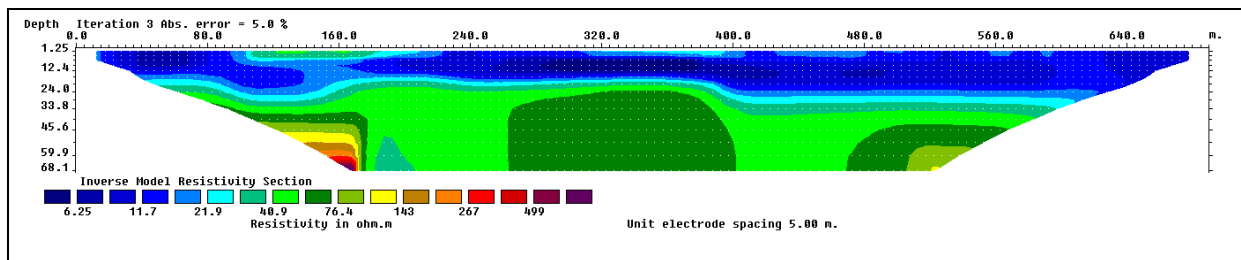


Figure 4-10: Resistivity survey profile at Solar PV Site 2 along a 690 m traverse line trending north-south (KOM Line 10).



Figure 4-11: Geophysical map showing resistivity survey lines

5 FIELDWORK RESULTS

5.1 Test Pitting Results

Fieldwork was conducted on the 22nd until 25th August 2023. A total of forty-seven (47) shallow test pits were excavated by means of a BELL TLB (named KTP1 to KTP47). Soil horizons in each of the pits were identified and described comprehensively applying the MCCSSO technique as advocated by Jennings *et al* (1973). The acronym – MCCSSO – stands for Moisture, Colour, Consistency, Structure, Texture and Origin. Note that due to erratic GPS satellite signal reception, the coordinates and elevations recorded will have an accuracy of only +/- 5 m. Disturbed soil samples were collected from the excavated test pits in the project area, for identification and analysis.

Their layout across the site is shown in **Figure 5-3**, while the detailed profile descriptions are presented in **APPENDIX A: SOIL PROFILES**.

The excavation of several test pits revealed material consistent with the regional geology of the area. The area is covered by transported soils underlain by pedogenic soils, residual soil and weathered sedimentary rocks, respectively. The profiles were encountered at the various site:

Soil Profile A: Solar PV Site 1

- Moist to very moist, brown, medium dense, intact, silty SAND. Hillwash.
- Moist to very moist, brown, medium dense, intact, clayey SAND with minor ferricrete. Residual soil.

Soil Profile B: Solar PV Site 2

- Fine to medium, sub-angular, closely packed GRAVEL of quartzite in a matrix of moist, brown, silty SAND. The overall consistency is very dense. Imported.
- Very moist, brown, medium dense, intact, clayey SAND. Hillwash.
- Moist to Very moist, orangey brown, firm to stiff, intact, CLAY. Hillwash.

Soil Profile C: BESS Unit 2

- Moist, brown, medium dense, intact, gravelly SAND. Hillwash.
- Moist to very moist, reddish brown, stiff, intact, clayey silty SAND. Hillwash.
- Moist to very moist, reddish brown, medium dense, intact, clayey silty SAND. Hillwash.

Soil Profile D: BESS Unit 3

- Moist to very moist, brown, medium dense, intact, clayey SAND with minor ferricrete. Residual soil.

Soil Profile E: BESS Unit 4

- Moist, brown, medium dense, intact, silty SAND. Hillwash.
- Moist to very moist, brown, medium dense, intact, clayey SAND with minor ferricrete. Residual soil.

Table 5-1: Summary of soil layers encountered in test pits

Site	Test Pit positions		Layer thickness (m)			Seepage
	Test Pit	Coordinates	Hillwash	Pedogenic	Residual	
Solar PV Site 1	KTP1	26.09103°S, 29.46031°E	0.0-0.7	-	0.7-2.2	-
	KTP2	26.09147°S, 29.45981°E	0.0-0.8	-	0.8-2.4	2.4 m
	KTP3	26.09211°S, 29.45872°E	0.0-0.4	-	0.4-1.8	-
	KTP4	26.09249°S, 29.45752°E	0.0-0.3	-	0.3-2.3	-
	KTP5	26.09342°S, 29.45654°E	0.0-0.4	-	0.4-2.5	-
	KTP6	26.09348°S, 29.45528°E	0.0-0.1	-	0.1-2.4	-
	KTP7	26.09448°S, 29.45366°E	-	0.0-1.9	-	-
	KTP8	22.09541°S, 29.45200°E	0.0-0.4	-	0.4-2.3	-
	KTP9	26.09517°S, 29.45013°E	0.0-0.5	1.0-1.5	0.5-1.0	-
	KTP10	26.09451°S, 29.45136°E	0.0-0.6	-	0.6-2.2	2.1 m
	KTP11	26.09395°S, 29.45266°E	0.0-0.7	-	0.7-2.4	-
	KTP12	26.09299°S, 29.45473°E	0.0-0.6	-	0.6-1.0	2.2 m
	KTP13	26.09207°S, 29.45693°E	0.0-0.3	-	0.3-1.8	-
	KTP14	26.09101°S, 29.45912°E	0.0-0.3	-	0.3-1.8	-
	KTP15	26.09648°S, 29.45065°E	0.0-0.6	-	0.6-2.5	-
	KTP16	26.09714°S, 29.45249°E	0.0-0.3	-	0.3-2.1	-
	KTP17	26.09634°S, 29.45518°E	0.0-0.2	-	0.2-2.2	-
	KTP18	26.09752°S, 29.45380°E	0.0-0.2	0.2-2.3		-
	KTP19	26.09809°S, 29.45230°E	0.0-0.6	-	0.6-2.3	-
	KTP20	26.10502°S, 29.45183°E	0.0-0.1	0.1-2.7		-
Solar PV Site 2	KTP21	26.10459°S, 29.45453°E	0.0-0.1	0.1-2.4		-
	KTP22	26.10280°S, 29.45677°E	0.0-0.2	-	0.2-2.3	-
	KTP23	26.10463°S, 29.45968°E	0.0-0.3	0.3-2.4		-
	KTP24	26.10455°S, 29.46198°E	0.0-0.2	0.2-2.4		-
	KTP25	26.10464°S, 29.46486°E	0.0-0.4	-	0.4-2.3	2.1 m
	KTP26	26.10768°S, 29.46568°E	0.0-0.6	-	0.6-2.1	1.9 m
	KTP27	26.11053°S, 29.46493°E	0.0-0.2	-	0.2-2.5	-
	KTP28	26.10930°S, 29.46242°E	0.0-0.2	-	0.2-2.2	-
	KTP29	26.10974°S, 29.45976°E	0.0-0.5	-	0.5-2.8	2.7 m
	KTP30	26.10929°S, 29.45623°E	0.0-0.2	-	0.2-2.4	-
	KTP31	26.10831°S, 29.45309°E	0.0-0.2	-	0.2-2.1	-

Site	Test Pit positions		Layer thickness (m)			Seepage
	Test Pit	Coordinates	Hillwash	Pedogenic	Residual	
	KTP32	26.10666°S, 29.45509°E	0.0-0.2	-	0.2-2.2	-
	KTP33	26.10634°S, 29.45810°E	0.0-0.2	-	0.2-2.4	-
	KTP34	26.10734°S, 29.46017°E	0.0-0.4	-	0.4-2.1	-
	KTP35	26.10676°S, 29.46280°E	0.0-0.2	-	0.2-2.6	-
	KTP36	26.10323°S, 29.46236°E	-	-	0.0-1.9	-
	KTP37	26.10548°S, 29.45538°E	0.0-0.3	-	0.3-2.4	-
	KTP38	26.10744°S, 29.45743°E	0.0-0.4	-	0.4-2.7	-
BESS Unit 2	KTP39	26.09237°S, 29.47603°E	0.0-3.2	-	-	-
	KTP40	26.09259°S, 29.47676°E	0.0-2.8	-	-	1.1 m
	KTP41	26.09168°S, 29.47651°E	0.0-1.4	-	1.2-2.0	-
BESS Unit 3	KTP42	26.09134°S, 29.46811°E	0.0-2.5	-	-	-
	KTP43	26.09128°S, 29.46880°E	0.0-2.4	-	-	-
	KTP44	26.09124°S, 29.46712°E	0.0-1.0	-	0.1-2.6	2.4 m
BESS Unit 4	KTP45	26.09263°S, 29.46832°E	0.0-0.1	-	0.1-2.2	-
	KTP46	26.09255°S, 29.46762°E	0.0-1.0	-	1.0-2.3	-
	KTP47	26.09230°S, 29.46650°E	0.0-2.2	-	-	1.4 m

5.2 Dynamic Cone Penetrometer Results (DCP)

A total of forty-seven (47) DCP tests were undertaken in order to estimate the in-situ material strength. Medium dense to very dense conditions best describe material consistency as tested on site. In terms of bearing the capacity of soils, the DCP tests conducted across the site revealed that the site soils can accommodate 80 - 100 kPa loads at an average depth of 1.0 m for low-rise masonry structures such as office buildings, control room, etc. The detailed DCP reports are presented in **APPENDIX B: DYNAMIC CONE PENETROMETER TEST RESULTS**.

5.3 Dynamic Probing Results (DPSH)

A total of twenty-three (23) DPSH tests were conducted to give an indication on soil strength and depth to competent bedrock where possible. Generally, the tests indicated medium dense to very dense material consistencies with refusal reached at a maximum depth of 7.2 meters, possibly on weathered sandstone and siltstone, thus indicating intermediate to hard rock conditions. The detailed DCP reports are presented in **APPENDIX D: DYNAMIC PROBING TEST RESULTS (DPSH)**.

The bearing capacity of soils encountered on site was evaluated based on Meyerhof (1956, 1974) expressions. **Figure 5-1** shows the estimated bearing capacity (q_d) vs the width of foundation (B). In terms of the above, the tested materials can accommodate a bearing capacity of between kPa loads at or below the

recommended founding depth of 1.0 m for low-rise masonry structures below the surface (Applied FOS=1.5).

Dynamic probing tests DPSH1 – DPSH10 were undertaken at Solar PV Site 1. These indicated dense to very dense material consistency within a profile depth 0 – 7.5 meters. The material can accommodate a bearing capacity of 180 – 300 kPa between 0.3 and 4.5 m; whereas a bearing capacity in excess of 300 kPa can be achieved between 4.5 and 7.5 m.

Dynamic probing tests DPSH11 – DPSH20 were undertaken at Solar PV Site 2. These indicated medium dense to very dense material consistency within a profile depth 0 – 6.9 meters. The material can accommodate a bearing capacity of 80 – 300 kPa between 0.3 and 1.5 m; whereas a bearing capacity in excess of 300 kPa can be achieved between 1.5 and 6.9 m.

Dynamic probing test DPSH21 was undertaken at BESS Unit 2. The test indicated medium dense to very dense material consistency within a profile depth 0 – 2.7 meters. The material can accommodate a bearing capacity in excess 300 kPa from 1.8 to 2.7 m.

Dynamic probing test DPSH22 was undertaken at BESS Unit 3. The test indicated medium dense to very dense material consistency within a profile depth 0 – 6.3 meters. The material can accommodate a bearing capacity of between 80 and 180 kPa from 0.3 to 1.2 m; whereas a bearing capacity in excess of 300 kPa can be achieved between 3.9 m. A section of very loose material consistency was encountered 1.5 to 2.7 m.

Dynamic probing test DPSH22 was undertaken at BESS Unit 4. The test medium dense to very dense material consistency within a profile depth 0 – 6.0 meters. The material can accommodate a bearing capacity of 100 – 300 kPa between 0.6 and 2.4 m; whereas a bearing capacity in excess of 300 kPa can be achieved between 2.4 and 6.0 m.

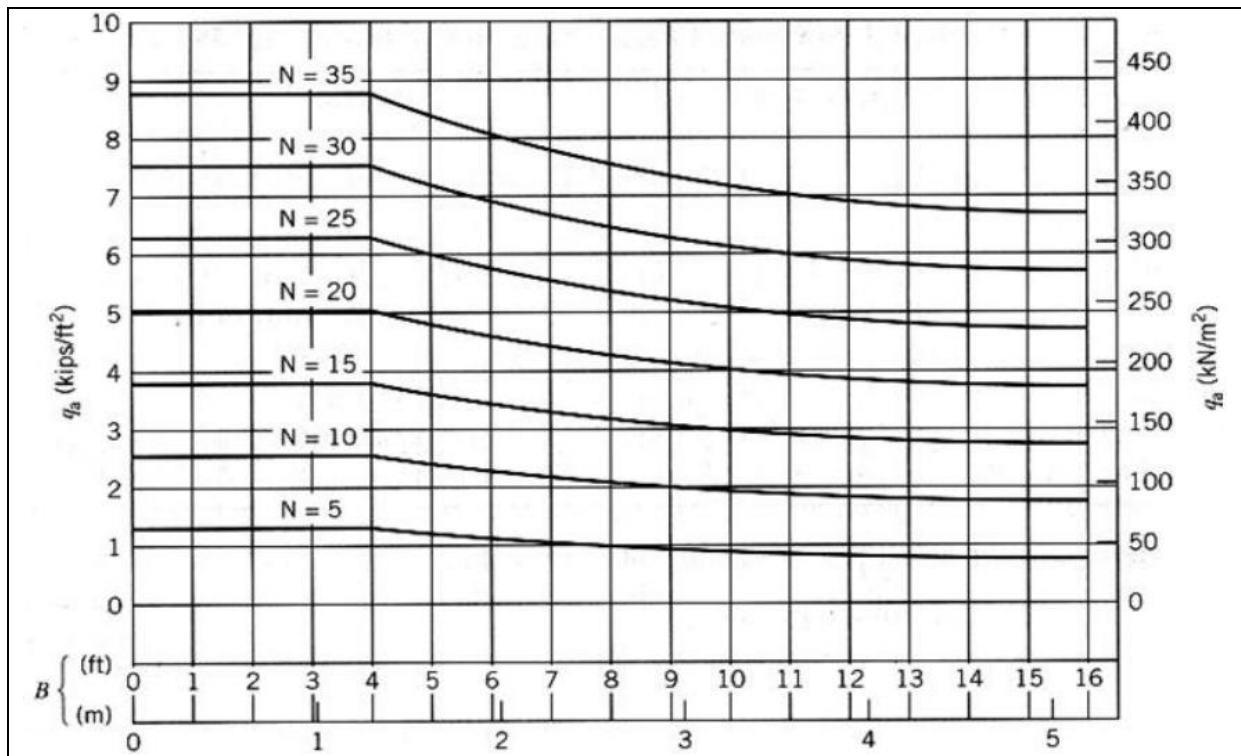


Figure 5-1: Estimation of bearing capacity based on Meyerhof's, where $k_d=1.0$ and n =Average DPSH/SPT value, note that $1 \text{ kN/m}^2 = 1 \text{ kPa}$

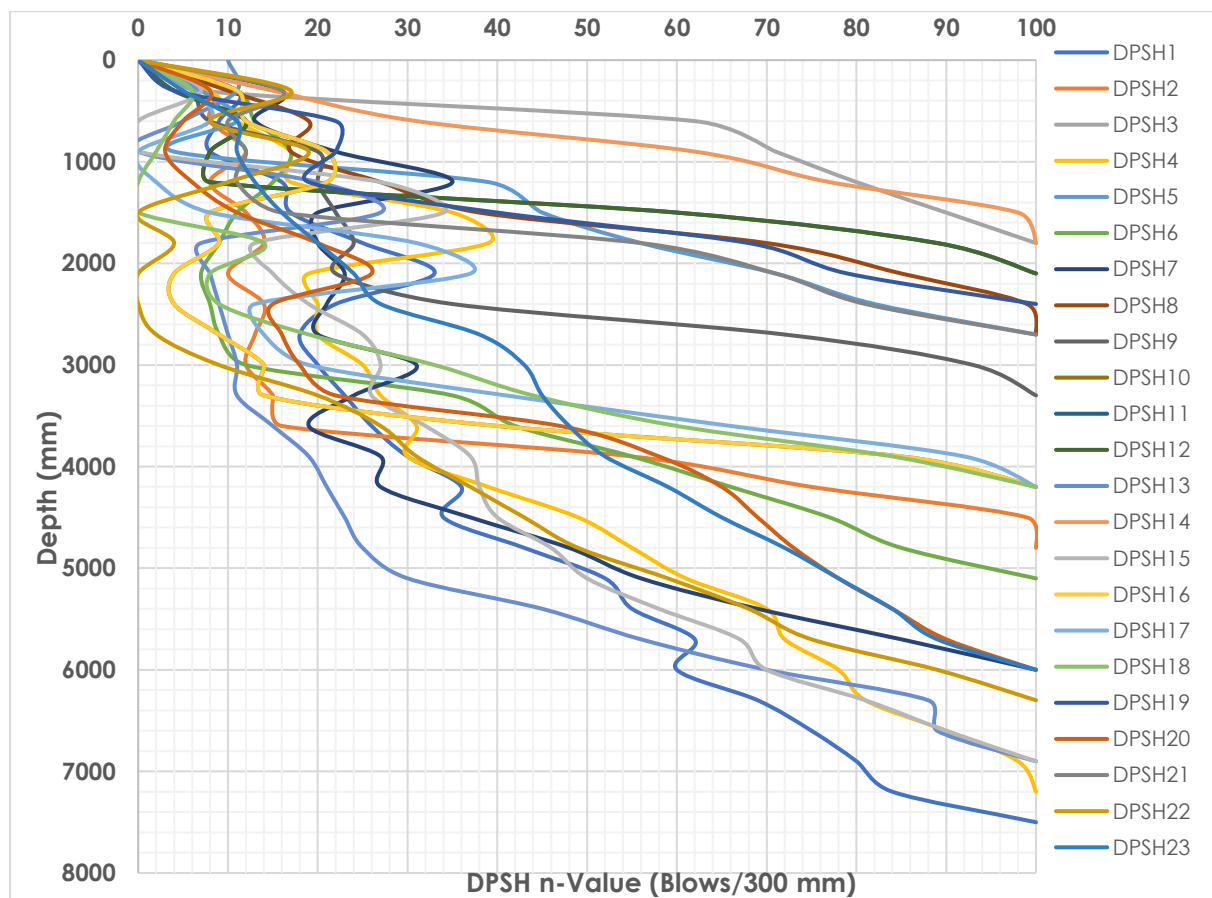


Figure 5-2: Summary of dynamic probing results

5.4 Percussion Borehole Results

A total of twenty-five (25) boreholes (named K-BH01 to K-BH25) were drilled at selected positions using a percussion drilling rig equipped with a compressor capable of reaching 21-bar. Chip samples were collected per meter drilled on-site and described according to the current industry standards as described in SANS 633. The main purpose of the percussion drilling at Solar PV site 1 (airstrip), was to investigate possible undermined ground and the depth thereof.

The drilling at Solar PV Site 1 generally revealed transported soils, underlain by residual sandstone, sandstone interlayered with siltstone, carbonaceous shale, shaly coal, and thin layers of coal, and diamictite, respectively. Some intrusive material interpreted as dolerite were encountered in places, particularly within the Vryheid Formation. Drilling results can be summarised as follows:

5.4.1 Material Description

The site is covered by a layer of transported soil (hillwash). This layer was generally dark brown to brown, clayey sand. The average thickness of this layer is 2.0 meters. The transported layer was encountered in all boreholes from surface up to 2.0 meter.

The transported soil is mostly underlain by a residual sandstone layer. The residual soil underlying the hillwash was generally greyish brown mottled orange, clayey sand of completely to highly weathered sandstone. The average thickness of this material is 4.0 meters and attains a maximum of 5.0 meters in other boreholes.

Greyish brown, highly to moderately weathered sandstone was encountered in all boreholes interlayered with siltstone. The combined thickness of the sandstone-siltstone interlayers can be in excess of 30 meters.

Layers of shale, shaly coal, and coal were encountered underneath the sandstone-siltstone interlayers. These were generally described as dark grey to black, moderate to highly weathered, soft rock carbonaceous shale of the Vryheid Formation with interlayered coal. The combined thickness of this layers is about 40 meters.

A layer of greyish pink, slightly weathered diamictite of the Dwyka Group was encountered in six (6) boreholes viz. K-BH6, K-BH7, K-BH8, K-BH10, K-BH20, and K-BH22 mainly at depths 65 meters underneath the coal layers.

Table 5-2 below summary of layers encountered, whereas detailed percussion borehole profiles are presented in **APPENDIX E: PERCUSSION DRILLING LOGS**.

Table 5-2: Summary of percussion borehole results

Hole No.	Quaternary Soils		Vryheid Formation					Intrusive	Dwyka Group	Cavity	Water Strike	Water Level
	Transported Soils	Residual Soils	Sandstone	Siltstone	Shale	Shaly coal	Coal	Dolerite	Diamictite			
K-BH01	0.0 - 2.0	2.0 - 4.0	4.0 - 15	15 - 20	52 - 57	28 - 29	26 - 27			58 - 60.5	56	25
			20 - 23	23 - 26			57 - 58					
			27 - 28	29 - 31								
			31 - 41	41 - 52								
K-BH02	0.0 - 2.0	2.0 - 7.0	7.0 - 22	36 - 40	22 - 25		25 - 26			60 - 62	21	28
			26 - 27	43 - 46	46 - 49		27 - 29					
			29 - 36	49 - 58			58 - 60					
			40 - 43									
K-BH03	0.0 - 1.0	1.0 - 3.0	3.0 - 16	16 - 20						55 - 58	21	25
			20 - 21	21 - 26								
			26 - 34	34 - 55								
K-BH04	0.0 - 2.0	2.0 - 5.0	5.0 - 20			20 - 29	29 - 30	30 - 50			30	16
K-BH05	0.0 - 2.0	2.0 - 5.0	5.0 - 11	31 - 41	11 - 28	28 - 31		41 - 51			10	6.8
K-BH06	0.0 - 2.0	2.0 - 5.0	24 - 27	41 - 70	5.0 - 24	70 - 72	30 - 31		89 - 101		67	26
			33 - 41	76 - 89	27 - 30		32 - 33					
					31 - 32		72 - 76					
K-BH07	0.0 - 2.0	2.0 - 6.0	33 - 40	26 - 28	6.0 - 9.0	60 - 62	28 - 29	9.0 - 12	66 - 81		33	11
				29 - 30	12 - 26		30 - 32					
				32 - 33			43 - 45					
				40 - 43			62 - 66					
				45 - 60								
K-BH08	0.0 - 2.0	2.0 - 6.0	9.0 - 12	33 - 42	6.0 - 9.0		45277		48 - 61		35	7
			21 - 33		17 - 21		42 - 48					
K-BH09	0.0 - 2.0	2.0 - 7.0	28 - 33	33 - 35	7.0 - 13	35 - 36	36 - 38	13 - 28		68 - 69.5	45	39
			40 - 48	38 - 40			48 - 52					
				52 - 66			66 - 68					
K-BH10	0.0 - 2.0	2.0 - 5.0	25 - 28	31 - 33	5.0 - 13	66 - 71	33 - 36	13 - 16	71 - 76		71	27
			37 - 46	36 - 37	16 - 20		46 - 49	20 - 25				
				49 - 63	28 - 31		63 - 66					

Hole No.	Quaternary Soils		Vryheid Formation					Intrusive	Dwyka Group	Cavity	Water Strike	Water Level
	Transported Soils	Residual Soils	Sandstone	Siltstone	Shale	Shaly coal	Coal	Dolerite	Diamictite			
K-BH11	0.0 - 2.0	2.0 - 6.0	18 - 21	21 - 26	6.0 - 18	37 - 38	26 - 30				35	24
			31 - 37	30 - 31		41 - 46	38 - 41					
			46 - 51									
K-BH12	0.0 - 5.0	2.0 - 5.0	5.0 - 19	44 - 47	56 - 70	36 - 44	53 - 56	19 - 33			21	17
			33 - 36	76 - 77	78 - 81	77 - 78	70 - 76					
			47 - 53									
K-BH13	0.0 - 5.0	2.0 - 5.0	25 - 28	7.0 - 25	5.0 - 7.0	28 - 32	32 - 36			65 - 66	16	30
			36 - 45	51 - 63		45 - 51	64 - 65					
						63 - 64						
K-BH14	0.0 - 1.0	1.0 - 4.0	4.0 - 15	32 - 34	30 - 32	34 - 38	46 - 47	15 - 26		46 - 47	42	17
			26 - 30	38 - 39								
			39 - 46									
K-BH15	0.0 - 1.0	1.0 - 3.0	3.0 - 15	35 - 38	15 - 17	38 - 39	52 - 55	17 - 29		71 - 74	30	17
			29 - 35	35 - 38			70 - 71					
			44 - 52	39 - 44								
				55 - 70								
K-BH16	0.0 - 1.0	1.0 - 3.0	3.0 - 15	59 - 69			33 - 36	15 - 25			30	16
			25 - 33									
			36 - 45				45 - 47					
			47 - 50				50 - 51					
			51 - 59				69 - 74					
			74 - 76									
K-BH17	0.0 - 1.0	1.0 - 4.0	4.0 - 30								24	5
K-BH18	0.0 - 1.0	1.0 - 3.0	3.0 - 17	36 - 41	17 - 19		41 - 43	19 - 31			34	11
			31 - 36	53 - 58	66 - 70		51 - 53					
			43 - 51	61 - 66			70 - 75					
			58 - 61									
			75 - 76									
K-BH19	0.0 - 1.0	1.0 - 4.0	4.0 - 17	38 - 41	17 - 19	57 - 58	42 - 44	20 - 34			36	12
			19 - 20	56 - 57	78 - 79		75 - 78					
			34 - 38	58 - 75			79 - 81					

Hole No.	Quaternary Soils		Vryheid Formation					Intrusive	Dwyka Group	Cavity	Water Strike	Water Level
	Transported Soils	Residual Soils	Sandstone	Siltstone	Shale	Shaly coal	Coal	Dolerite	Diamictite			
			41 - 42									
			44 - 56									
			81 - 84									
K-BH20	0.0 - 1.0	1.0 - 4.0	4.0 - 17	17 - 19	76 - 77		42 - 44	19 - 33	82 - 86		52	7
			33 - 38	38 - 42			73 - 76					
			44 - 51	51 - 73			77 - 78					
				78 - 82								
K-BH21	0.0 - 2.0	2.0 - 7.0	36 - 45	46 - 52	7.0 - 18	34 - 35	27 - 28	18 - 25		65 - 65.5	57	28
				61 - 62	25 - 27	52 - 61	30 - 31					
					28 - 30		32 - 34					
					31 - 32		45 - 46					
							62 - 65					
K-BH22	0.0 - 2.0	2.0 - 6.0	23 - 25	18 - 23	6.0 - 10	60 - 62	30 - 31	10 - 13	71 - 76		37	17
			31 - 32	33 - 35	13 - 18	66 - 71	32 - 33					
			35 - 43	47 - 57	25 - 30		43 - 47					
					57 - 60		62 - 66					
K-BH23	0.0 - 2.0	2.0 - 5.0	5.0 - 34	35 - 37		50 - 51	34 - 35			71 - 74	62	11
			38 - 39	39 - 42		69 - 70	37 - 38					
			42 - 50	56 - 69			51 - 53					
			53 - 56				70 - 71					
K-BH24	0.0 - 2.0	2.0 - 6.0	20 - 25	57 - 63	6.0 - 8.0		29 - 31	8.0 - 20			23	12
			28 - 29		25 - 27	27 - 28	40 - 41					
			31 - 40		41 - 47							
			47 - 57									
K-BH25	0.0 - 2.0	2.0 - 6.0	6.0 - 14		14 - 26	46 - 47	26 - 29				30	27
			29 - 30	30 - 40			40 - 46					
			47 - 51									

5.5 Rotary Core Drilling Results

A total of fifteen (15) rotary core boreholes (named BH1 to BH15) were planned and drilled to depths between 15 to 20 meters or 6 meters into hard rock, whichever comes first. All fifteen (15) boreholes have been completed. Standard Penetration Tests (SPT) were also conducted at 1.5-meter intervals and where clayey material was encountered.

SPTs, at 1.5 m intervals and commencing at a depth of 1.5 m. Below SPT refusal depths, NWD4 rotary drilling followed in through the underlying gravel/boulder beds and/or bedrock. Temporary steel casings (N-size) were used to stabilise the boreholes during the drilling.

Boreholes were terminated at depths of between 15 – 20 meters within the underlying bedrock. The soil samples obtained from the SPTs and the core recovered from the rotary drilling were profiled and logged in detail according to the Guidelines for Soil and Rock Logging in South Africa. The following parameters were recorded:

- For soil: colour, consistency, structure (where evident), soil type and origin.
- For rock: colour, weathering, structure, hardness, rock type and origin.

In addition to the description of the soil and rock layers, the following drilling attributes were also recorded:

- Depth below ground level and reduced level relative to mean sea level.
- Drilling method.
- Core recovery (%CR).
- Rock quality designation (%RQD).

5.5.1 Material Description

0.0 – 1.0 m Dark grey, clayey sand. Transported soil.

1.0 – 4.50 m Grey, clayey sand with ferricrete gravel. Residual soil.

4.50 – 9.0 m Light grey, moderately weathered, coarse-grained, jointed and medium to thinly layered, medium hardrock, siltstone. Vryheid Formation.

9.0 – 13.0 m Grey, highly weathered, fine-grained, thinly layered, highly fractured, soft rock, carbonaceous shale. Vryheid Formation.

13.0 – 14.0 m Shaly Coal: Black, highly weathered, thinly layered, highly fractured, very soft rock, shaly coal. Vryheid Formation.

14.0 - 20.0 m Grey, moderately weathered, coarse grained, layered, medium hardrock, sandstone. Vryheid Formation

7.0 – 16.0 m Grey, moderately weathered, coarse grained, hard rock, ~dolerite (intrusive).

5.5.2 Standard Penetration Test

SPT tests were carried out in accordance with the method specifications given in the ASTM using a Raymond Split-spoon sampler. In all cases, the blow count for the advance over a distance of 75mm for a total run length of 450mm (where possible) was recorded, with refusal defined as less than 75mm advance for 25 blows. The SPT-N values were determined by omitting the first two 75mm advance blow count intervals and summing the remaining four intervals (300mm). The correlation between SPT-N value and consistency of the soil horizons, relative density (%) and friction angle (degrees) is given in **Table 5-3**.

Table 5-3: The summary of Standard Penetration tests undertaken at Solar PV Site 1 and 2.

Hole No.	SPT Depth	SPT Test Results				n-Value	Consistency	Relative Density %	Friction Angle (°)
		75	150	225	450				
BH01	1.50 - 2.00 m	17	18	20	22	n = 77	Very dense	> 80	> 45
	3.00 - 3.50 m	1	2	4	5	n = 12	Medium dense	40 - 60	35 - 40
BH02	1.50 - 2.00 m	4	4	4	4	n = 16	Medium dense	40 - 60	35 - 40
	3.00 - 3.50 m	4	4	4	5	n = 17	Medium dense	40 - 60	35 - 40
	4.12 - 4.70 m	Ref				Ref	Very dense	> 80	> 45
BH03	1.50 - 2.00 m	3	3	3	4	n = 13	Medium dense	40 - 60	35 - 40
	3.00 - 3.50 m	1	2	3	4	n = 10	Medium dense	40 - 60	35 - 40
	4.50 - 5.00 m	4	6	6	10	n = 26	Medium dense	40 - 60	35 - 40
BH04	1.50 - 2.00 m	2	2	3	4	n = 11	Medium dense	40 - 60	35 - 40
	3.00 - 3.50 m	1	2	3	4	n = 10	Medium dense	40 - 60	35 - 40
	4.50 - 5.00 m	Ref					Very dense	> 80	> 45
BH05	1.50 - 2.00 m	3	3	4	4	n = 14	Medium dense	40 - 60	35 - 40
	3.00 - 3.50 m	5	7	9	9	n = 30	Dense	60 - 80	40 - 45
	4.50 - 5.00 m	6	6	6	6	n = 24	Medium dense	40 - 60	35 - 40
BH06	1.50 - 2.00 m	3	4	5	6	n = 18	Medium dense	40 - 60	35 - 40
	3.00 - 3.50 m	4	5	5	5	n = 19	Medium dense	40 - 60	35 - 40
	4.50 - 5.00 m	Ref				Ref	Very dense	> 80	> 45
BH07	1.50 - 2.00 m	1	2	2	3	n = 8	Loose	20 - 40	30 - 35
	3.00 - 3.50 m	2	3	6	6	n = 17	Medium dense	40 - 60	35 - 40
BH08	1.50 - 2.00 m	Ref				Ref	Very dense	> 80	> 45
	3.00 - 3.50 m	2	3	6	6	n = 17	Medium dense	40 - 60	35 - 40
	4.50 - 5.00 m	6	7	10	11	n = 34	Dense	60 - 80	40 - 45
BH09	1.50 - 2.00 m	3	4	6	7	n = 20	Medium dense	40 - 60	35 - 40
	3.00 - 3.50 m	4	4	10	10	n = 28	Dense	60 - 80	40 - 45

Hole No.	SPT Depth	SPT Test Results				n-Value	Consistency	Relative Density %	Friction Angle (°)
		75	150	225	450				
	4.50 - 5.00 m	4	5	Ref		n = 9	Medium dense - very dense	40 - 60	35 - 40
BH10	1.50 - 2.00 m	Ref				Ref	Very dense	> 80	> 45
	3.00 - 3.50 m	6	7	9	11	n = 33	Dense	60 - 80	40 - 45
	4.50 - 5.00 m	13	19	23	Ref	n = 55	Very dense	> 80	> 45
BH11	1.50 - 2.00 m	2	3	4	5	n = 14	Medium dense	40 - 60	35 - 40
	3.00 - 4.21 m	Ref					Very dense	> 80	> 45
BH12	1.50 - 2.00 m	7	11	12	14	n = 44	Dense	60 - 80	40 - 45
	3.00 - 3.00 m	Ref				Ref	Very dense	> 80	> 45
BH13	1.50 - 2.00 m	5	6	6	7	n = 24	Medium dense	40 - 60	35 - 40
	3.00 - 3.50 m	4	6	6	7	n = 23	Medium dense	40 - 60	35 - 40
BH14	1.84 - 2.34 m	4	4	5	5	n = 18	Medium dense	40 - 60	35 - 40
	3.00 - 3.50 m	4	5	7	9	n = 25	Medium dense	40 - 60	35 - 40
	4.00 - 4.50 m	Ref				Ref	Very dense	> 80	> 45
BH15	1.80 - 2.30 m	7	25	Ref		n = 32	Dense -very dense	60 - 80	40 - 45
	3.50 - 3.90 m	12	13	20	20	n = 65	Very dense	> 80	> 45

Table 5-4: Summary of rotary core borehole results

Hole No.	Layers encountered (mbgl)						E.O.H (mbgl)
	Transported soils	Residual Soils	Sandstone	Shale	Sandstone interlayered with Siltstone	Shaly coal	
BH01	0.0 - 0.95	0.95 - 3.68		9.25 - 12.85	3.68 - 9.25	12.85 - 14.08	15.07
				14.08 - 15.07	8.11 - 8.80		
BH02	0.0 - 0.86	0.86 - 3.45			3.45 - 12.25	12.25 - 13.00	19.2
					13.00 - 15.35	15.35 - 17.54	
					17.54 - 19.12		
BH03	0.0 - 1.23	1.23 - 5.0	5.0 - 5.39	5.39 - 8.11	8.11 - 8.80		20.30
			8.80 - 20.30				
BH04	0.0 - 0.56	0.56 - 3.50	3.50 - 4.68		4.68 - 14.50		19.98
					14.50 - 18.20		
					18.20 - 19.98		
BH05	0.0 - 0.82	0.82 - 4.02	4.02 - 7.74			7.74 - 15.47	15.47
BH06	0.0 - 0.80	0.80 - 5.55			5.55 - 14.25	14.25 - 16.21	16.21

Hole No.	Layers encountered (mbgl)						E.O.H (mbgl)
	Transported soils	Residual Soils	Sandstone	Shale	Sandstone interlayered with Siltstone	Shaly coal	
BH07	0.0 - 1.18		1.18 - 11.30	11.30 - 11.42			15.55
			11.42 - 15.55				
BH08	0.0 - 1.50			1.50 - 2.00	2.00 - 15.41		15.41
BH09	0.0 - 1.16	1.16 - 1.5	1.5 - 5.81		5.81 - 14.16		15.38
					14.16 - 15.38		
BH10	0.0 - 0.96	0.96 - 2.58	2.58 - 5.61		5.61 - 15.26		15.26
BH11	0.0 - 8.7	0.87 - 2.73	2.73 - 4.78	4.78 - 11.70	11.70 - 16.21		16.21
BH12	0.0 - 0.5	0.5 - 1.20	1.20 - 8.66		8.66 - 11.88		15.07
					11.88 - 15.07		
BH13	0.0 - 0.45	0.45 - 3.62	3.62 - 4.22		4.22 - 7.11		16.21
			7.11 - 10.21		10.21 - 16.21		
BH14	0.0 - 1.45	1.45 - 4.40			4.40 - 16.21		16.21
BH15	0.0 - 0.94	0.94 - 2.83	12.91 - 16.21		2.83 - 8.71		16.21
					8.71 - 12.91		

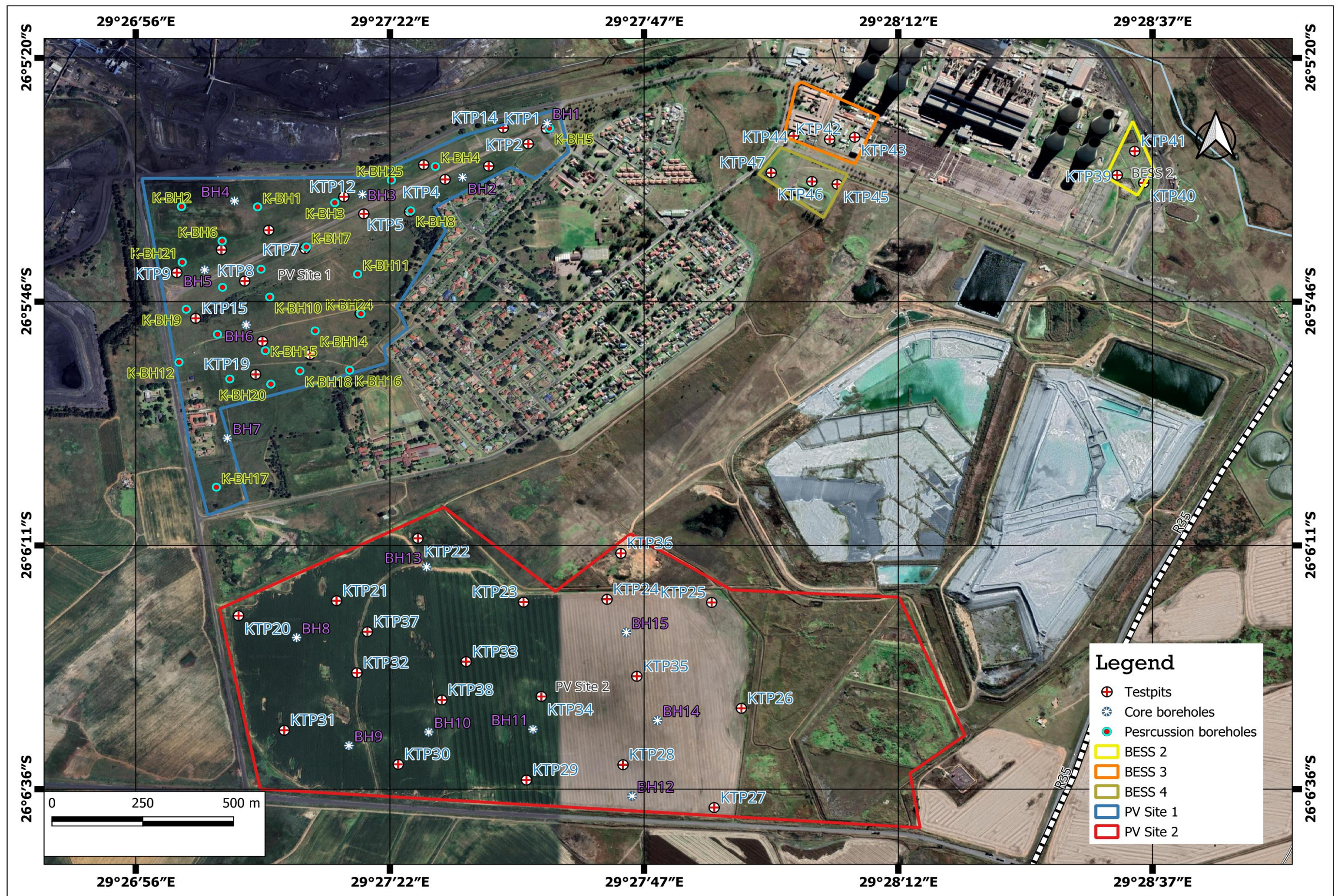


Figure 5-3: Geotechnical map showing all testing and sampling points undertaken with the sites

6 LABORATORY TEST RESULTS

6.1 General

A total of thirty-five (35) disturbed samples were recovered from representative soil horizons in selected test pits across the sites and submitted to Specialised Testing Laboratory Pty Ltd (STLLAB), a SANAS accredited civil engineering laboratory for further classification. These were ordered for foundation indicator tests comprising Atterberg Limits (<0.425mm), Sieve Analysis (<0.075mm), Heave Classification (<0.002 mm), and Particle Size Distribution in order to determine basic engineering properties for soils encountered within the sites. Due to the crumbling nature of the material, no undisturbed sample were taken test pits. Furthermore, twenty-two (22) bulk samples were taken for compaction tests including Moisture Density Relationship (MDR) and California Bearing Ratio (CBR) at Mod. AASTHO density.

The laboratory results are summarized in **Table 6-2** below, while detailed test results are presented in **APPENDIX C: LABORATORY TEST RESULTS**. The following tests were undertaken:

- **Foundation Indicator Tests** (Atterberg Limits, Grading Analysis, etc.) - used to establish the soil type, its potential for heave and give an indication of its suitability for use in pavement layers.
- **Compaction Tests** (CBR & MOD) - used to assess the potential for materials to be used in pavement layers.
- **Chemical Tests** (pH & EC)– to assess the potential for soils to corrode materials and to assess the extent of soil contamination.

6.2 Indicator Testing

The tested soils generally graded as clayey sand and sandy clay within minor gravel. The plasticity index (PI) for the material ranged between 0 and 23%; the Liquid limit ranged from 0 to 40%; the linear shrinkage ranged from 0 to 12.0%; whereas the grading modulus ranged between 0.54 and 1.96, all indicating low to medium potential expansiveness for the material encountered within the site (refer **Figure 6-1**).

6.3 Compaction Test Results

The maximum dry density (MDD) for the tested soil ranged between 1675 and 2095 kg/m³ at an optimum moisture content (OMC) of between 9.4 and 18.9%. The CBR values for this material ranged between 2.8 and 16 at 93%, 3.3 and 19 at 95%, 5.2 and 24 at 100% Mod AASHTO dry density. In terms of the COLTO, the material tested can be classified as G7 to worse than G9 quality soil mixtures.

6.4 Aggressiveness Towards Concrete

The aggressiveness of the soils encountered on site was analysed based on the method developed by JJ Basson (1989). **Table 6-1** below shows the determined chemical indices used to evaluate the corrosivity and aggressiveness of soil towards concrete.

The pH and conductivity of soil were determined to get an indication of the potential corrosiveness of the soil towards metals. Corrosion on metals may take place in acidic soils with a pH-value of less than 6. It is, however, dependent on various factors, including; soil resistivity, pH, chloride content, sulphate content, sulphide ion content, soil moisture and oxygen content.

The tested soil samples indicated a general pH-value of between 5.4 to 7.0, which may be interpreted as corrosive towards metals.

Table 6-1: Summary of the Basson Index test results

Test Pit No.	Depth (m)	Chemical Analysis				Corrosivity Indices	Basson Index
		pH Stability	LI	RI	N _c		
KTP6	0.0 – 1.0	9.5	-3.1	12.5	2607	Corrosive	Aggressive
KTP11	0.7 – 2.4	9.8	-3.1	13.0	2646	Corrosive	Aggressive
KTP17	0.2 – 2.2	10.4	-4.3	14.7	3463	Corrosive	Aggressive
KTP24	0.2 – 2.4	9.9	-3.3	13.2	2744	Corrosive	Aggressive
KTP27	0.2 – 2.5	9.3	-2.9	12.1	2468	Corrosive	Aggressive
KTP34	0.4 – 2.1	9.5	-3.0	12.5	2549	Corrosive	Aggressive
KTP38	0.4 – 2.7	9.9	-3.6	13.5	2978	Corrosive	Aggressive
KTP40	0.1 – 1.1	8.6	-1.1	9.7	1204	Corrosive	Aggressive
KTP45	0.1 – 2.2	10.0	-4.6	14.7	3720	Corrosive	Aggressive

LI = Langelier Index, RI = Ryznar Index, N_c = Aggressiveness Index

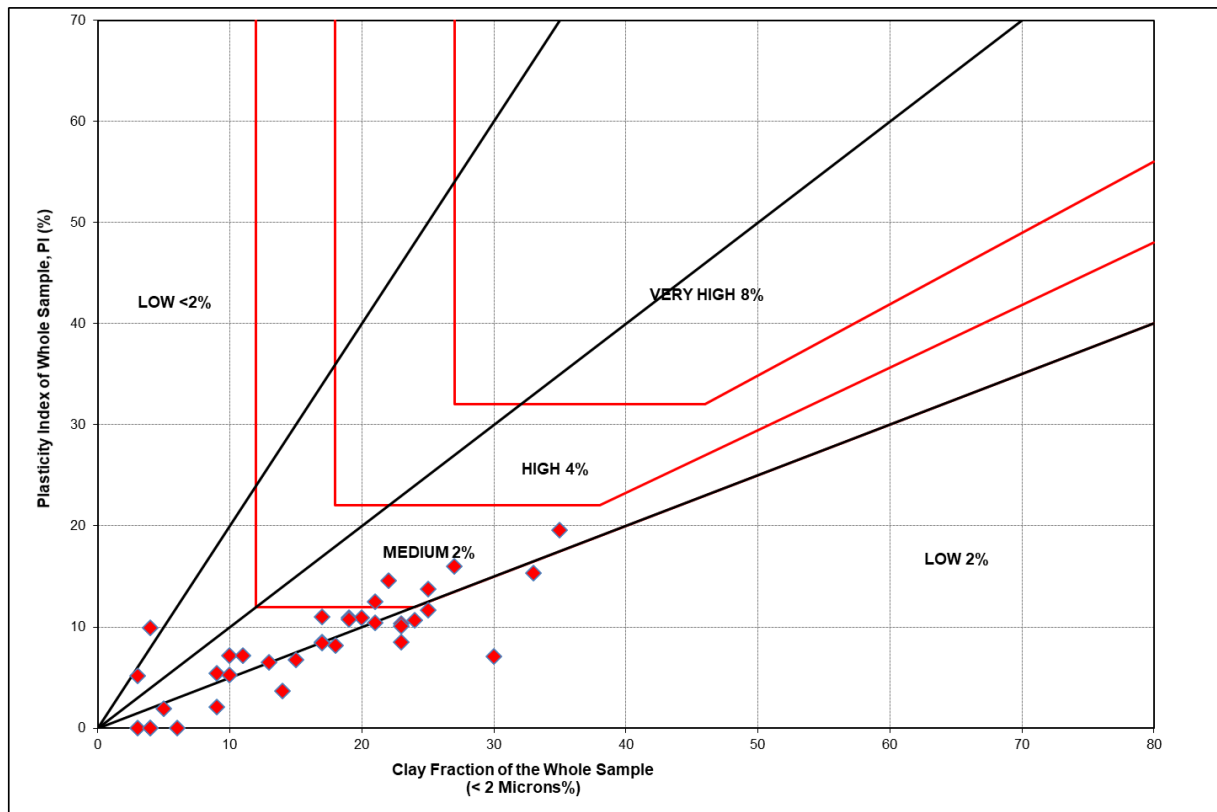


Figure 6-1: The illustrations of the potential expansiveness for materials encountered within the sites.

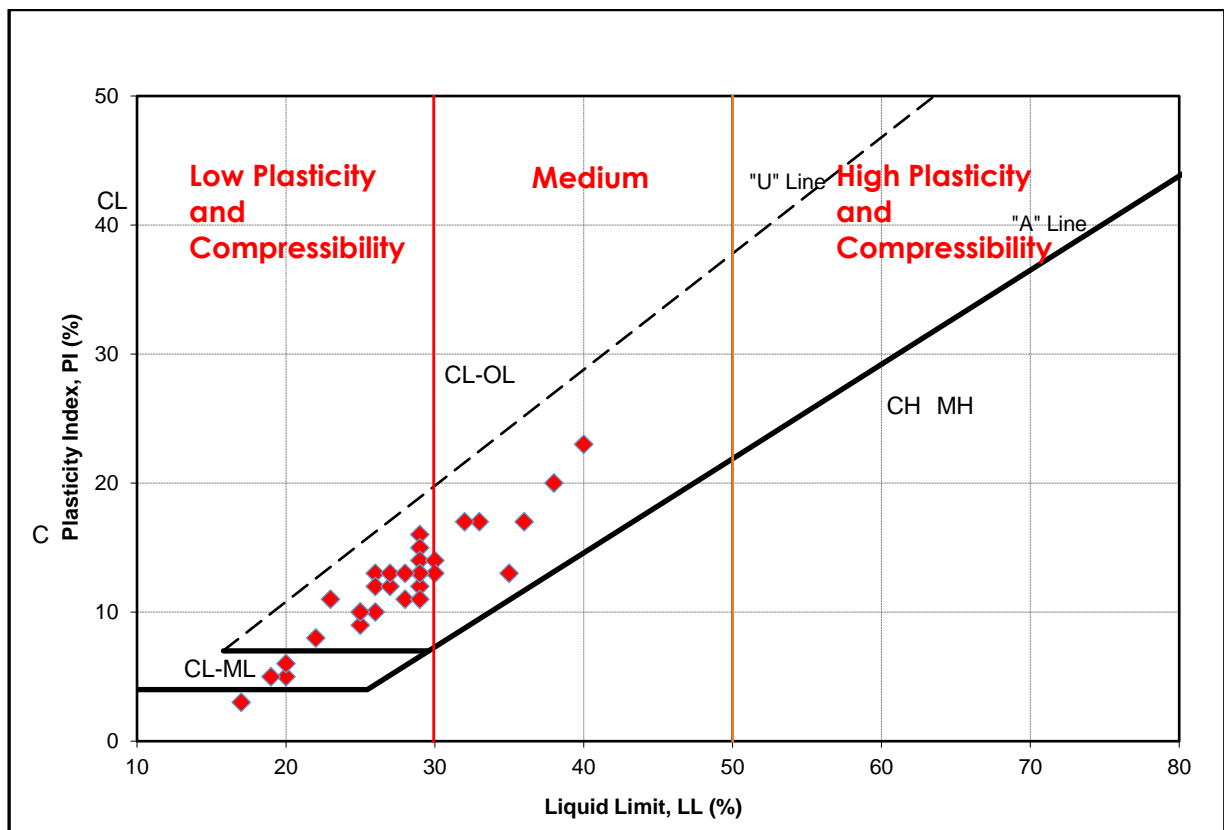


Figure 6-2: The typical illustrations of the plasticity for material encountered within the sites.

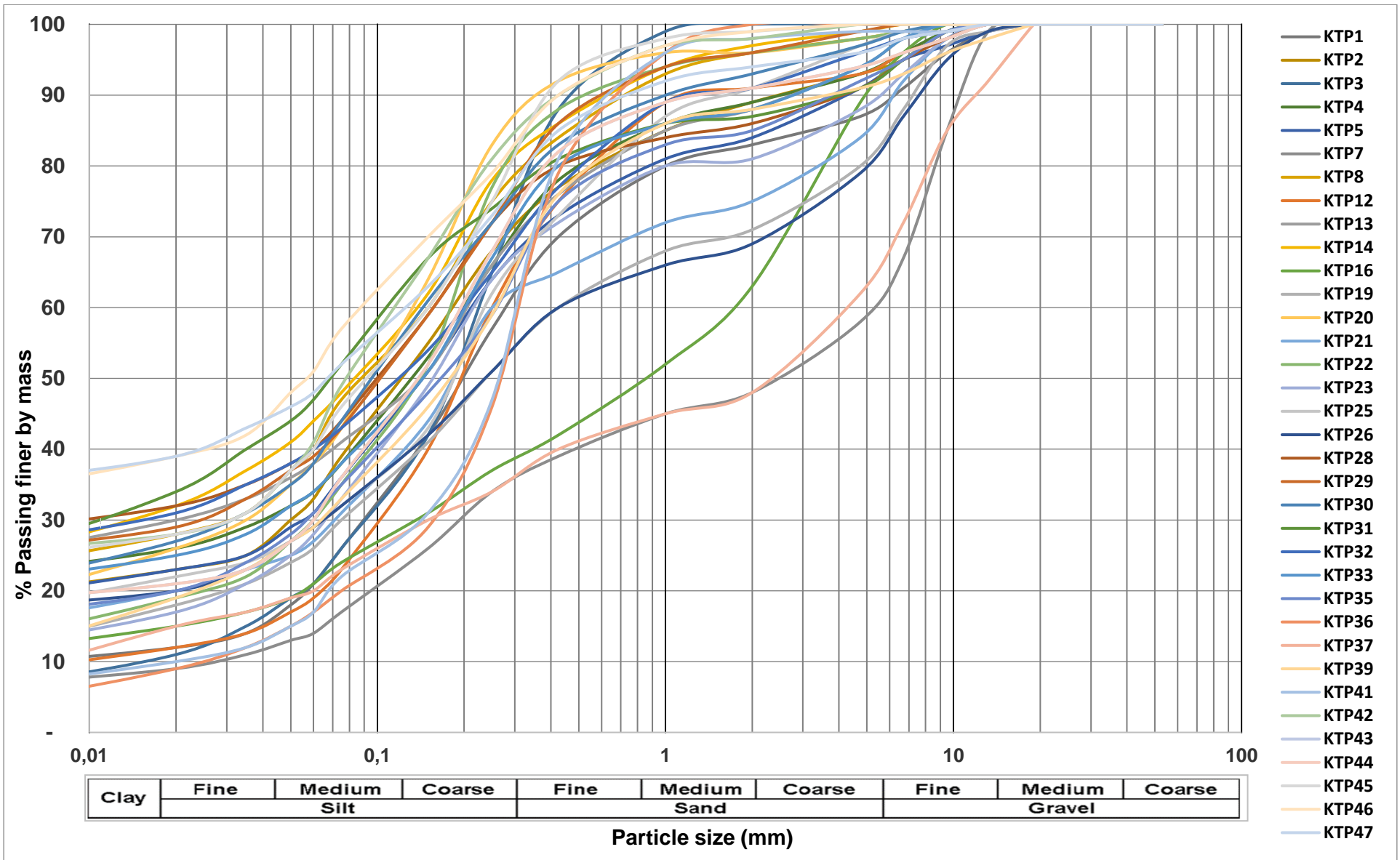


Figure 6-3: Soil gradation curves showing the particle size distribution for materials encountered within the sites

Table 6-2: Summary of Laboratory Test Results

Test Pit	Sample Depth	% Clay	Foundation Indicator Test						Compaction Test					COLTO Class	USCS	pH-Value	EC (s/m)
			Atterberg Limits					GM	MDR		CBR at % Mod, AASHTO						
			wPI	PI (%)	LI (%)	LS (%)	% 0,425		MDD	OMC	100%	95%	93%				
KTP1	0.7 - 2.2	9	2	3	17	2.5	70	1.21	2062	9.4	11.6	9.0	8.1	G9	sc-sm	-	-
KTP2	0.8 - 2.4	17	8	11	23	5.0	77	0.95	1951	11.9	9.0	6.7	5.9	-	SC	-	-
KTP3	0.0 -0.4	4	0	0	0	0.5	88	0.86	-	-	-	-	-	-	SM	6.2	0.014
KTP4	0.3 - 2.3	21	12	16	29	8.0	78	0.95	1835	15.6	5.2	3.3	2.8	-	SC	-	-
KTP5	0.4 - 2.5	19	11	15	29	7.5	73	1.07	1872	13.9	5.5	3.6	3.0	-	SC	-	-
KTP7	0.0 - 1.9	5	2	5	20	2.5	39	1.96	-	-	-	-	-	-	gc-gm	6.1	0.005
KTP8	0.4 - 2.3	20	11	13	26	6.0	84	0.73	-	-	-	-	-	-	SC	-	-
KTP12	0.0 - 0.6	6	0	0	0	0.5	75	1.11	-	-	-	-	-	-	SM	-	-
KTP13	0.3 - 1.8	24	11	14	29	7.0	76	0.95	1883	13.9	8.2	6.9	5.9	-	SC	-	-
KTP14	0.3 - 1.8	22	15	17	33	8.5	86	0.69	-	-	-	-	-	-	SC	6.4	0.018
KTP16	0.3 - 2.1	9	5	13	29	6.5	42	1.71	2017	9.5	22	19	16	G7	SC	-	-
KTP19	0.6 - 2.3	10	7	12	26	5.5	60	1.39	-	-	-	-	-	-	SC	-	-
KTP20	0.1 - 2.7	17	11	12	27	6.0	92	0.69	1843	13.1	5.8	4.1	3.5	-	SC	6.1	0.017
KTP21	0.1 - 2.4	13	7	10	26	5.0	65	1.29	2095	10.7	24	19	16	G7	SC	-	-
KTP22	0.2 - 2.3	10	5	6	20	3.0	88	0.81	-	-	-	-	-	-	sc-sm	7.0	0.007
KTP23	0.3 - 2.4	11	7	10	25	5.0	72	1.14	1991	11.7	10.5	8.2	7.5	G9	SC	-	-
KTP25	0.4 - 2.3	14	4	5	19	2.5	73	1.04	2000	12.5	15	9.0	6.0	-	sc-sm	-	-

Test Pit	Sample Depth	% Clay	Foundation Indicator Test						GM	Compaction Test					COLTO Class	USCS	pH-Value	EC (S/m)
			Atterberg Limits					MDR		CBR at % Mod, AASHTO								
			wPI	PI (%)	LL (%)	LS (%)	% 0,425	MDD		OMC	100%	95%	93%					
KTP26	0.6 - 2.1	17	8	14	30	6.5	60	1.39	1968	11.6	8.2	6.0	5.3	-	SC	-	-	
KTP28	0.2 - 2.2	27	16	20	38	9.5	80	0.90	-	-	-	-	-	-	SC	6.1	0.023	
KTP29	0.5 - 2.8	23	10	12	29	6.0	86	0.75	1872	13.5	8.7	6.8	5.3	-	SC	-	-	
KTP30	0.2 - 2.4	19	11	13	35	7.0	83	0.80	1675	18.9	10.0	8.6	6.1	-	SC	-	-	
KTP31	0.2 - 2.1	25	14	17	36	8.5	81	0.80	-	-	-	-	-	-	CL	5.9	0.015	
KTP32	0.2 - 2.0	23	8	11	28	5.5	77	0.89	2022	11.1	15	12	10	G8	SC	-	-	
KTP33	0.2 - 2.4	21	10	13	30	6.5	80	0.94	1965	11.7	13	10	9.0	-	SC	-	-	
KTP35	0.2 - 2.6	15	7	9	25	4.0	75	1.05	2030	10.3	16	12	11	G9	SC	-	-	
KTP36	0.9 - 1.9	3	0	0	0	0.0	79	1.01	1914	10.5	13	10	8.0	G9	SM	-	-	
KTP37	0.3 - 2.4	3	5	13	29	6.5	40	1.89	-	-	-	-	-	-	SC	5.5	0.007	
KTP39	1.1 - 3.2	4	10	13	27	6.5	76	1.03	1910	12.0	7.6	4.9	3.7	-	SC	4.7	0.070	
KTP41	1.4 - 2.0	4	0	0	0	0.0	81	0.99	1937	10.0	21	14	12	G8	SM	-	-	
KTP42	0.7 - 2.5	25	12	13	28	6.5	90	0.63	1889	13.9	15	11	9.0	-	SC	5.4	0.014	
KTP43	0.1 - 2.4	30	7	8	22	4.0	89	0.57	-	-	-	-	-	-	CL	-	-	
KTP44	0.1 - 2.6	18	8	10	25	5.0	82	0.91	1942	11.1	17	13	11	G8	SC	-	-	
KTP45	0.1 - 2.2	23	10	11	29	6.0	92	0.63	-	-	-	-	-	-	SC	-	-	
KTP46	1.0 - 2.3	33	15	17	32	8.5	90	0.54	1911	12.9	11	8	6	-	CL	-	-	
KTP47	0.4 - 2.2	35	20	23	40	12.0	85	0.69	-	-	-	-	-	-	CL	6.4	0.027	

7 GEOTECHNICAL ASSESSMENT

The purpose of this section is to evaluate the likely geohazards for the project area against the typical geotechnical constraints for urban development as identified by Partridge, Wood & Brink **(Ref.6)**. A geologic hazard is a natural geologic event that can endanger human lives and threaten infrastructure developments. Earthquakes, geomagnetic storms, landslides, sinkholes, tsunamis, volcanoes and even the less dramatic slower, but nevertheless costly problem soils (expansive, shrinking, collapsing, consolidating, dispersive [erosion], acidic), are all types of geologic hazards that can occur.

7.1 Expansive Soil Profile

Damage to structures erected on potentially active soils occurs where the expansiveness has not been determined and necessary remedial measures not employed. The potential expansiveness of a soil depends upon its clay content, the type of clay mineral present, its chemical composition and mechanical character. A material is potentially expansive if it exhibits the following properties:

- *Clay content of more than 12%.*
- *Plasticity index of more than 12.*
- *Liquid limit of more than 30%.*
- *Linear shrinkage of more than 8%.*

Based on fieldwork coupled with laboratory testing, the potential expansiveness for the soils encountered within the sites is generally "Low to Medium". The possibility of structural distress resulting from cyclic drying shrinkage in dry seasons and swell after wetting is therefore anticipated to be low to moderate.

7.2 Collapsible/Compressible Soil Profile

The site is generally characterised by transported clayey soils underlain residual sandy soils with low potential collapsibility. Furthermore, none of the horizons profiled show any typical characteristics associated with compressible soils.

7.3 Erodibility of Soil

The site is located in a region with the erodibility index of 9-15, thus indicating a Low to medium soil erodibility potential according to the national erodibility potential map presented in the National Housing Code (2009).

7.4 Excavation Classification

The excavatability of materials across the site has been evaluated according to the South African Bureau of Standards' Standardized Specification for Civil Engineering Construction classification for earthworks **(Ref.9)**. Soft excavation conditions in terms of the above standard are expected up to an average depth of 2.5 meters. In terms

of this classification and the in-situ soil/rock consistencies as profiled, the relationships given below are generally applicable:

- **Soft Excavation:** Material which can be efficiently removed by a back-acting excavator of flywheel power > 0, 10 kW for each mm of tined-bucket width
- **Intermediate Excavation:** Material which can be removed by a back-acting excavator having flywheel power > 0,10 kW for each mm of tined-bucket width or with the use of pneumatic tools before removal by a machine capable of removing soft material .
- **Hard Excavation:** Material that cannot be removed without blasting or wedging and splitting.

7.5 Undermined Ground

Subsidence in the order of centimetres and even metres, typically occurs in areas with large shallow (i.e.: at <180 m depth) underground cavities typically resulting from spatially extensive, shallow to very shallow mining (e.g.: coal, platinum) and also from dolomite/limestone dissolution over geological time.

The proposed Eskom sites are situated adjacent to several operational coal mines currently exploited for coal seams in excess of 45 meters by board-and-pillar mining. The undertaken percussion drilling revealed several cavities with the average thickness of 3 meters at depths between 45 – 74 meters corresponding to the average depth of the mined coal seams in the area. Although no signs of any on-going or historical subsidence features were observed, such events may be triggered by loss of support from the existing mining pillars, thus resulting in ground subsidence. Further studies focusing on undermining in the vicinity of Komati are therefore recommended in order to delineate and quantify the risks associated with undermined ground.

7.6 Areas Subject to Flooding

Surface run-off from the Eskom sites is generally by means of sheetwash towards the north and ultimately into northwest flowing streams traversing the Komati the region. It is the author's view that flood-lines may affect certain sections of the sites, particularly areas adjacent to streams and wetlands.

Due to the risk of flood leading to property damage and life hazards, it is recommended that flood line investigation is undertaken so as to delineate the 1: 50-year and 1: 100-year flood lines.

7.7 Steep Slopes and Unstable Natural Slopes

Slope stability is a function of slope height, slope face angle, soil and rock shear and cohesive properties, presence of unfavourably oriented discontinuities (joints, faults), plus external influences such as seismic accelerations, crest loading, and toe erosion (rivers, sea waves, man-made excavations).

There are no signs of the presence of unstable natural slope observed within the area. Furthermore, consequent shallow slope gradients encountered are insufficient to overcome resisting forces to initiate material movement under the action of gravity.

7.8 Instability of Soluble Rock

The site is not characterised by soluble rocks such as dolomite or limestone, therefore the formation of karst-related subsurface topography leading to sinkholes and subsidences is unlikely.

7.9 Seismic Activity

The seismic hazard map of South Africa indicates that the area generally lies within an area where there is a 10% probability that Peak Ground Accelerations of 0.12 g will be exceeded in 50 years, which corresponds to a seismic intensity of VII on the Modified Mercalli Scale (refer **Figure 7-1**).

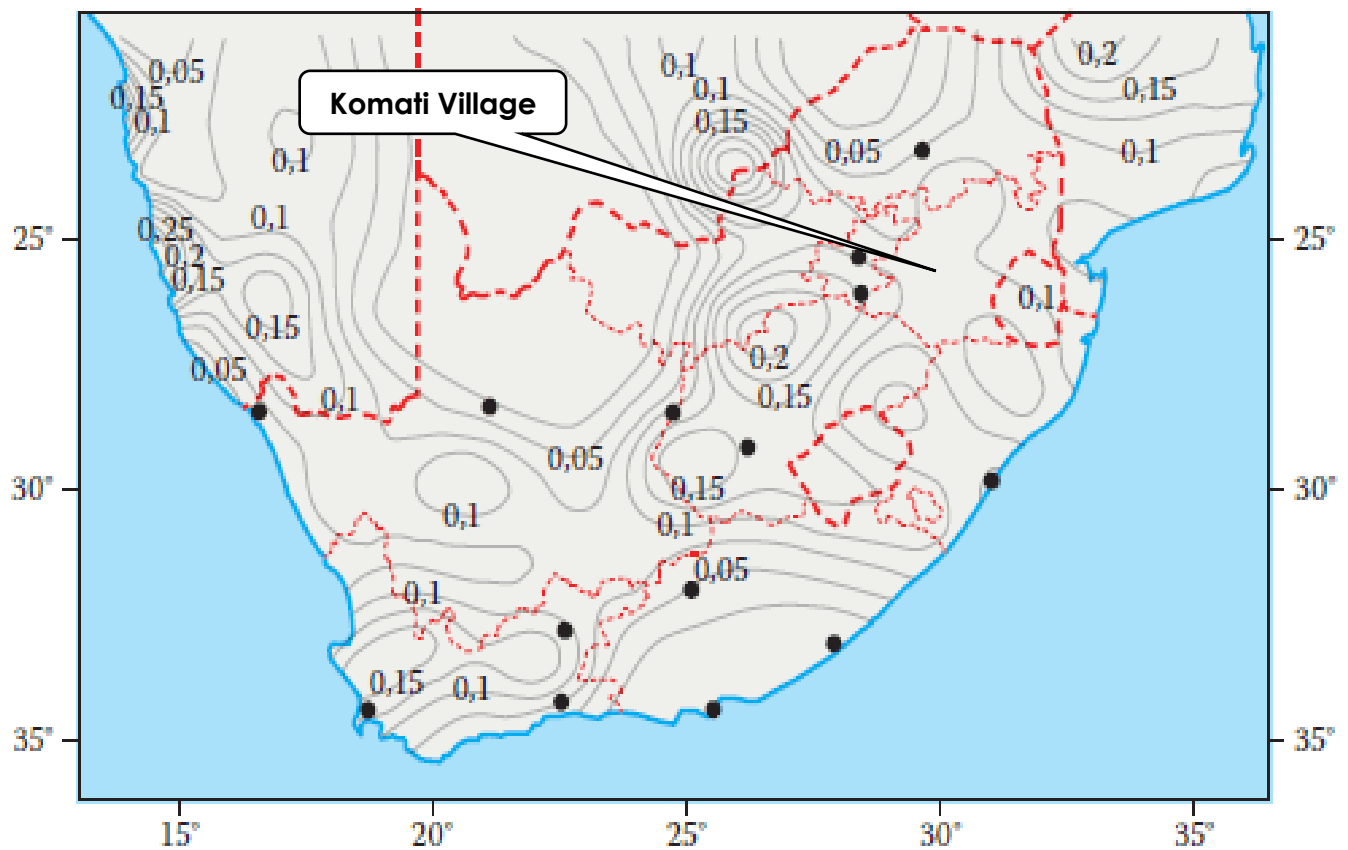


Figure 7-1: Seismic Hazard Map showing the Peak Ground Acceleration with 10% probability of being exceeded in a 50-year period.

Table 7-1: Geotechnical classification for urban development (Partridge et al. 1993).

CONSTRAINT		Most Favorable(1)	Intermediate (2)	Least favorable (3)
A	Collapsible Soil	Any collapsible horizon or consecutive horizons totaling depth of less than 750mm in thickness.*	Any collapsible horizon or consecutive horizons with a depth of more than 750 mm in thickness.	A least favorable* situation for this constraint does not occur.
B	Seepage	Permanent or perched water table more than 1,5m below ground surface	Permanent or perched water table less than 1,5m below ground surface.	Swamps and marshes
C	Active Soil	Low soil-heave potential predicted*	Moderate soil heave potential predicted.	High soil heave potential predicted
D	Highly compressible Soil	Low soil compressibility expected *	Moderate soil compressibility expected	High soil compressibility expected
E	Erodibility of soil	Low	Intermediate	High
F	Difficulty of excavation to 1.5m depth	Scattered or occasional boulders less than 10% of the total volume	Rock or hardpan pedocretes between 10 and 40% of the total volume	Rock or hardpan pedocretes more than 40% of the total volume.
G	Undermined ground –	Undermining at a depth greater than 100m below surface	Old undermined areas to a depth of 100m below surface where slope closure has ceased	Mining within less than 100m of surface or where extraction mining total has taken place.
H	Instability in areas of soluble rock - <u>No soluble rocks</u>	Possibly unstable	Probably unstable	Known sinkholes and dolines
I	Steep slopes	Between 2 and 6 degrees (all regions)	Slopes between 6 and 18 degrees and less than 2 degrees (Natal and Western Cape). Slopes between 6 and 12 degrees and less than 2 degrees	More than 18 degrees (Natal and Western Cape) More than 12 degrees (all other regions)
J	Areas of unstable natural slope	Low risk	Intermediate risk	High risk (especially in areas subject to seismic activity)
K	Areas subject to seismic activity	10% probability of an event less than 100 cm/s ² within 50 years	Mining-induced seismic activity more than 100cm/s ²	Natural seismic activity more than 100 cm/s ²
L	Areas subject to flooding.	A “most favorable” situation for this constraint does not occur.	Areas adjacent to a known drainage channel or floodplain with slope less than 1%	Areas within a known drainage channel or floodplain.

8 CONCLUSION AND RECOMMENDATIONS

BAV Consulting Pty Ltd was appointed by **Eskom Holdings SOC Limited** to conduct a preliminary geotechnical investigation in order to determine and assess the subsurface soil conditions at selected sites for the renewable energy project. The sites of investigation are situated within and/or near Komati Power Station on Eskom owned land within Steve Tshwete Local Municipality.

The proposed Eskom sites are situated adjacent to several operational coal mines currently exploited for coal seams in excess of 45 meters by either board and pillar or open-pit mining. The Blinkpan underground operation located just west of the Komati area which utilises a mechanised bord-and-pillar underground mining method confirmed that there's no past or on-going mining operation underneath the proposed site.

Komati Village and its neighbouring areas are regionally characterized by rocks of the Karoo Supergroup covered by Quaternary deposits. The Karoo Supergroup rocks in the area can be further classified as the Vryheid Formation of the Ecca Group. The rocks of the Vryheid Formation typically include sandstone, siltstone, shale and coal beds. These were later intruded by Jurassic dolerite dykes and sills.

There are no soluble rocks such as dolomite or limestone underlying the sites, therefore, the presence of karst-related subsurface topography leading to the formation of sinkholes and subsidence features is unlikely. However, the area is known to have been previously undermined, therefore further studies should be undertaken to assess possible land-subsidence events resulting from the mobilisation of overburden material into underlying abandoned mines.

A total of twenty-five (25) boreholes were drilled at selected positions at Solar PV Site 1. The borehole drilling generally revealed transported soils, underlain by residual sandstone, sandstone interlayered with siltstone, shale, shaly coal, and coal, and diamictite, respectively. Some intrusive material interpreted as dolerite sills were encountered at variable depths.

Underground cavities were intercepted at various locations at depths in excess of 45 meters corresponding with layers of coal, thus pointing to previous mining activities at that depth. The drilling further revealed that the cavities were filled with groundwater, however, the quantity is unknown. Therefore, groundwater yield testing is recommended in order to estimate the Storativity (S) and Specific yield (Sy) of the underlying cavities. Note that groundwater strikes were generally shallower than the cavities in the majority of boreholes, thus, a leaky aquifer situation may exist at Solar PV Site 1.

The stability of the sites mainly depends on the type and thickness of the overburden material, degree of weathered, and to a certain extent groundwater level drawdown. The overburden material consists of alternating layers of slightly

weathered to highly weathered, shale, siltstone, sandstone and dolerite sills with the estimated total thickness in excess of 45 meters. Thus, the likelihood of the formation of sinkhole or subsidence features at Solar PV Site 1 is **low**, owing to the type of overburden and the thickness thereof.

The likelihood of sinkhole and subsidence formations at Solar PV Site 2 was not evaluated. Therefore, additional investigations such as percussion drilling, rotary core drilling, SPT testing are recommended in order to assess the foundation parameters and the long-term stability of the site. The implementation of photovoltaics and battery energy storage systems is provisionally supported at Solar PV Site 1, 2 and all BESS sites, subject to additional investigations.

Additional percussion drilling is recommended at Solar PV Site 1 and 2 in order to accurately confirm and delineate areas affected by previous mining activities. Furthermore, rotary core drilling and associated strength tests such as SPT, UCS, pile tests, etc. are recommended at the footprint of the proposed structures.

The presence of water in the cavities may not necessarily negate the durability and strength of the pillars, but sudden excessive groundwater level drawdown without rehabilitation may induce roof collapse leading to sinkhole and subsidences on surface.

Conditions prevailing on site suggest that no problems are foreseen for the development of agrivoltaics, wind turbines and a BESS system, provided that the contents of this report are acknowledged, and recommendations, as outlined in the report, are adhered to. Note that heavy loaded structures such as the proposed wind turbines are not recommended at Solar PV Site 1 due to the presence of cavities that can manifest into subsidence or sinkholes triggered by continued weathered of overburden material, groundwater level fluctuations, and collapse of underground mine pillars. It recommended that formal mining layouts or documents be sourced from the neighbouring mines.

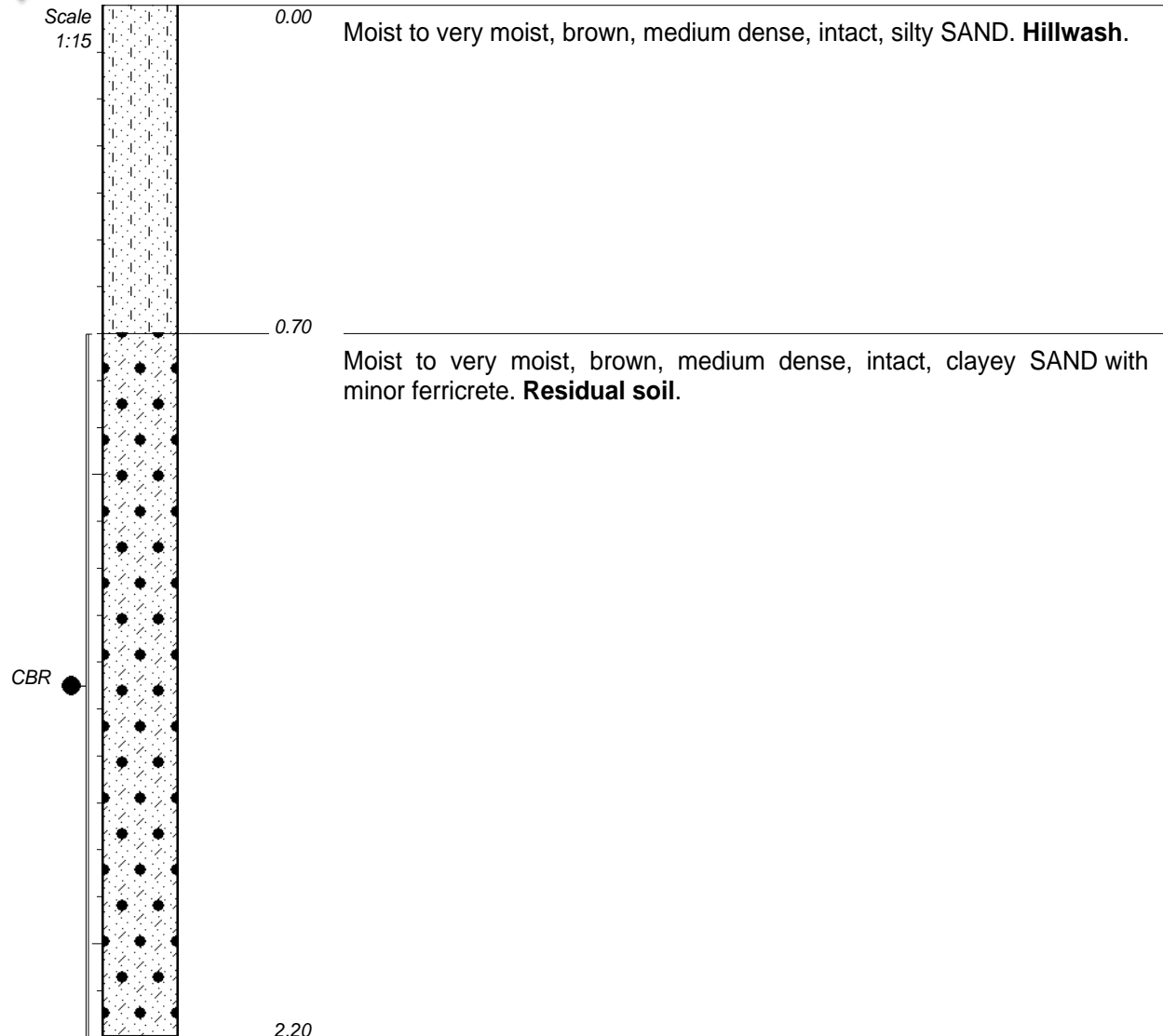
The comments and recommendations contained in this report are based on a limited number of data points. It is, therefore recommended that all excavations and foundation be inspected by a geotechnical engineer or engineering geologist to verify that the founding conditions are not at variance with those described herein.

9 REFERENCES

- 1) Acocks JPH (1988) Veld types of South Africa. (Third edition) *Mem. Bot. Surv. S. Afr.* No. 57: 1-146
- 2) Basson J.J. (1989). Deterioration of concrete in aggressive waters – Measuring aggressiveness and taking countermeasures. Concrete Durability Bureau of the Portland Cement Institute. Midrand, South Africa.
- 3) Brink, A.B.A. and Bruin, R.M.H. (2002). *Guidelines for soil and rock logging in South Africa*, 2nd impression, 2002.
- 4) Jennings JE., Brink, AAB. and Williams, A.A.B. 1973. Revised guide to soil profiling for civil engineering purposes in South Africa. *The Civil Engineer in South Africa*, Vol. 15, January 1973, pp. 3-12.
- 5) Johnson, M.R., Anhaeusser, C.R. And Thomas, R.J. (Eds.) (2006). *The geology of South Africa*. Geological Society of South Africa. Johannesburg/Council for Geoscience, Pretoria, 691 pp.
- 6) Partridge T.C., Wood C.K. and Brink A.B.A. (1993). Priorities for urban expansion within the PWV metropolitan region. The primacy of geotechnical constraints. *South African Geographical Journal*: Vol. 75, pp 9 – 13.
- 7) South African Institution of Civil Engineering (SAICE) - Geotechnical Division, (2010). *Site Investigation Code of Practice*, 1st Edition.
- 8) SANS 1936, Development on dolomite land -Part 1: (2012) *General Principles and Requirements*.
- 9) South African Bureau of Standards SANS 1200D. *Standardised specification for civil engineering construction Section D: Earthworks*. Pretoria 2002.
- 10) Van der Merwe, D.H. 1964 The Prediction of heave from the Plasticity Index and Percentage Clay Fraction. *Trans. S.A. Ins. Civ. Eng.* No.6, pp103-107.
- 11) Weinert, HH 1980. *The Natural Road Construction Materials of Southern Africa*. H & R Academia Publ., Pretoria, 298 pp.

APPENDIX A: SOIL PROFILES

(SANS 633: 2012-Soil Profiling and Rotary percussion borehole drilling on dolomite land in Southern Africa for engineering purposes)



NOTES

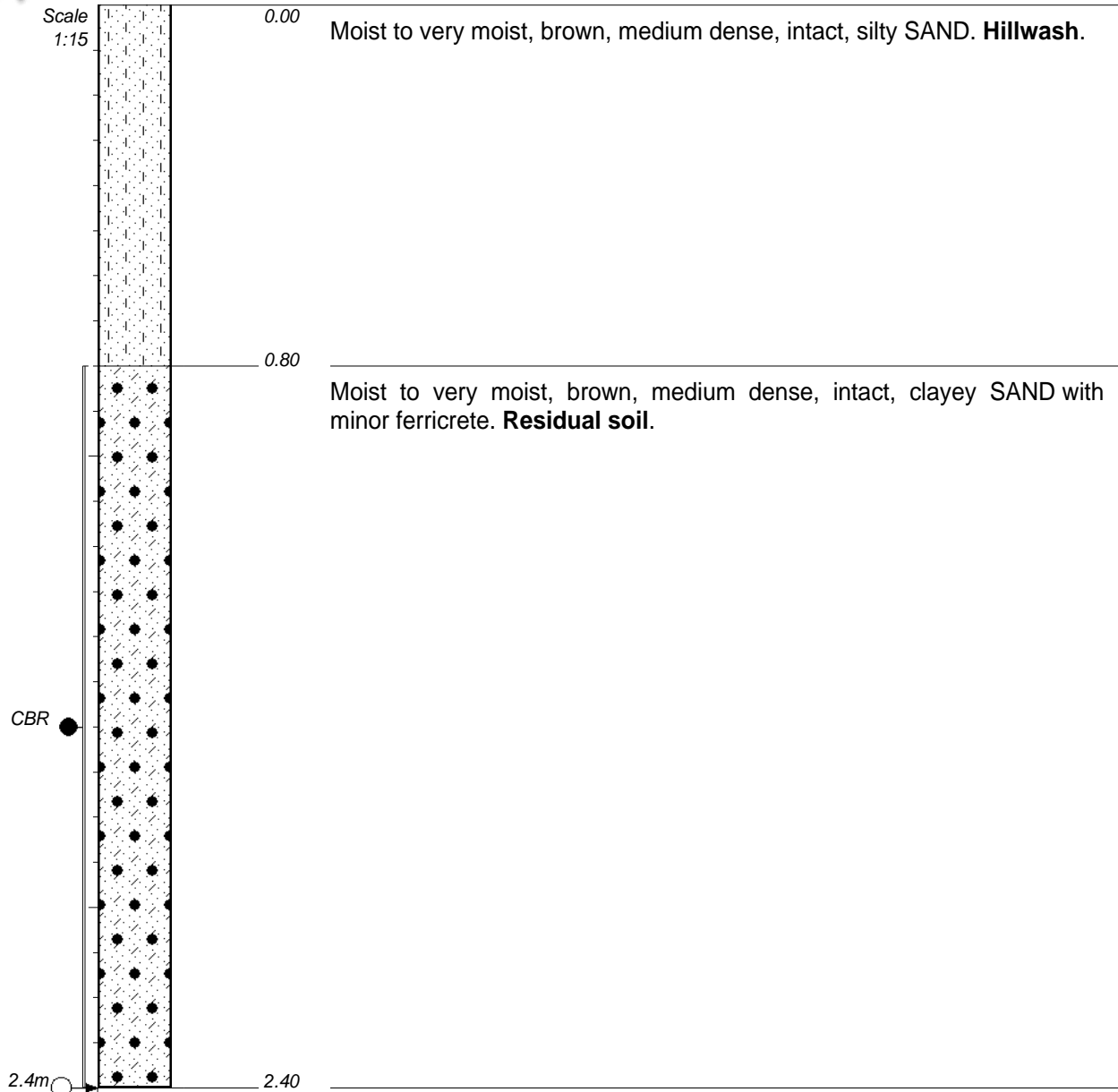
- 1) Excavation stopped at 2.2m due to required depth.
- 2) No groundwater seepage.
- 3) Sidewall stable.
- 4) Disturbed sample at 0.7--2.2m: CBR, F-IND.

CONTRACTOR :
MACHINE : BELL TLB
DRILLED BY : Xolani Shabangu
PROFILED BY : Andries Vukeya
TYPE SET BY : Andries Vukeya
SETUP FILE : STANDARD.SET

INCLINATION :
DIAM :
DATE : 22-08-2023
DATE : 20-09-2023
DATE : 06/12/2023 13:50
TEXT : ..ppendixASoilProfiles.txt

ELEVATION : 1614 m
X-COORD : 29.46031°E
Y-COORD : 26.09103°S

HOLE No: KTP1



NOTES

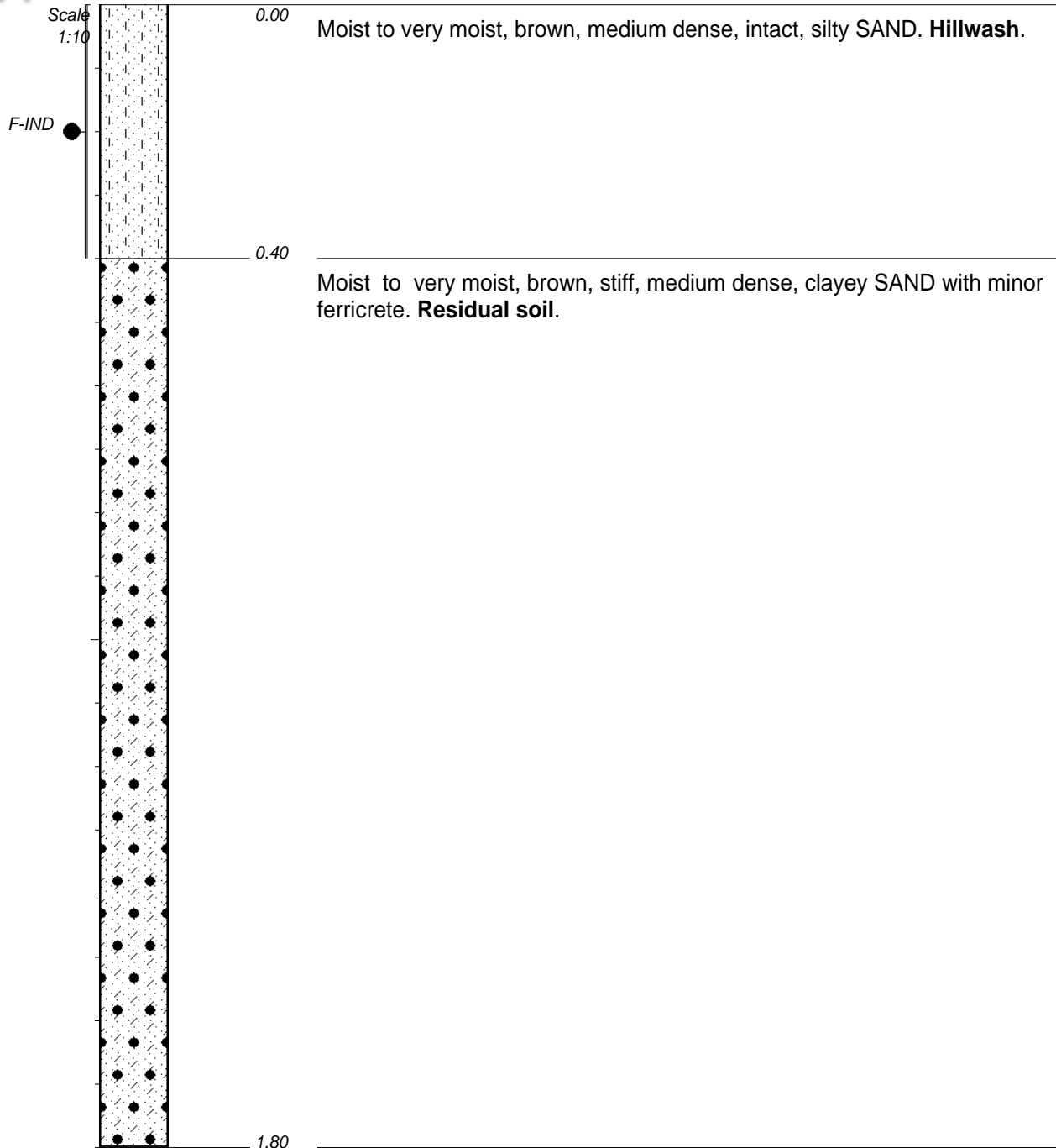
- 1) Excavation stopped at 2.4m due to required depth.
- 2) Groundwater seepage at 2.4m.
- 3) Sidewall stable.
- 4) Disturbed sample at 0.8--2.4m: CBR, F-IND.

CONTRACTOR :
MACHINE : BELL TLB
DRILLED BY : Xolani Shabangu
PROFILED BY : Andries Vukeya
TYPE SET BY : Andries Vukeya
SETUP FILE : STANDARD.SET

INCLINATION :
DIAM :
DATE : 22-08-2023
DATE : 20-09-2023
DATE : 06/12/2023 13:50
TEXT : ..ppendixASoilProfiles.txt

ELEVATION : 1614 m
X-COORD : 29.45981°E
Y-COORD : 26.09147°S

HOLE No: KTP2



NOTES

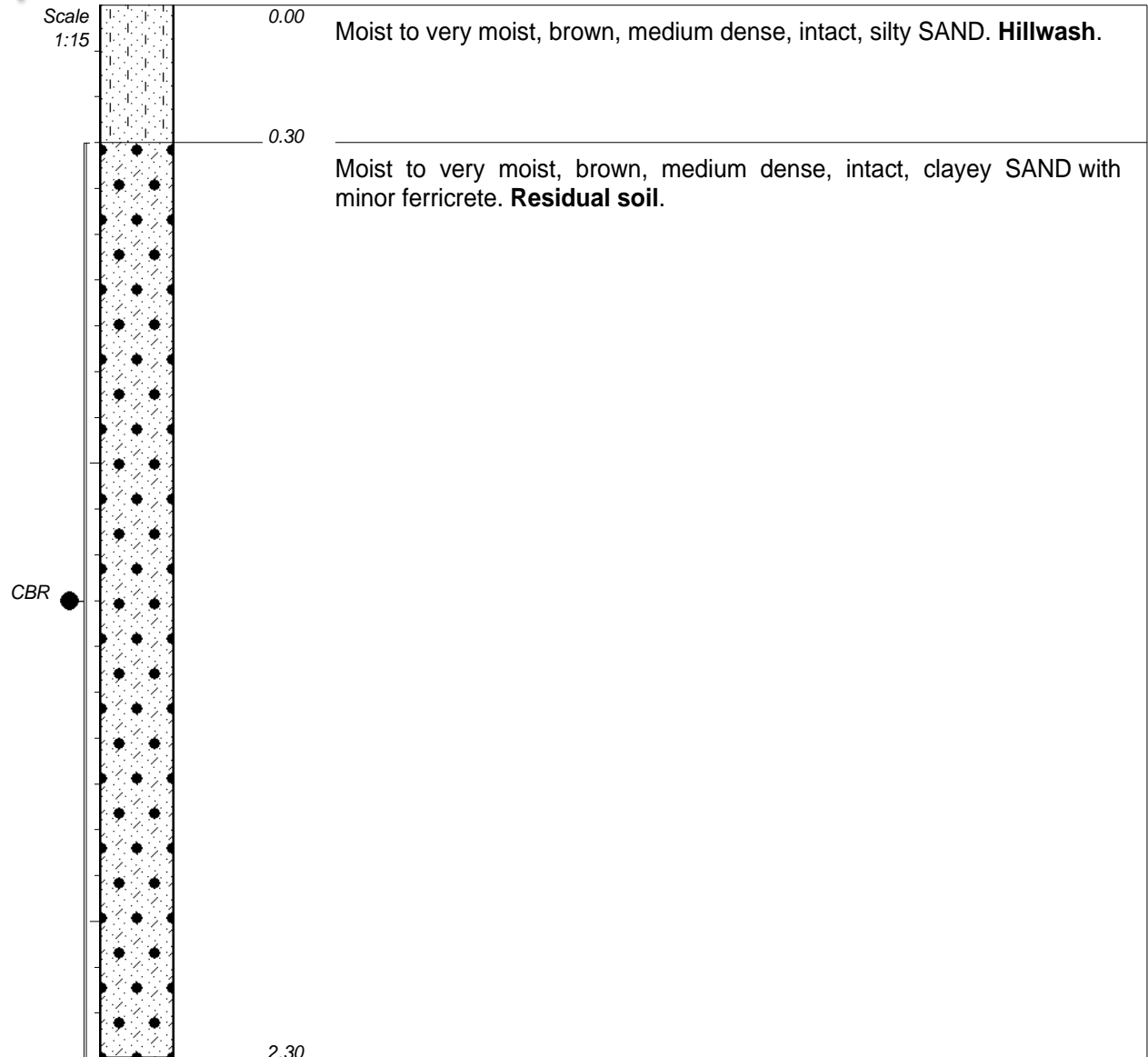
- 1) Excavation stopped at 1.8m due to slow progress.
- 2) No groundwater seepage.
- 3) Sidewall stable.
- 4) Disturbed sample at 0.0--0.4m: F-IND.

CONTRACTOR :
MACHINE : BELL TLB
DRILLED BY : Xolani Shabangu
PROFILED BY : Andries Vukeya
TYPE SET BY : Andries Vukeya
SETUP FILE : STANDARD.SET

INCLINATION :
DIAM :
DATE : 22-08-2023
DATE : 20-09-2023
DATE : 06/12/2023 13:50
TEXT : ..ppendixASoilProfiles.txt

ELEVATION : 1611 m
X-COORD : 29.45872°E
Y-COORD : 26.09211°S

HOLE No: KTP3



NOTES

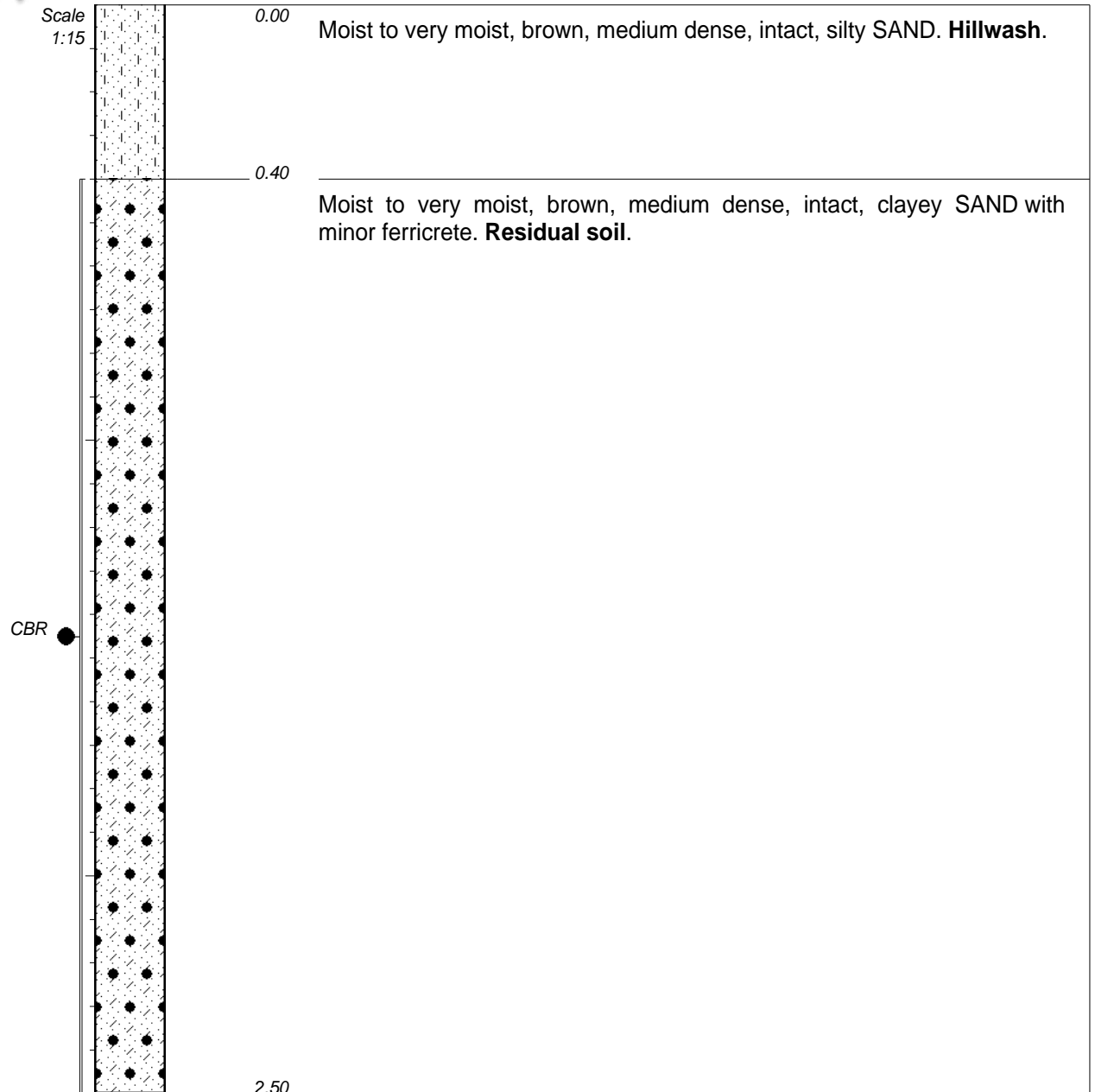
- 1) Excavation stopped at 2.3m due to required depth.
- 2) No groundwater seepage.
- 3) Sidewall stable.
- 4) Disturbed sample at 0.3--2.3m: CBR, F-IND.

CONTRACTOR :
MACHINE : BELL TLB
DRILLED BY : Xolani Shabangu
PROFILED BY : Andries Vukeya
TYPE SET BY : Andries Vukeya
SETUP FILE : STANDARD.SET

INCLINATION :
DIAM :
DATE : 22-08-2023
DATE : 20-09-2023
DATE : 06/12/2023 13:50
TEXT : ..ppendixASoilProfiles.txt

ELEVATION : 1607 m
X-COORD : 29.45752°E
Y-COORD : 26.09249°S

HOLE No: KTP4



NOTES

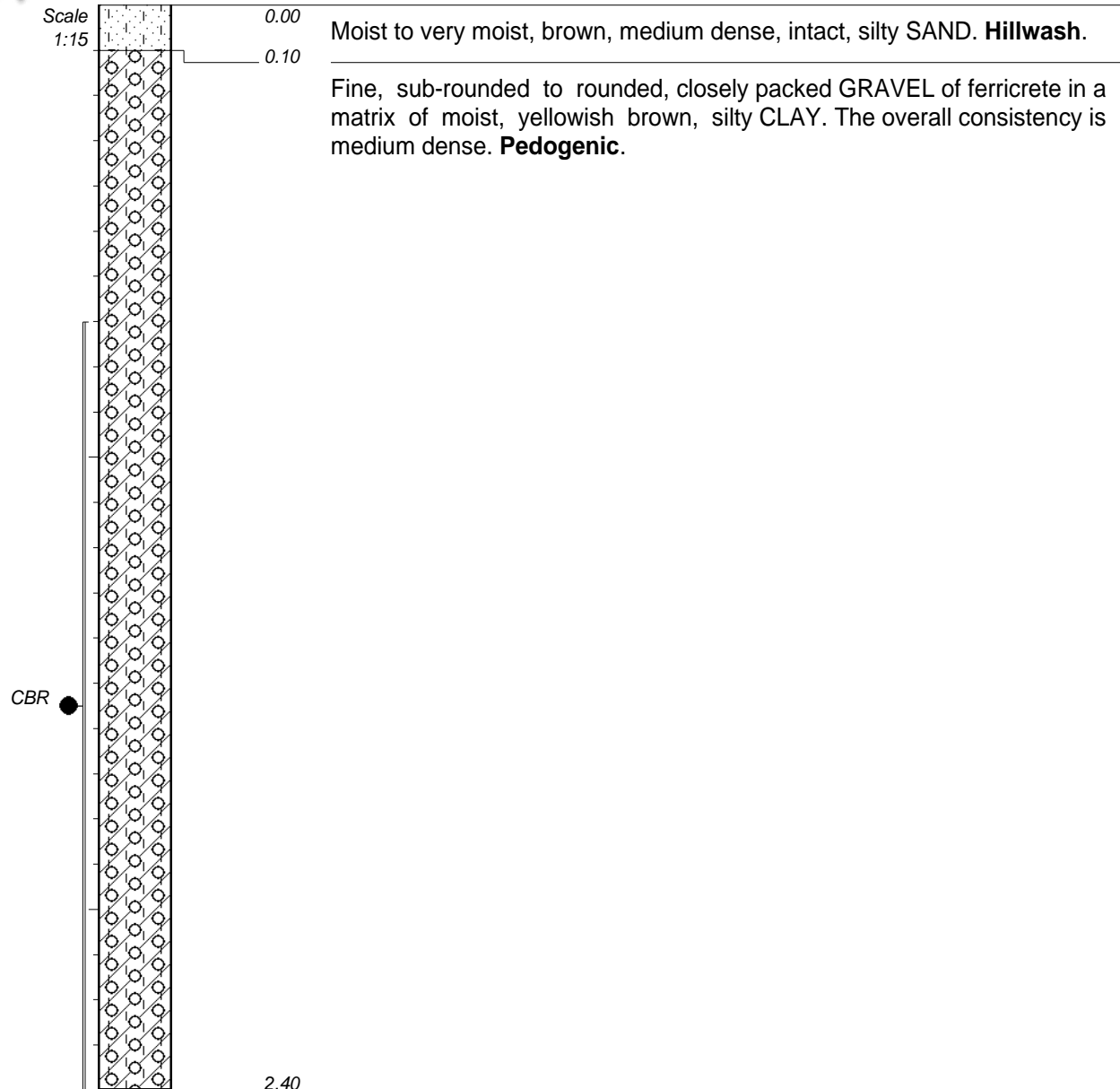
- 1) Excavation stopped at 2.5m due to required depth.
- 2) No groundwater seepage.
- 3) Sidewall stable.
- 4) Disturbed sample at 0.4--2.5m: CBR, F-IND.

CONTRACTOR :
MACHINE : BELL TLB
DRILLED BY : Xolani Shabangu
PROFILED BY : Andries Vukeya
TYPE SET BY : Andries Vukeya
SETUP FILE : STANDARD.SET

INCLINATION :
DIAM :
DATE : 22-08-2023
DATE : 20-09-2023
DATE : 06/12/2023 13:50
TEXT : ..ppendixASoilProfiles.txt

ELEVATION : 1608 m
X-COORD : 29.45654°E
Y-COORD : 26.09342°S

HOLE No: KTP5


NOTES

- 1) Excavation stopped at 2.4m due to required depth.
- 2) No groundwater seepage.
- 3) Sidewall stable.
- 4) Disturbed sample at 0.7--2.4m: CBR, F-IND.

CONTRACTOR :
 MACHINE : BELL TLB
 DRILLED BY : Xolani Shabangu
 PROFILED BY : Andries Vukeya
 TYPE SET BY : Andries Vukeya
 SETUP FILE : STANDARD.SET

INCLINATION :
 DIAM :
 DATE : 22-08-2023
 DATE : 20-09-2023
 DATE : 06/12/2023 13:50
 TEXT : ..ppendixASoilProfiles.txt

ELEVATION : 1617 m
 X-COORD : 29.45528°E
 Y-COORD : 26.09348°S

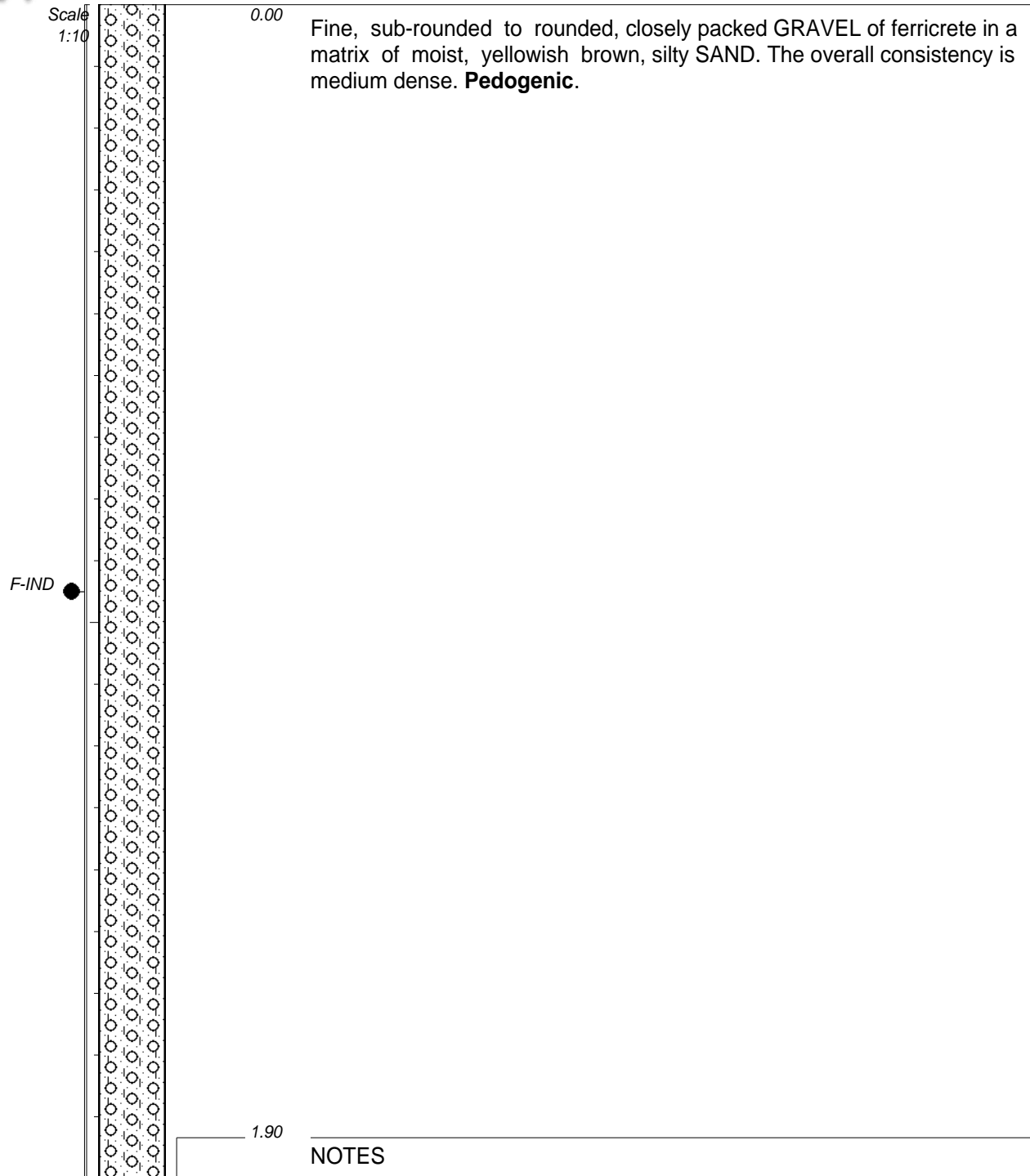
HOLE No: KTP6



ESKOM
PRELIMINARY GEOTECHNICAL INVESTIGATION:
KOMATI POWER STATION

HOLE No: KTP7
Sheet 1 of 1

JOB NUMBER: MAK152.08.23



NOTES

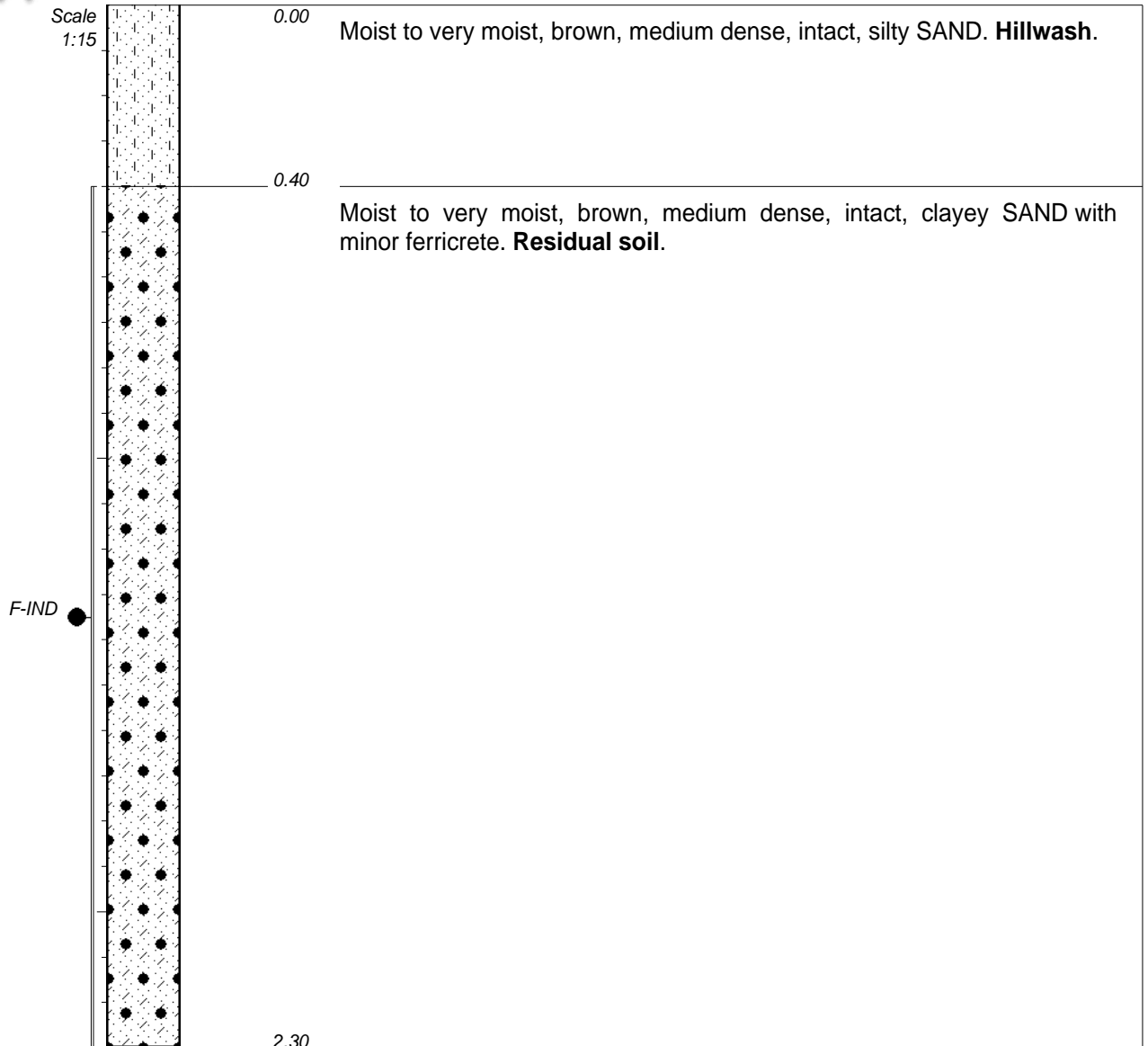
- 1) Excavation stopped at 1.9m due to required depth
- 2) No groundwater seepage.
- 3) Sidewall stable.
- 4) Disturbed sample at 0.0--1.9m: F-IND.

CONTRACTOR :
MACHINE : BELL TLB
DRILLED BY : Xolani Shabangu
PROFILED BY : Andries Vukeya
TYPE SET BY : Andries Vukeya
SETUP FILE : STANDARD.SET

INCLINATION :
DIAM :
DATE : 22-08-2023
DATE : 20-09-2023
DATE : 06/12/2023 13:50
TEXT : ..ppendixASoilProfiles.txt

ELEVATION : 1623 m
X-COORD : 29.45366°E
Y-COORD : 26.09448°S

HOLE No: KTP7



NOTES

- 1) Excavation stopped at 2.3m due to required depth.
- 2) No groundwater seepage.
- 3) Sidewall stable.
- 4) Disturbed sample at 0.4--2.3m: F-IND.

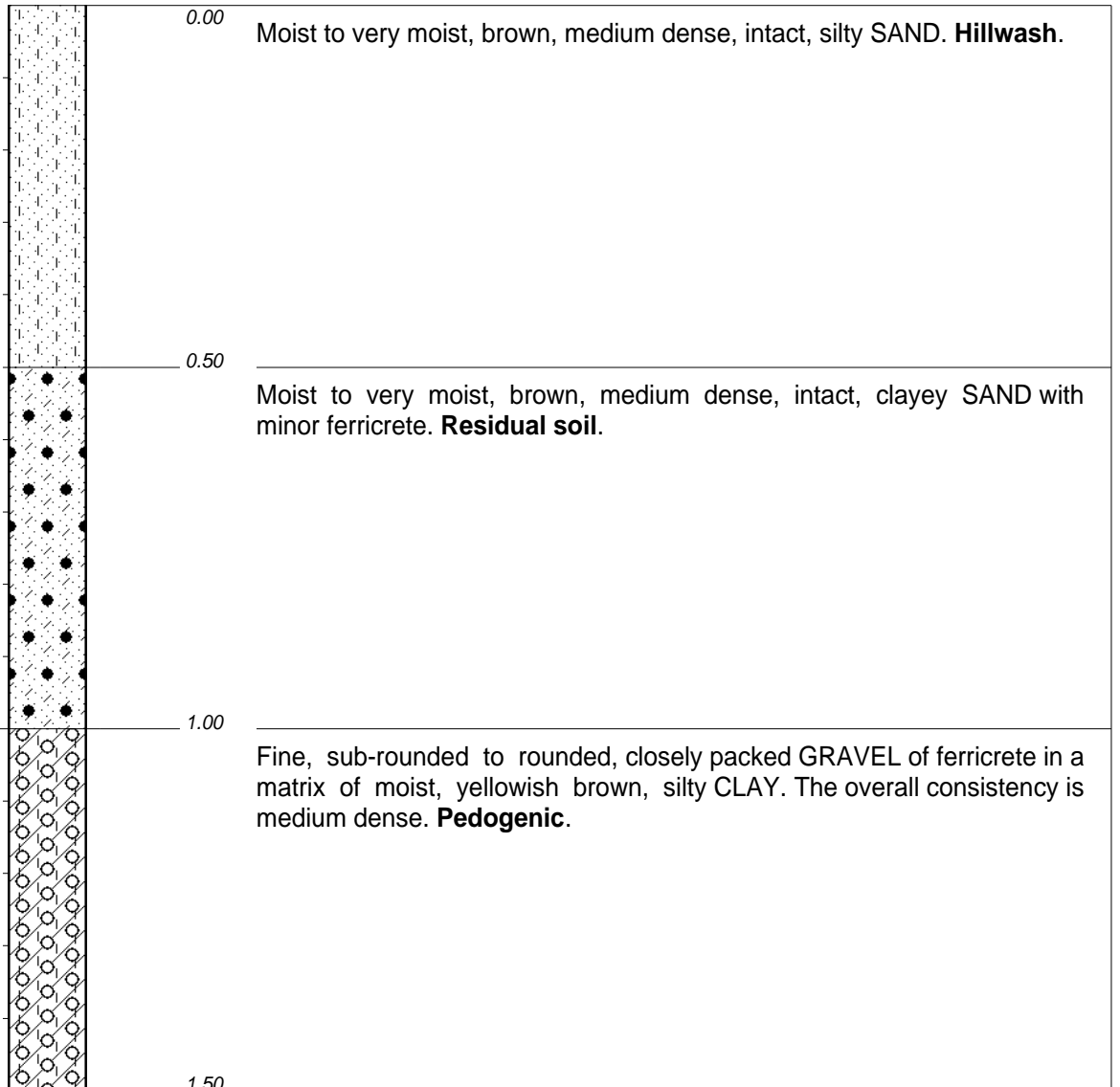
CONTRACTOR :
MACHINE : BELL TLB
DRILLED BY : Xolani Shabangu
PROFILED BY : Andries Vukeya
TYPE SET BY : Andries Vukeya
SETUP FILE : STANDARD.SET

INCLINATION :
DIAM :
DATE : 22-08-2023
DATE : 20-09-2023
DATE : 06/12/2023 13:50
TEXT : ..ppendixASoilProfiles.txt

ELEVATION : 1620 m
X-COORD : 29.45200°E
Y-COORD : 22.09541°S

HOLE No: KTP8

Scale
1:10



NOTES

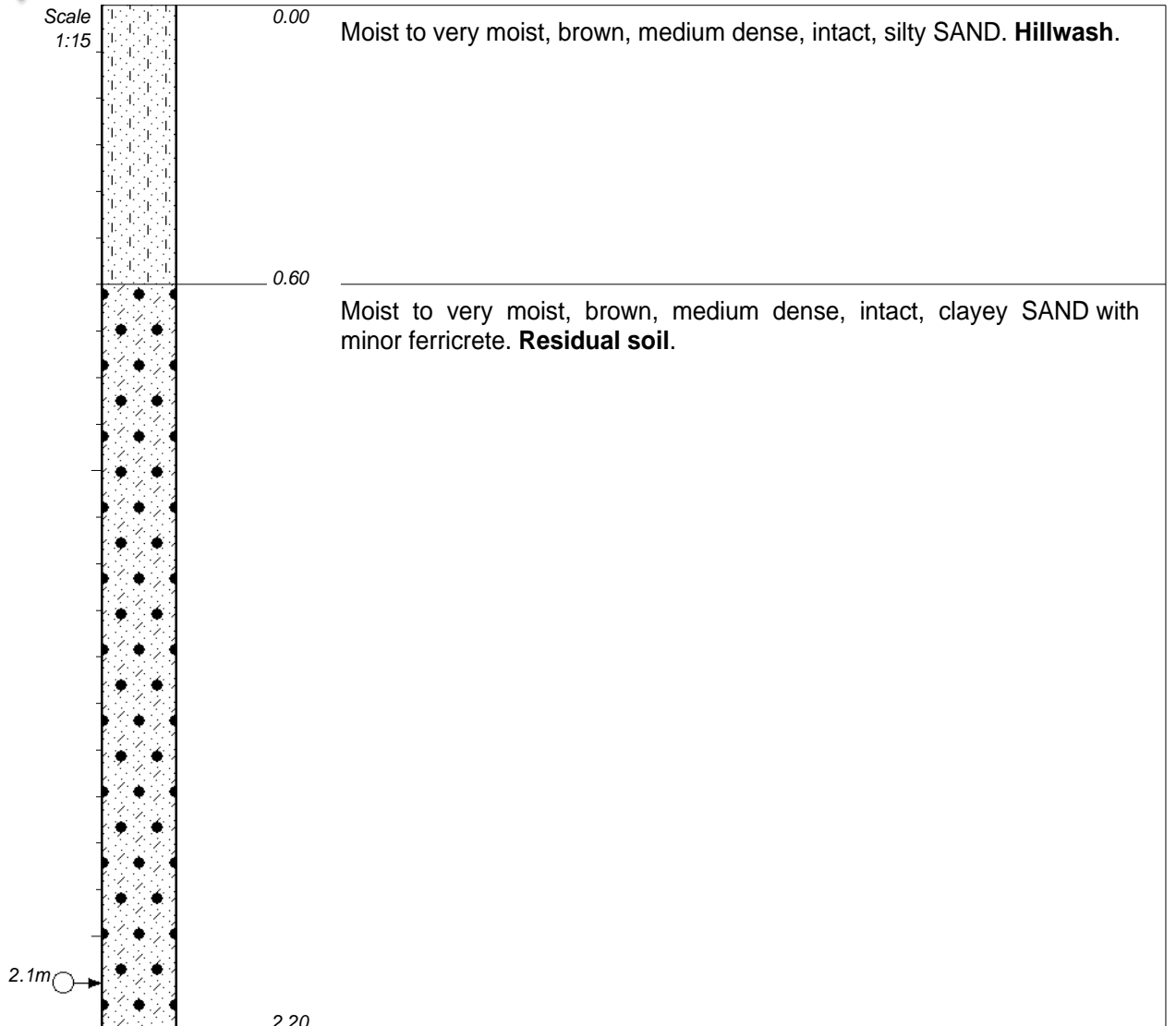
- 1) Excavation stopped at 1.5 m due to slow progress.
- 2) No groundwater seepage.
- 3) Sidewall stable.

CONTRACTOR :
MACHINE : **BELL TLB**
DRILLED BY : **Xolani Shabangu**
PROFILED BY : **Andries Vukeya**
TYPE SET BY : **Andries Vukeya**
SETUP FILE : **STANDARD.SET**

INCLINATION :
DIAM :
DATE : **22-08-2023**
DATE : **20-09-2023**
DATE : **06/12/2023 13:50**
TEXT : **..ppendixASoilProfiles.txt**

ELEVATION : **1621 m**
X-COORD : **29.45013°E**
Y-COORD : **26.09517°S**

HOLE No: KTP9



NOTES

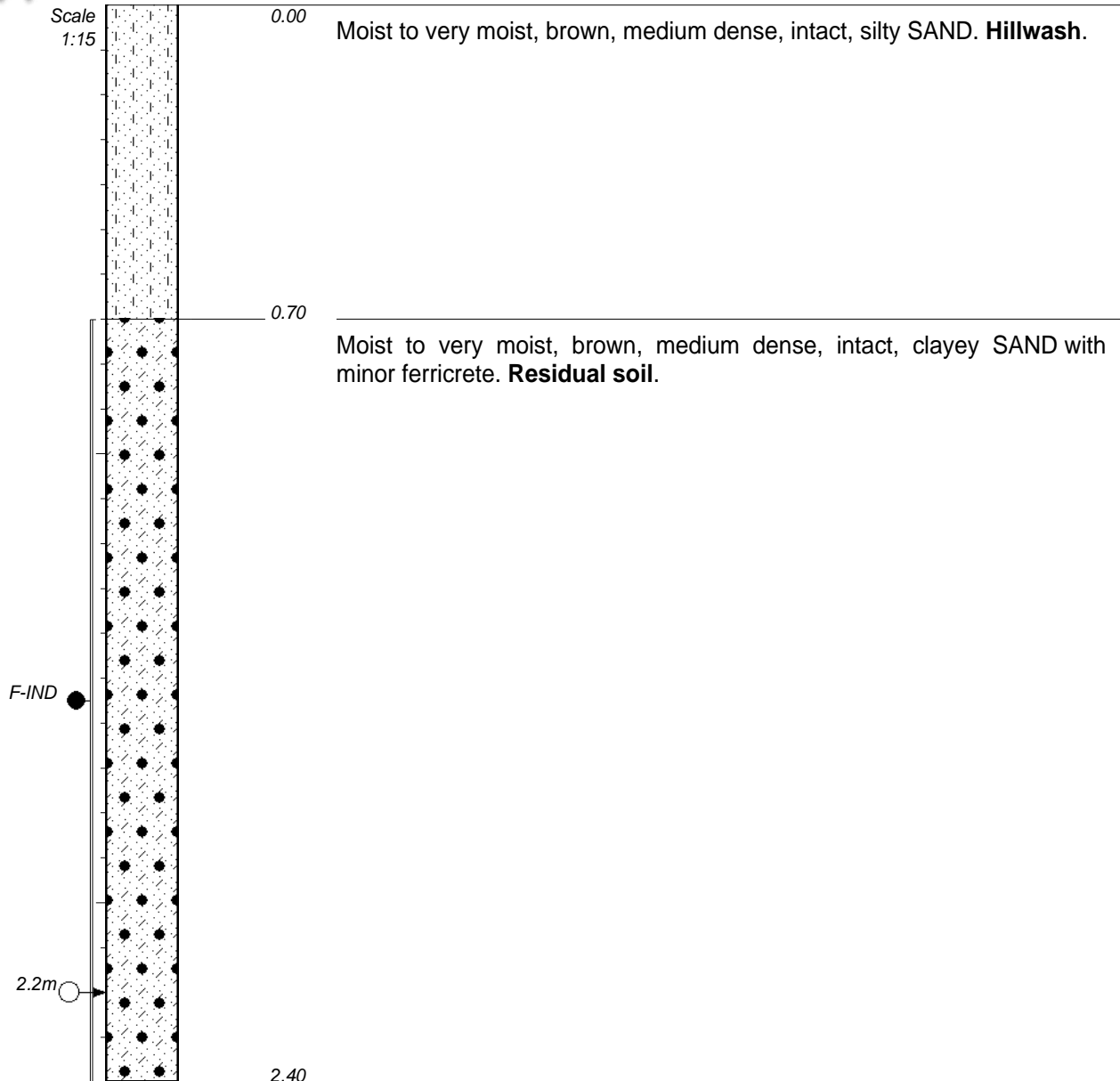
- 1) Excavation stopped at 2.2m due to required depth.
- 2) Groundwater seepage at 2.1m.
- 3) Sidewall stable.

CONTRACTOR :
MACHINE : BELL TLB
DRILLED BY : Xolani Shabangu
PROFILED BY : Andries Vukeya
TYPE SET BY : Andries Vukeya
SETUP FILE : STANDARD.SET

INCLINATION :
DIAM :
DATE : 22-08-2023
DATE : 20-09-2023
DATE : 06/12/2023 13:50
TEXT : ..ppendixASoilProfiles.txt

ELEVATION : 1614 m
X-COORD : 29.45136°E
Y-COORD : 26.09451°S

HOLE No: KTP10



NOTES

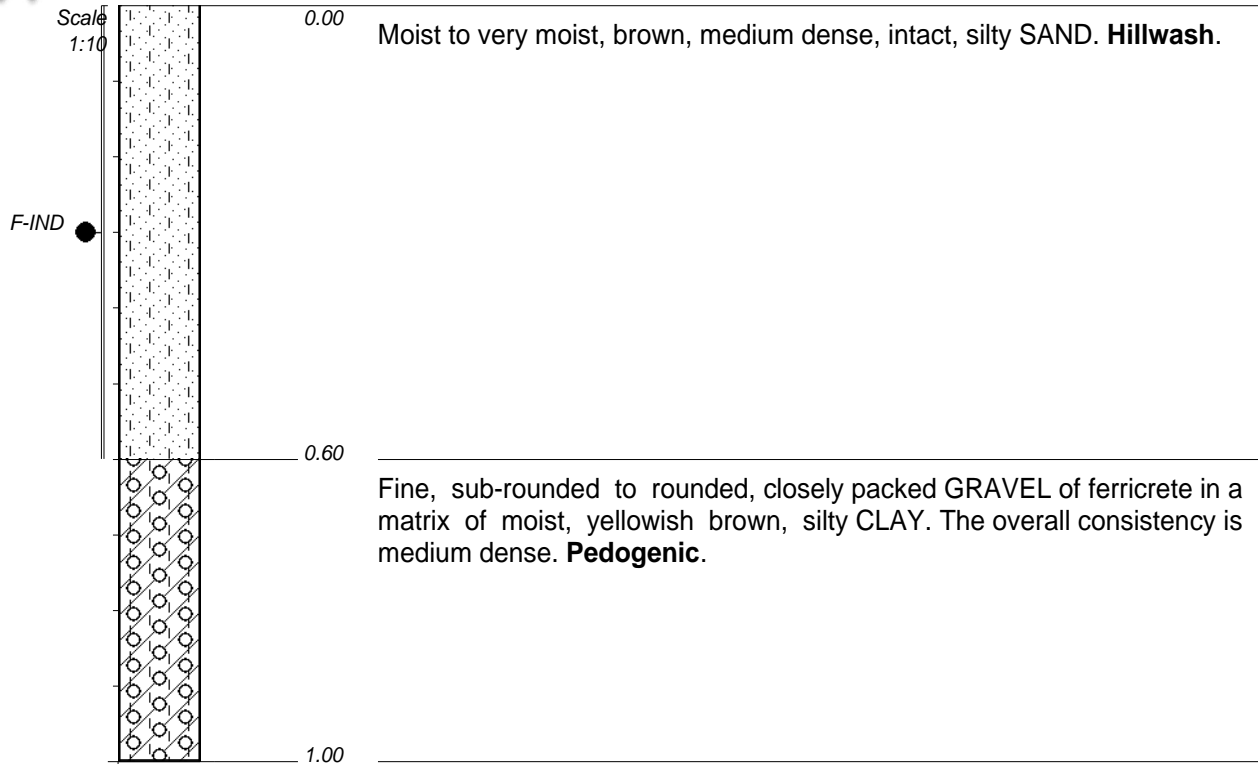
- 1) Excavation stopped at 2.4m due required depth.
- 2) Groundwater seepage at 2.2m.
- 3) Sidewall stable.
- 4) Disturbed sample at 0.7--2.4m: F-IND.

CONTRACTOR :
MACHINE : BELL TLB
DRILLED BY : Xolani Shabangu
PROFILED BY : Andries Vukeya
TYPE SET BY : Andries Vukeya
SETUP FILE : STANDARD.SET

INCLINATION :
DIAM :
DATE : 22-08-2023
DATE : 20-09-2023
DATE : 06/12/2023 13:50
TEXT : ..ppendixASoilProfiles.txt

ELEVATION : 1617 m
X-COORD : 29.45266°E
Y-COORD : 26.09395°S

HOLE No: KTP11



NOTES

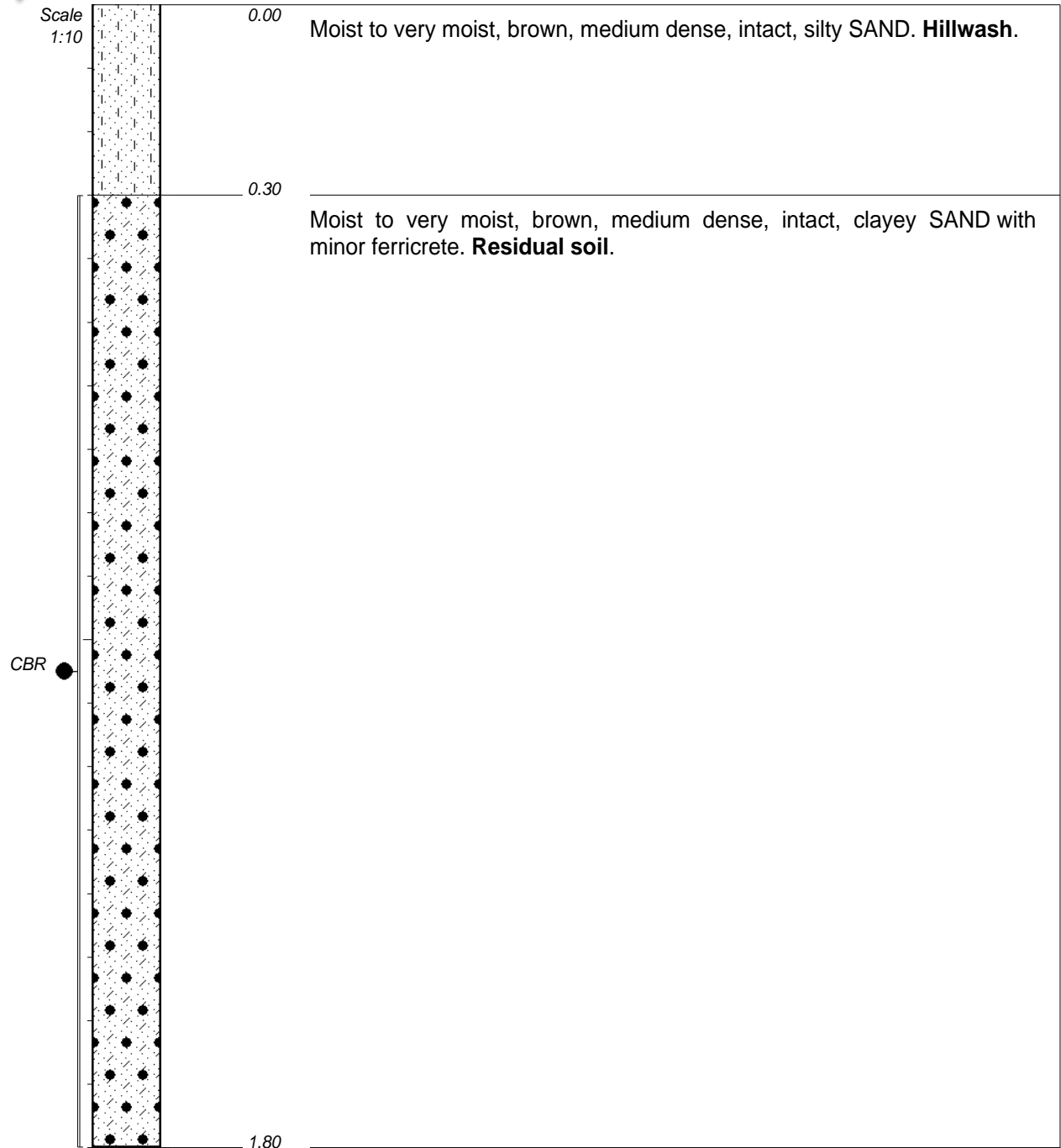
- 1) Excavation stopped at 1.0m due to refusal on hardpan ferricrete.
- 2) No groundwater seepage.
- 3) Sidewall stable.
- 4) Disturbed sample at 0.0--0.6m: F-IND.

CONTRACTOR :
MACHINE : BELL TLB
DRILLED BY : Xolani Shabangu
PROFILED BY : Andries Vukeya
TYPE SET BY : Andries Vukeya
SETUP FILE : STANDARD.SET

INCLINATION :
DIAM :
DATE : 22-08-2023
DATE : 20-09-2023
DATE : 06/12/2023 13:50
TEXT : ..ppendixASoilProfiles.txt

ELEVATION : 1616 m
X-COORD : 29.45473°E
Y-COORD : 26.09299°S

HOLE No: KTP12



NOTES

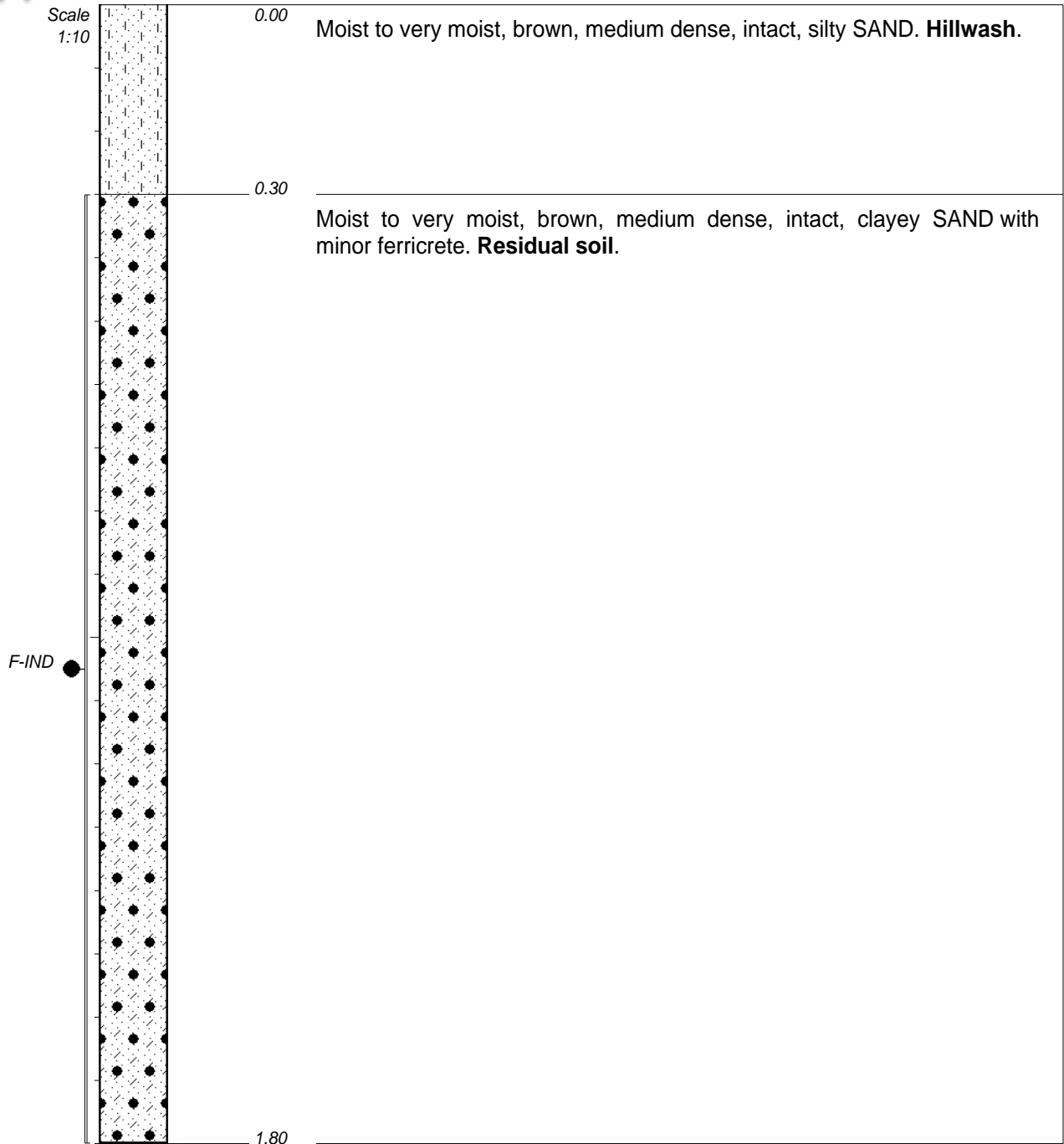
- 1) Excavation stopped at 1.8m due to required depth.
- 2) No groundwater seepage.
- 3) Sidewall stable.
- 4) Disturbed sample at 0.3--1.8m: CBR, F-IND.

CONTRACTOR :
MACHINE : BELL TLB
DRILLED BY : Xolani Shabangu
PROFILED BY : Andries Vukeya
TYPE SET BY : Andries Vukeya
SETUP FILE : STANDARD.SET

INCLINATION :
DIAM :
DATE : 22-08-2023
DATE : 20-09-2023
DATE : 06/12/2023 13:50
TEXT : ..ppendixASoilProfiles.txt

ELEVATION : 1612 m
X-COORD : 29.45693°E
Y-COORD : 26.09207°S

HOLE No: KTP13



NOTES

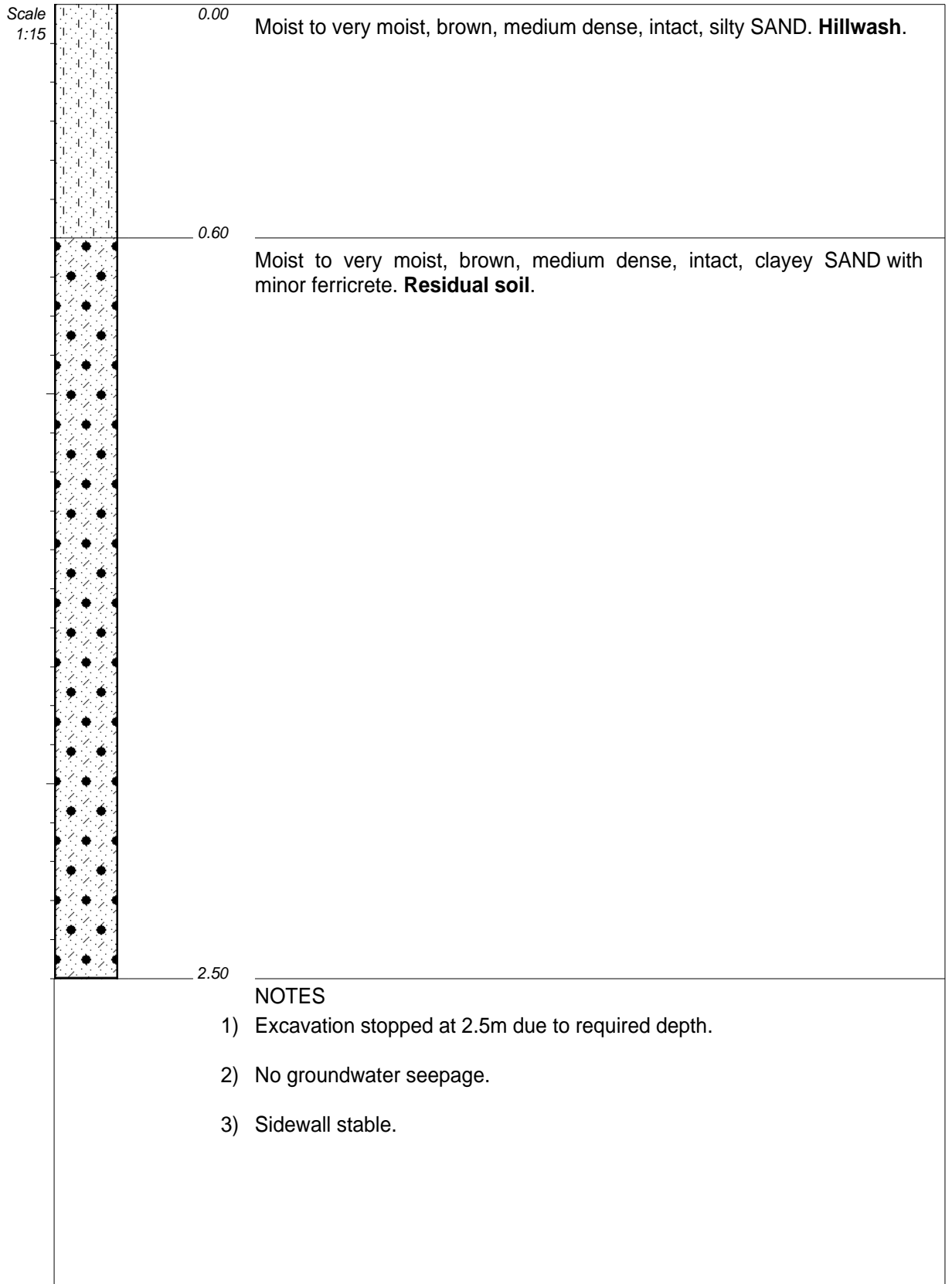
- 1) Excavation stopped at 1.8m due to required depth.
- 2) No groundwater seepage.
- 3) Sidewall stable.
- 4) Disturbed sample at 0.3--1.8m: F-IND.

CONTRACTOR :
MACHINE : BELL TLB
DRILLED BY : Xolani Shabangu
PROFILED BY : Andries Vukeya
TYPE SET BY : Andries Vukeya
SETUP FILE : STANDARD.SET

INCLINATION :
DIAM :
DATE : 22-08-2023
DATE : 20-09-2023
DATE : 06/12/2023 13:50
TEXT : ..ppendixASoilProfiles.txt

ELEVATION : 1615 m
X-COORD : 29.45912°E
Y-COORD : 26.09101°S

HOLE No: KTP14

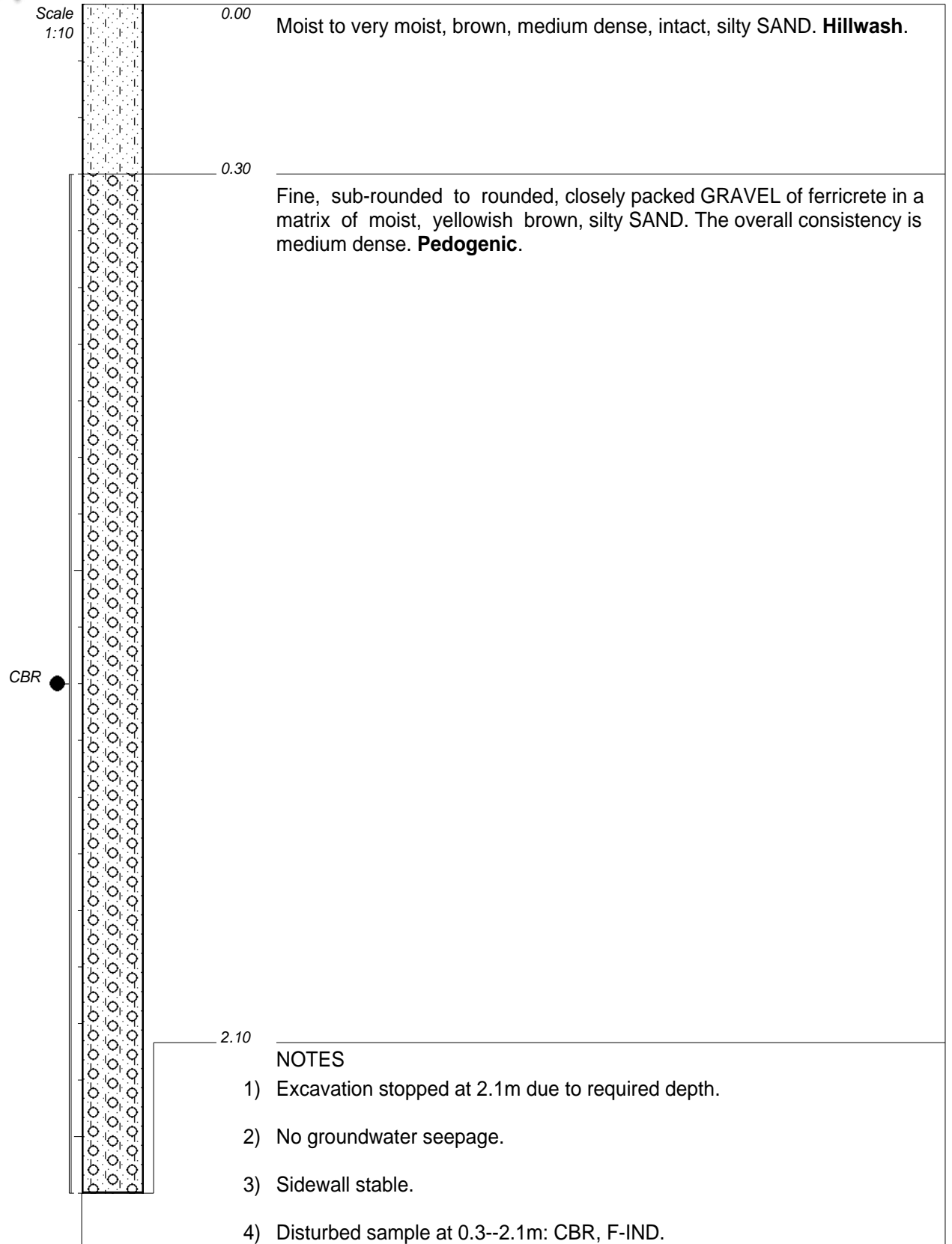


CONTRACTOR :
MACHINE : BELL TLB
DRILLED BY : Xolani Shabangu
PROFILED BY : Andries Vukeya
TYPE SET BY : Andries Vukeya
SETUP FILE : STANDARD.SET

INCLINATION :
DIAM :
DATE : 22-08-2023
DATE : 20-09-2023
DATE : 06/12/2023 13:50
TEXT : ..ppendixASoilProfiles.txt

ELEVATION : 1617 m
X-COORD : 29.45065°E
Y-COORD : 26.09648°S

HOLE No: KTP15

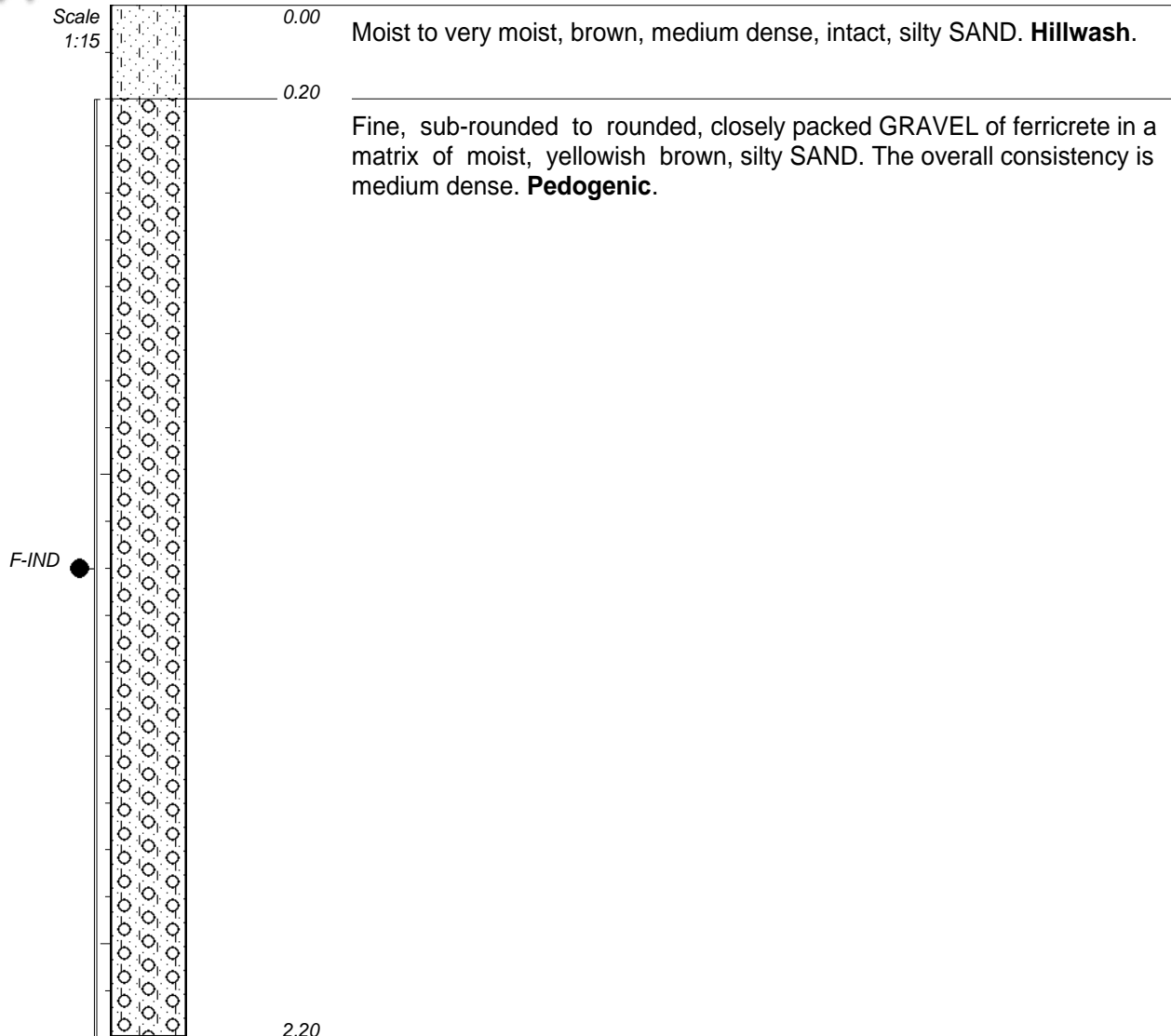


CONTRACTOR :
MACHINE : BELL TLB
DRILLED BY : Xolani Shabangu
PROFIED BY : Andries Vukeya
TYPE SET BY : Andries Vukeya
SETUP FILE : STANDARD.SET

INCLINATION :
DIAM :
DATE : 22-08-2023
DATE : 20-09-2023
DATE : 06/12/2023 13:50
TEXT : ..ppendixASoilProfiles.txt

ELEVATION : 1622 m
X-COORD : 29.45249°E
Y-COORD : 26.09714°S

HOLE No: KTP16


NOTES

- 1) Excavation stopped at 2.2m due to required depth.
- 2) No groundwater seepage.
- 3) Sidewall stable.
- 4) Disturbed sample at 0.2--2.2m: F-IND.

CONTRACTOR :
 MACHINE : BELL TLB
 DRILLED BY : Xolani Shabangu
 PROFILED BY : Andries Vukeya
 TYPE SET BY : Andries Vukeya
 SETUP FILE : STANDARD.SET

INCLINATION :
 DIAM :
 DATE : 22-08-2023
 DATE : 20-09-2023
 DATE : 06/12/2023 13:50
 TEXT : ..ppendixASoilProfiles.txt

ELEVATION : 1624 m
 X-COORD : 29.45518°E
 Y-COORD : 26.09634°S

HOLE No: KTP17

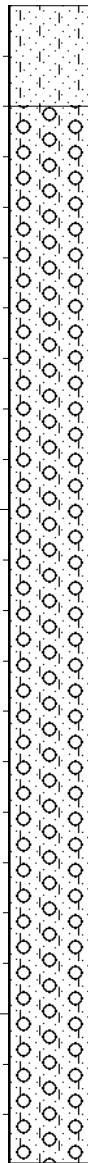


ESKOM
PRELIMINARY GEOTECHNICAL INVESTIGATION:
KOMATI POWER STATION

HOLE No: KTP18
Sheet 1 of 1

JOB NUMBER: MAK152.08.23

Scale
1:15



0.00

Moist to very moist, brown, medium dense, intact, silty SAND. **Hillwash.**

0.20

Fine, sub-rounded to rounded, closely packed GRAVEL of ferricrete in a matrix of moist, yellowish brown, silty SAND. The overall consistency is medium dense. **Pedogenic.**

2.30

NOTES

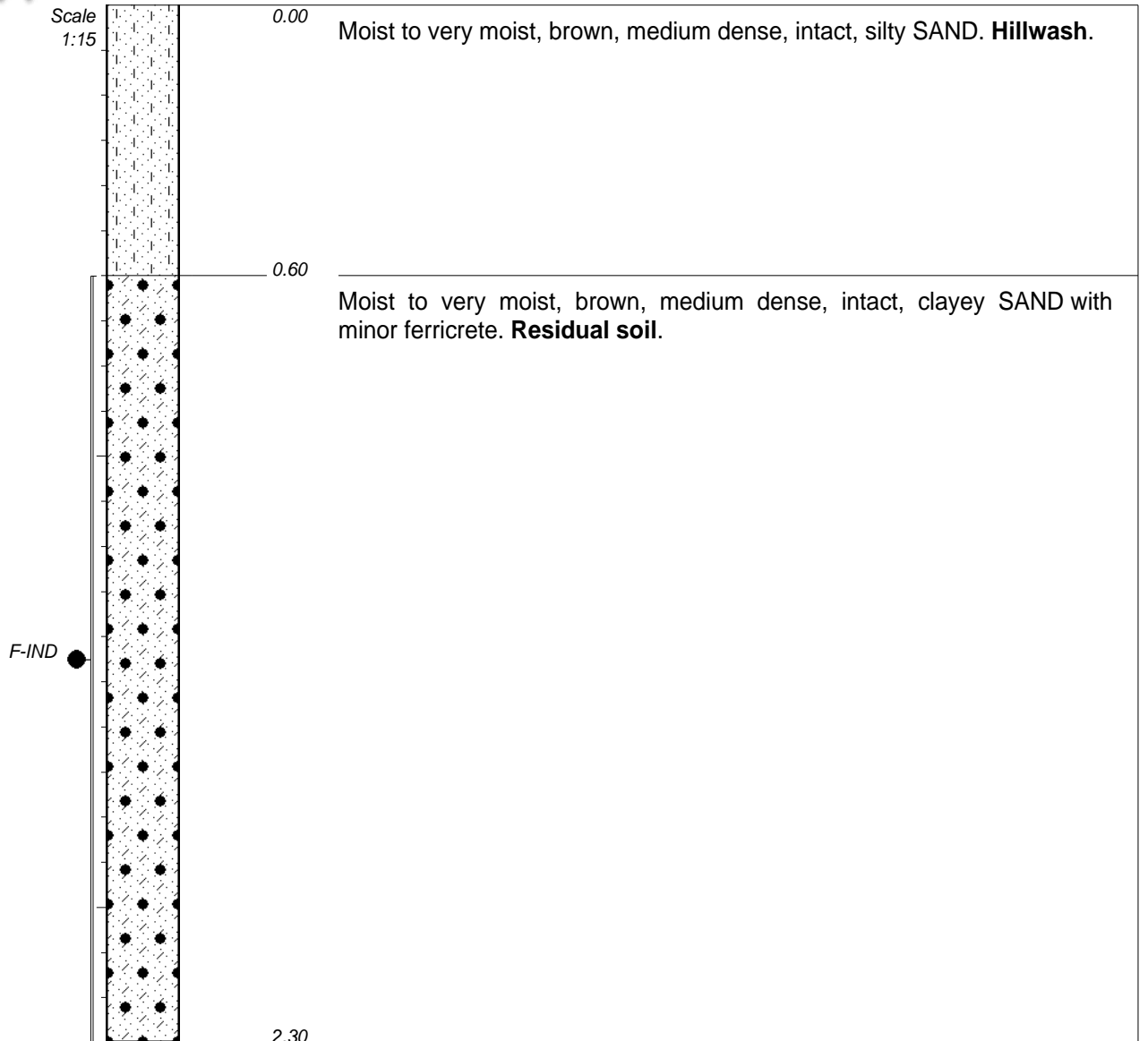
- 1) Excavation stopped at 2.3m due to required depth.
- 2) No groundwater seepage.
- 3) Sidewall stable.

CONTRACTOR :
MACHINE : BELL TLB
DRILLED BY : Xolani Shabangu
PROFILED BY : Andries Vukeya
TYPE SET BY : Andries Vukeya
SETUP FILE : STANDARD.SET

INCLINATION :
DIAM :
DATE : 22-08-2023
DATE : 20-09-2023
DATE : 06/12/2023 13:50
TEXT : ..ppendixASoilProfiles.txt

ELEVATION : 1626 m
X-COORD : 29.45380°E
Y-COORD : 26.09752°S

HOLE No: KTP18



NOTES

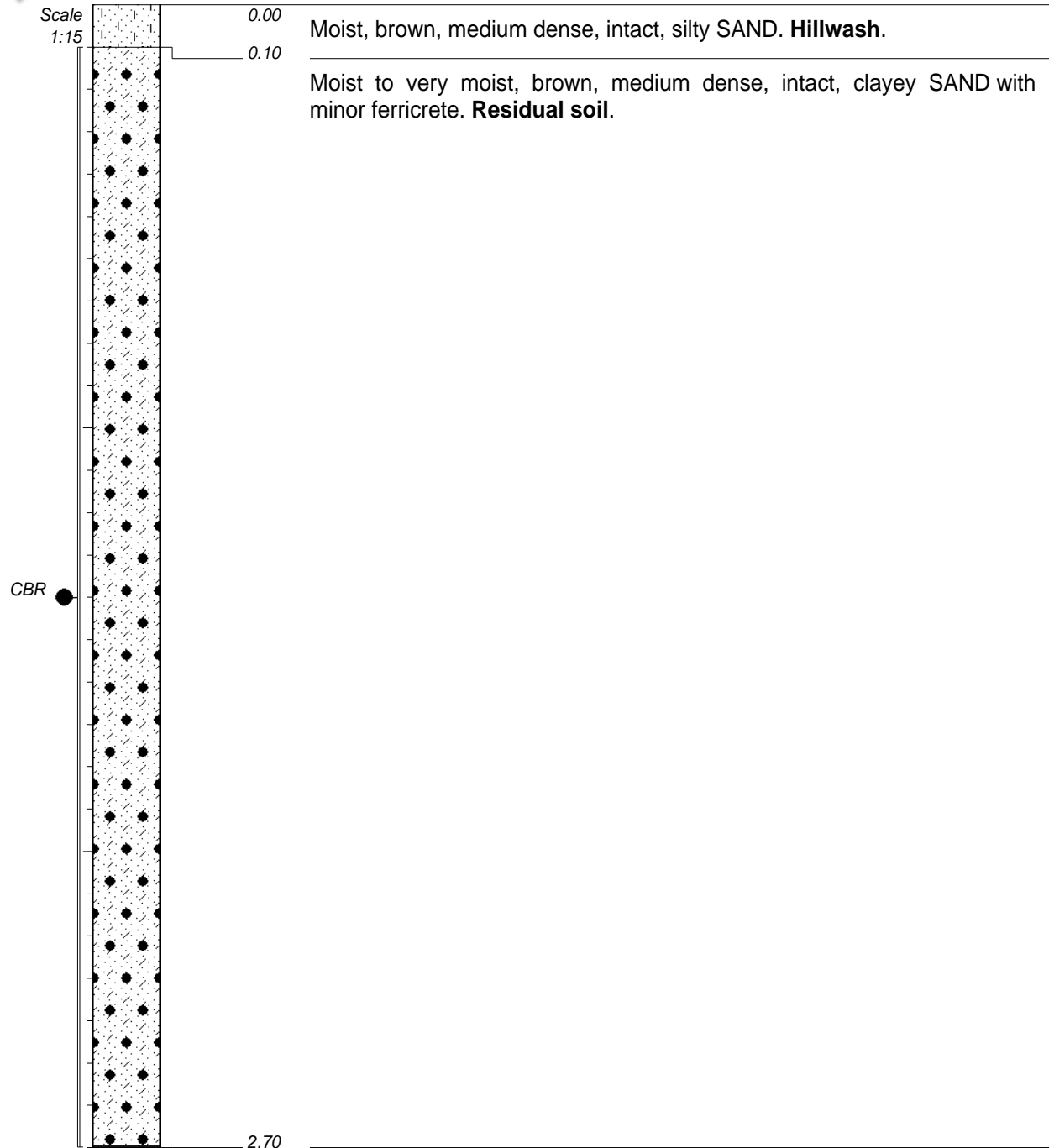
- 1) Excavation stopped at 2.3m due to required depth.
- 2) No groundwater seepage.
- 3) Sidewall stable.
- 4) Disturbed sample at 0.6--2.3m: F-IND.

CONTRACTOR :
MACHINE : BELL TLB
DRILLED BY : Xolani Shabangu
PROFILED BY : Andries Vukeya
TYPE SET BY : Andries Vukeya
SETUP FILE : STANDARD.SET

INCLINATION :
DIAM :
DATE : 22-08-2023
DATE : 20-09-2023
DATE : 06/12/2023 13:50
TEXT : ..ppendixASoilProfiles.txt

ELEVATION : 1624 m
X-COORD : 29.45230°E
Y-COORD : 26.09809°S

HOLE No: KTP19



NOTES

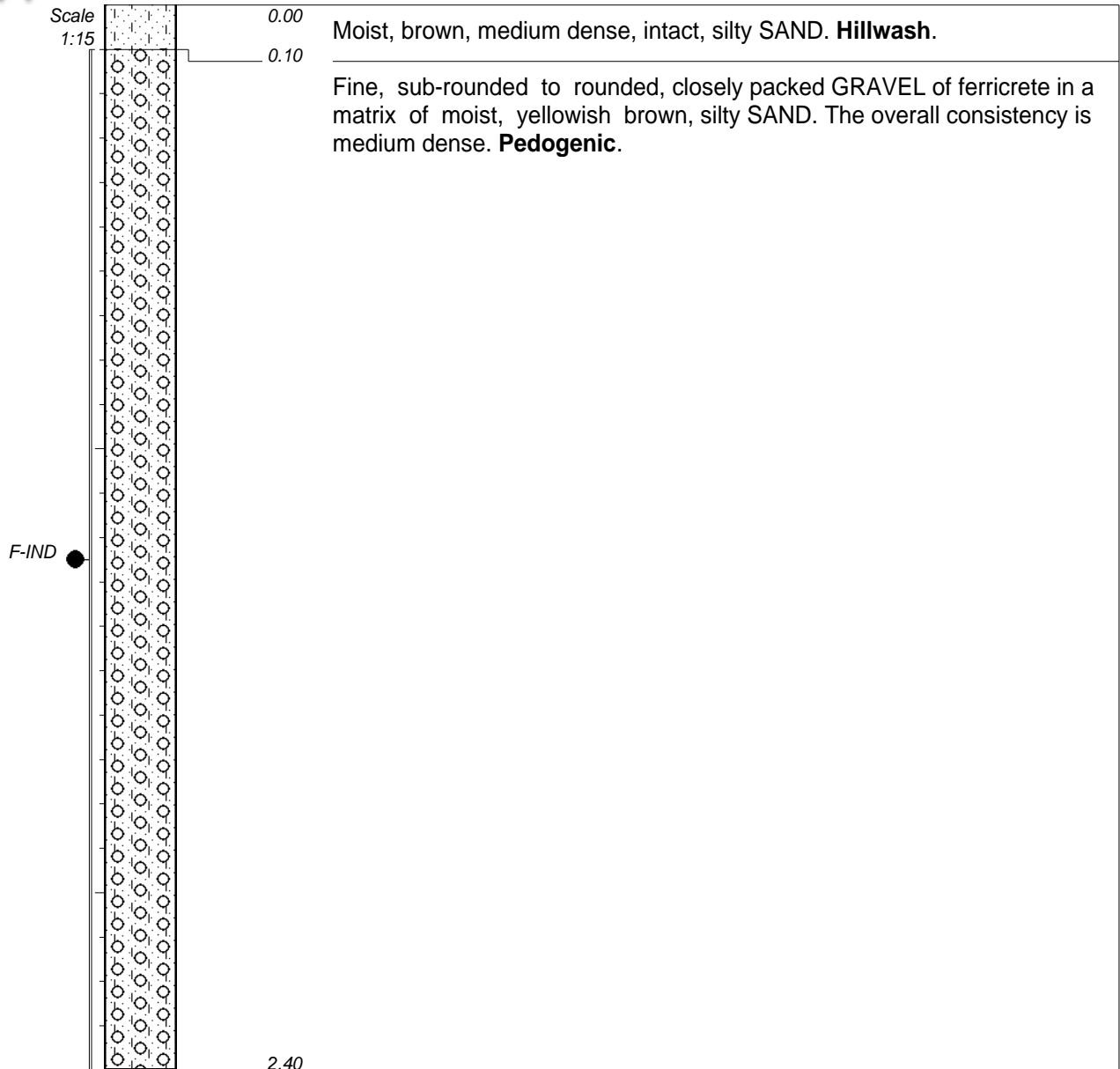
- 1) Excavation stopped at 2.7m due to required depth.
- 2) No groundwater seepage.
- 3) Sidewall stable.
- 4) Disturbed sample at 0.1--2.7m: CBR, F-IND.

CONTRACTOR :
MACHINE : BELL TLB
DRILLED BY : Xolani Shabangu
PROFILED BY : Andries Vukeya
TYPE SET BY : Andries Vukeya
SETUP FILE : STANDARD.SET

INCLINATION :
DIAM :
DATE : 22-08-2023
DATE : 20-09-2023
DATE : 06/12/2023 13:50
TEXT : ..ppendixASoilProfiles.txt

ELEVATION : 1629 m
X-COORD : 29.45183°E
Y-COORD : 26.10502°S

HOLE No: KTP20



NOTES

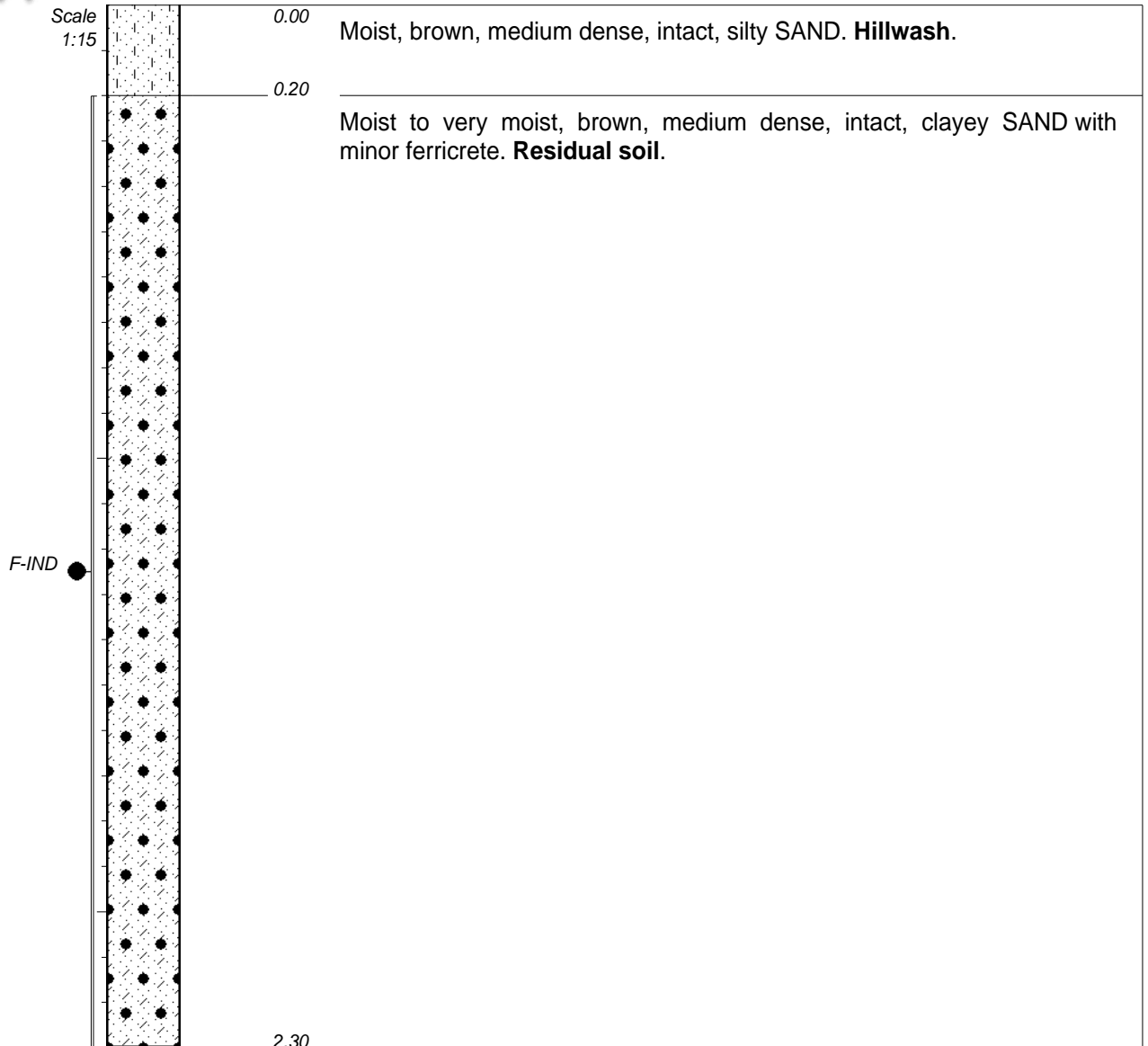
- 1) Excavation stopped at 2.4m due to required depth.
- 2) No groundwater seepage.
- 3) Sidewall stable.
- 4) Disturbed sample at 0.1--2.4m: F-IND.

CONTRACTOR :
MACHINE : BELL TLB
DRILLED BY : Xolani Shabangu
PROFILED BY : Andries Vukeya
TYPE SET BY : Andries Vukeya
SETUP FILE : STANDARD.SET

INCLINATION :
DIAM :
DATE : 22-08-2023
DATE : 20-09-2023
DATE : 06/12/2023 13:50
TEXT : ..ppendixASoilProfiles.txt

ELEVATION : 1641 m
X-COORD : 29.45453°E
Y-COORD : 26.10459°S

HOLE No: KTP21



NOTES

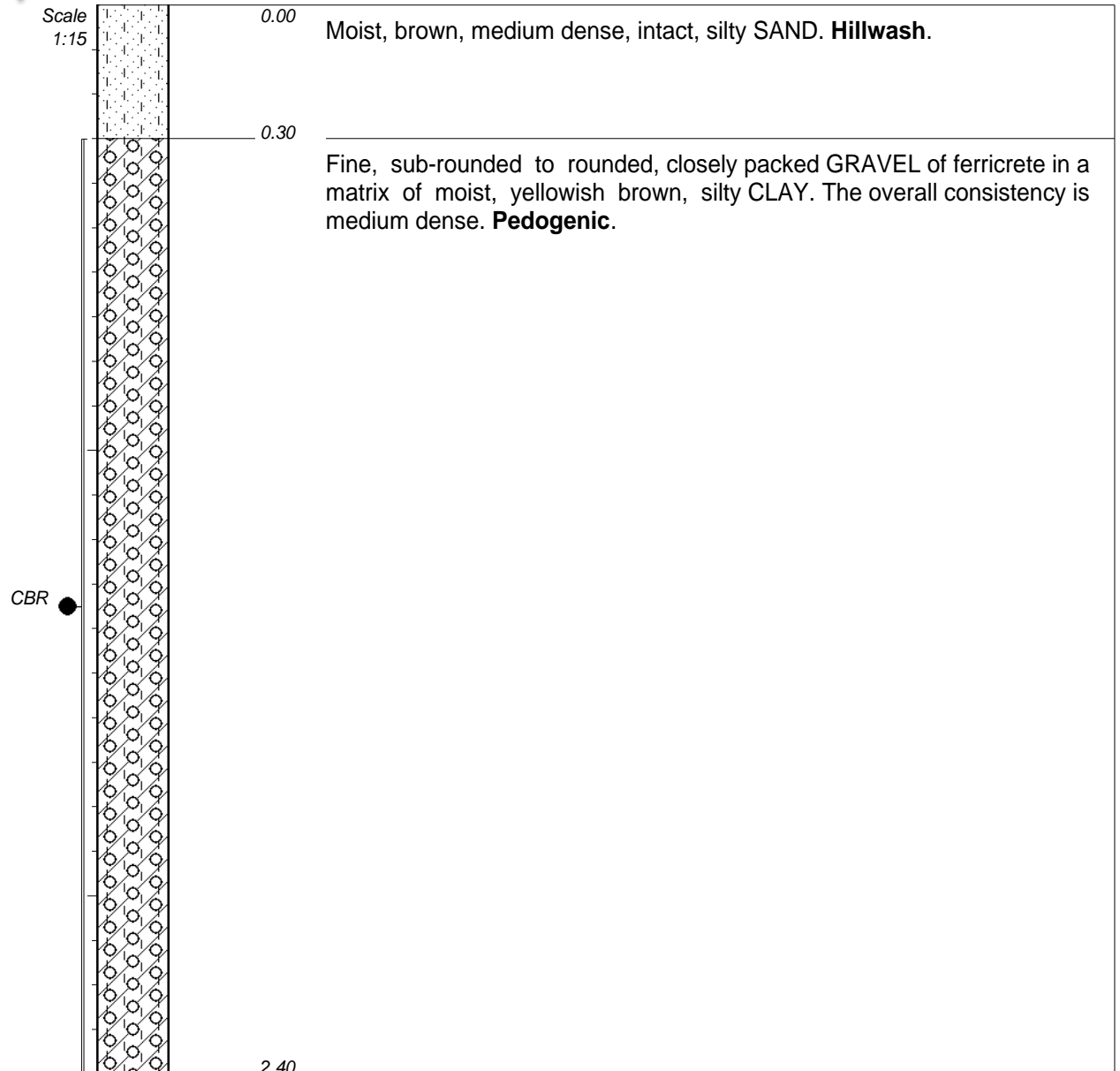
- 1) Excavation stopped at 2.3m due to required depth.
- 2) No groundwater seepage.
- 3) Sidewall stable.
- 4) Disturbed sample at 0.2--2.3m: F-IND.

CONTRACTOR :
MACHINE : BELL TLB
DRILLED BY : Xolani Shabangu
PROFILED BY : Andries Vukeya
TYPE SET BY : Andries Vukeya
SETUP FILE : STANDARD.SET

INCLINATION :
DIAM :
DATE : 22-08-2023
DATE : 20-09-2023
DATE : 06/12/2023 13:50
TEXT : ..ppendixASoilProfiles.txt

ELEVATION : 1641 m
X-COORD : 29.45677°E
Y-COORD : 26.10280°S

HOLE No: KTP22



NOTES

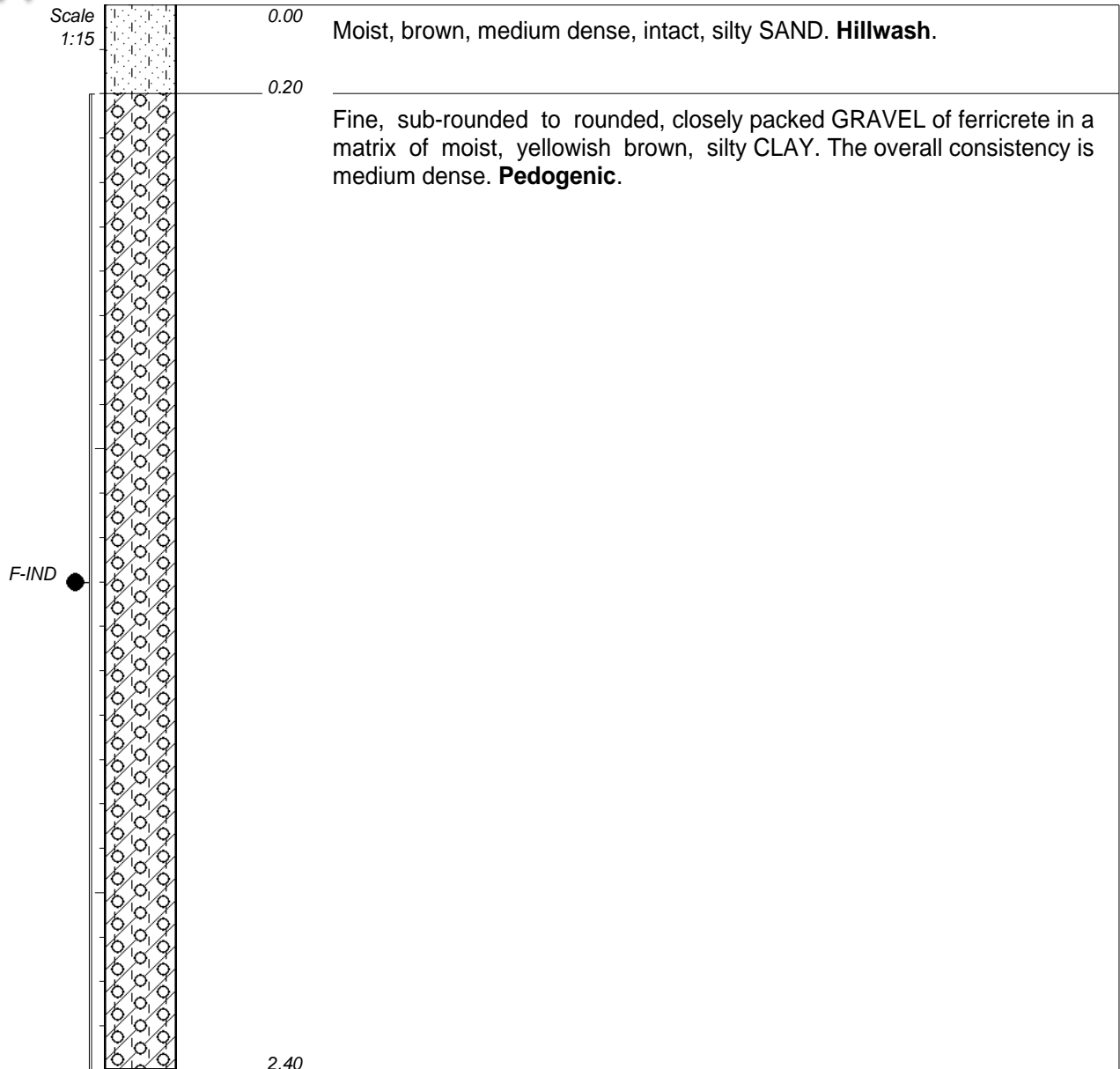
- 1) Excavation stopped at 2.4m due to required depth.
- 2) No groundwater seepage.
- 3) Sidewall stable.
- 4) Disturbed sample at 0.3--2.4m: CBR, F-IND.

CONTRACTOR :
MACHINE : BELL TLB
DRILLED BY : Xolani Shabangu
PROFILED BY : Andries Vukeya
TYPE SET BY : Andries Vukeya
SETUP FILE : STANDARD.SET

INCLINATION :
DIAM :
DATE : 22-08-2023
DATE : 20-09-2023
DATE : 06/12/2023 13:50
TEXT : ..ppendixASoilProfiles.txt

ELEVATION : 1646 m
X-COORD : 29.45968°E
Y-COORD : 26.10463°S

HOLE No: KTP23



NOTES

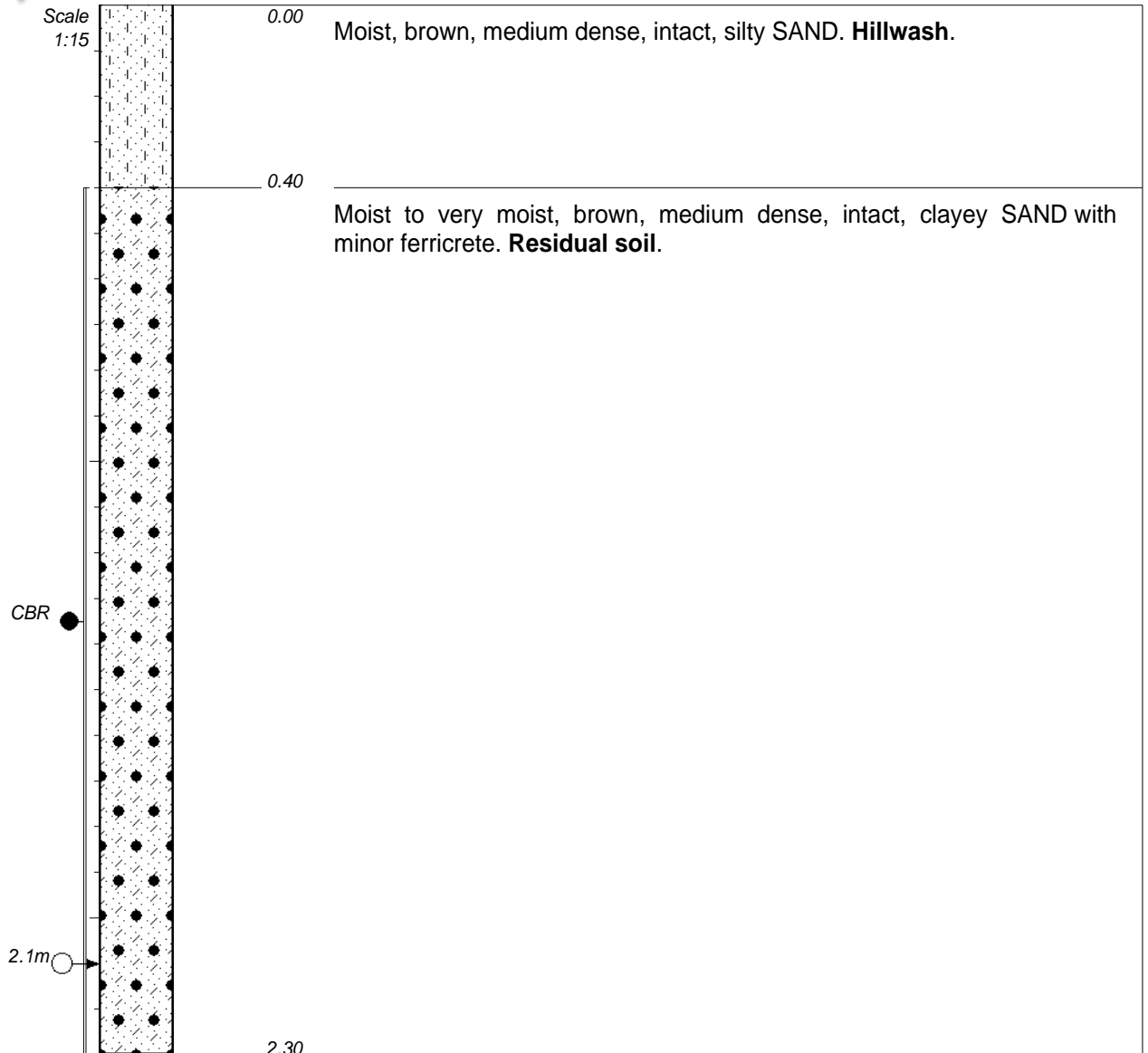
- 1) Excavation stopped at 2.4m due to required depth.
- 2) No groundwater seepage.
- 3) Sidewall stable.
- 4) Disturbed sample at 0.2--2.4m: F-IND.

CONTRACTOR :
MACHINE : BELL TLB
DRILLED BY : Xolani Shabangu
PROFILED BY : Andries Vukeya
TYPE SET BY : Andries Vukeya
SETUP FILE : STANDARD.SET

INCLINATION :
DIAM :
DATE : 22-08-2023
DATE : 20-09-2023
DATE : 06/12/2023 13:50
TEXT : ..ppendixASoilProfiles.txt

ELEVATION : 1644 m
X-COORD : 29.46198°E
Y-COORD : 26.10455°S

HOLE No: KTP24



NOTES

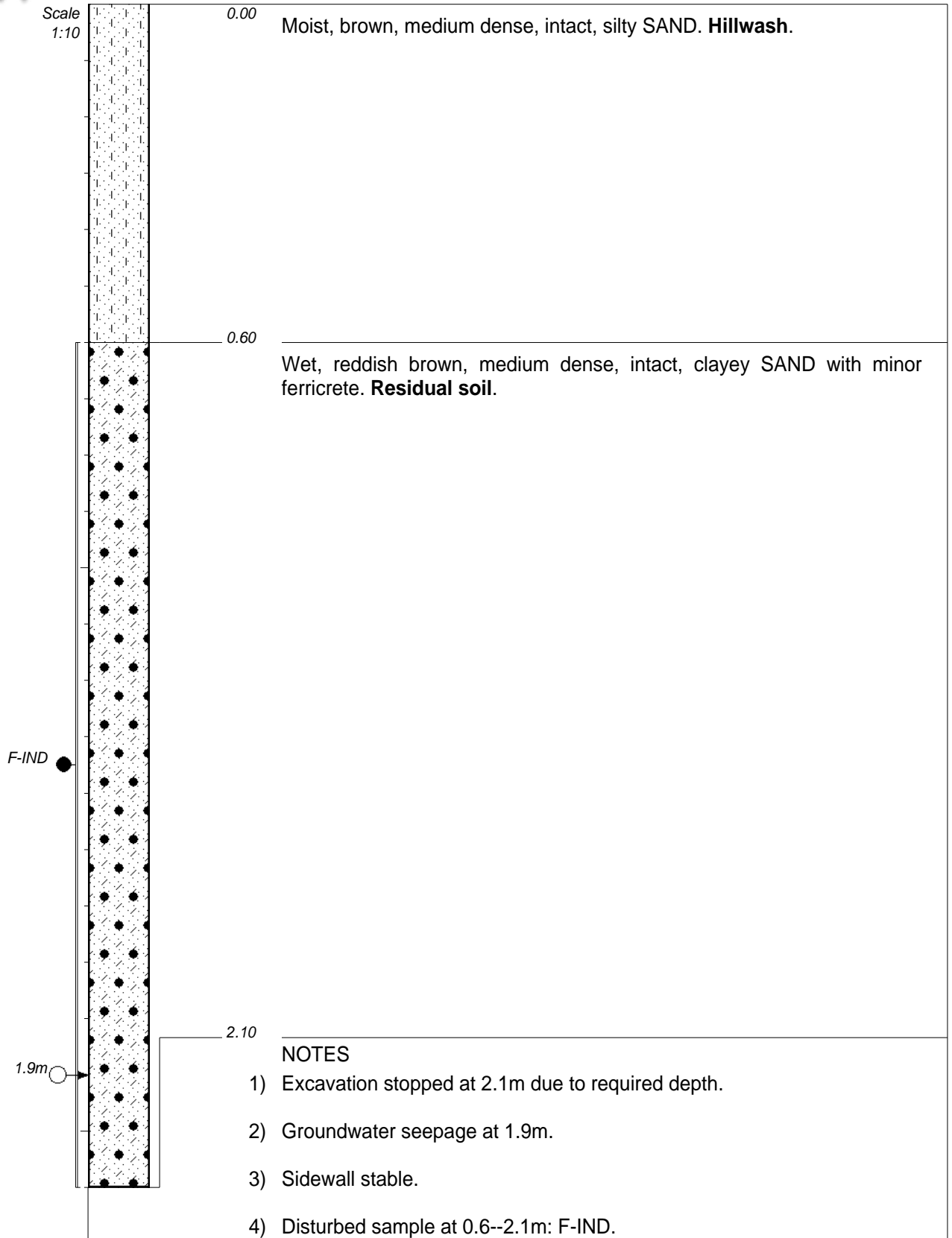
- 1) Excavation stopped at 2.3m due to required depth.
- 2) Groundwater seepage at 2.1m.
- 3) Sidewall stable.
- 4) Disturbed sample at 0.4--2.3m: CBR, F-IND.

CONTRACTOR :
MACHINE : BELL TLB
DRILLED BY : Xolani Shabangu
PROFILED BY : Andries Vukeya
TYPE SET BY : Andries Vukeya
SETUP FILE : STANDARD.SET

INCLINATION :
DIAM :
DATE : 22-08-2023
DATE : 20-09-2023
DATE : 06/12/2023 13:50
TEXT : ..ppendixASoilProfiles.txt

ELEVATION : 1647 m
X-COORD : 29.46486°E
Y-COORD : 26.10464°S

HOLE No: KTP25

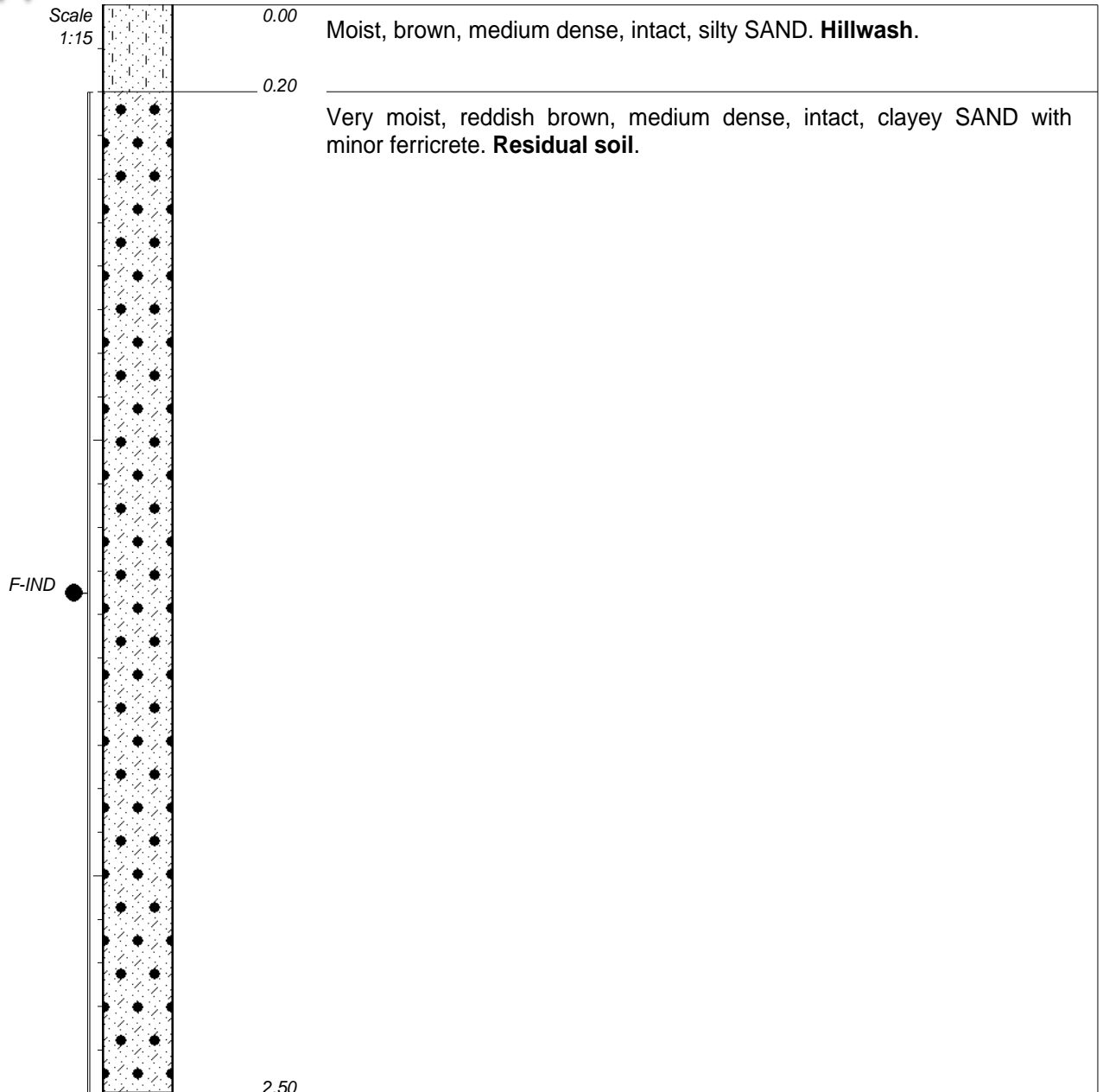


CONTRACTOR :
MACHINE : BELL TLB
DRILLED BY : Xolani Shabangu
PROFILED BY : Andries Vukeya
TYPE SET BY : Andries Vukeya
SETUP FILE : STANDARD.SET

INCLINATION :
DIAM :
DATE : 22-08-2023
DATE : 20-09-2023
DATE : 06/12/2023 13:50
TEXT : ..ppendixASoilProfiles.txt

ELEVATION : 1652 m
X-COORD : 29.46568°E
Y-COORD : 26.10768°S

HOLE No: KTP26



NOTES

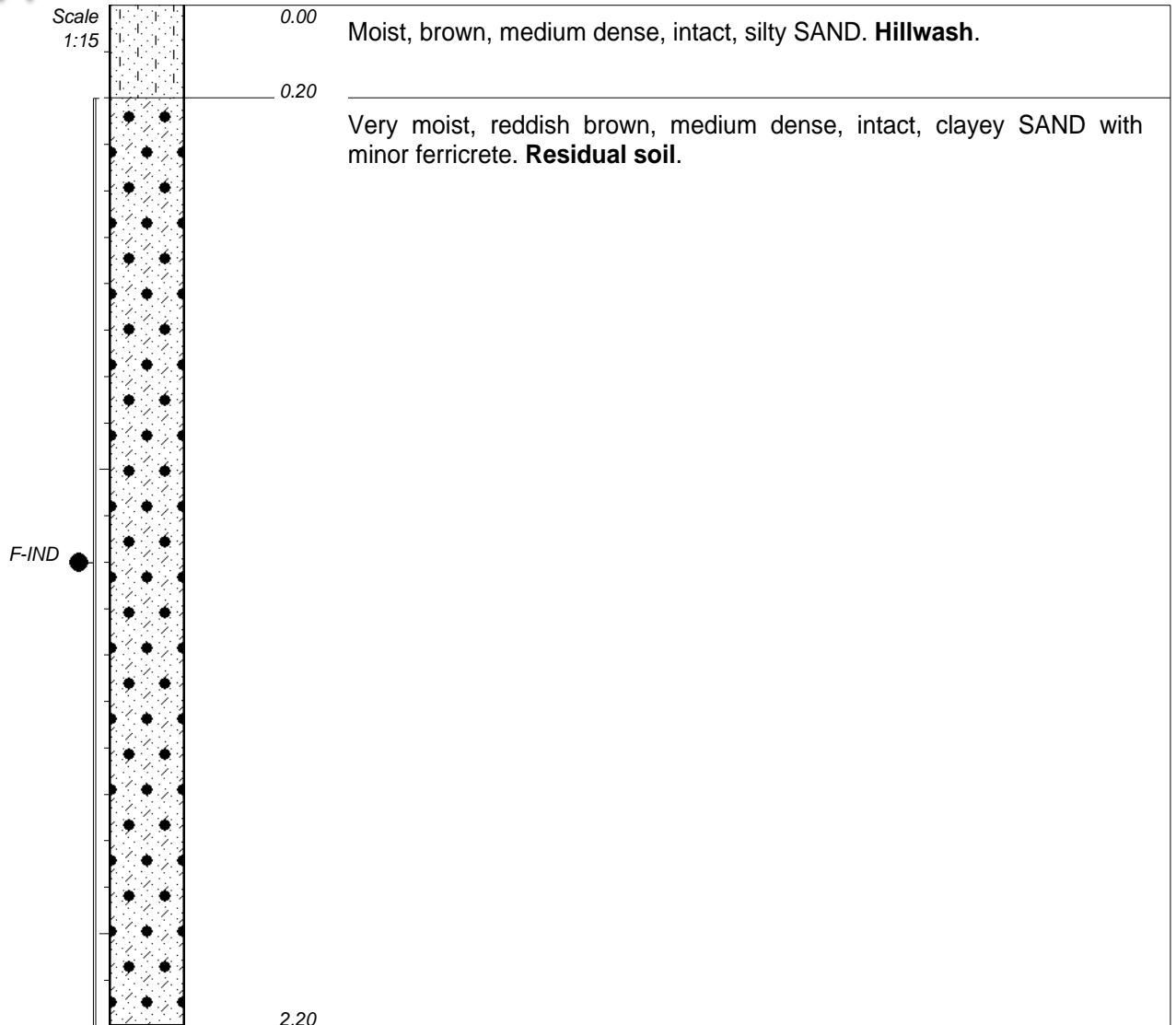
- 1) Excavation stopped at 2.5m due to required depth.
- 2) No groundwater seepage.
- 3) Sidewall stable.
- 4) Disturbed sample at 0.2--2.5m: F-IND.

CONTRACTOR :
MACHINE : BELL TLB
DRILLED BY : Xolani Shabangu
PROFILED BY : Andries Vukeya
TYPE SET BY : Andries Vukeya
SETUP FILE : STANDARD.SET

INCLINATION :
DIAM :
DATE : 22-08-2023
DATE : 20-09-2023
DATE : 06/12/2023 13:50
TEXT : ..ppendixASoilProfiles.txt

ELEVATION : 1658 m
X-COORD : 29.46493°E
Y-COORD : 26.11053°S

HOLE No: KTP27



NOTES

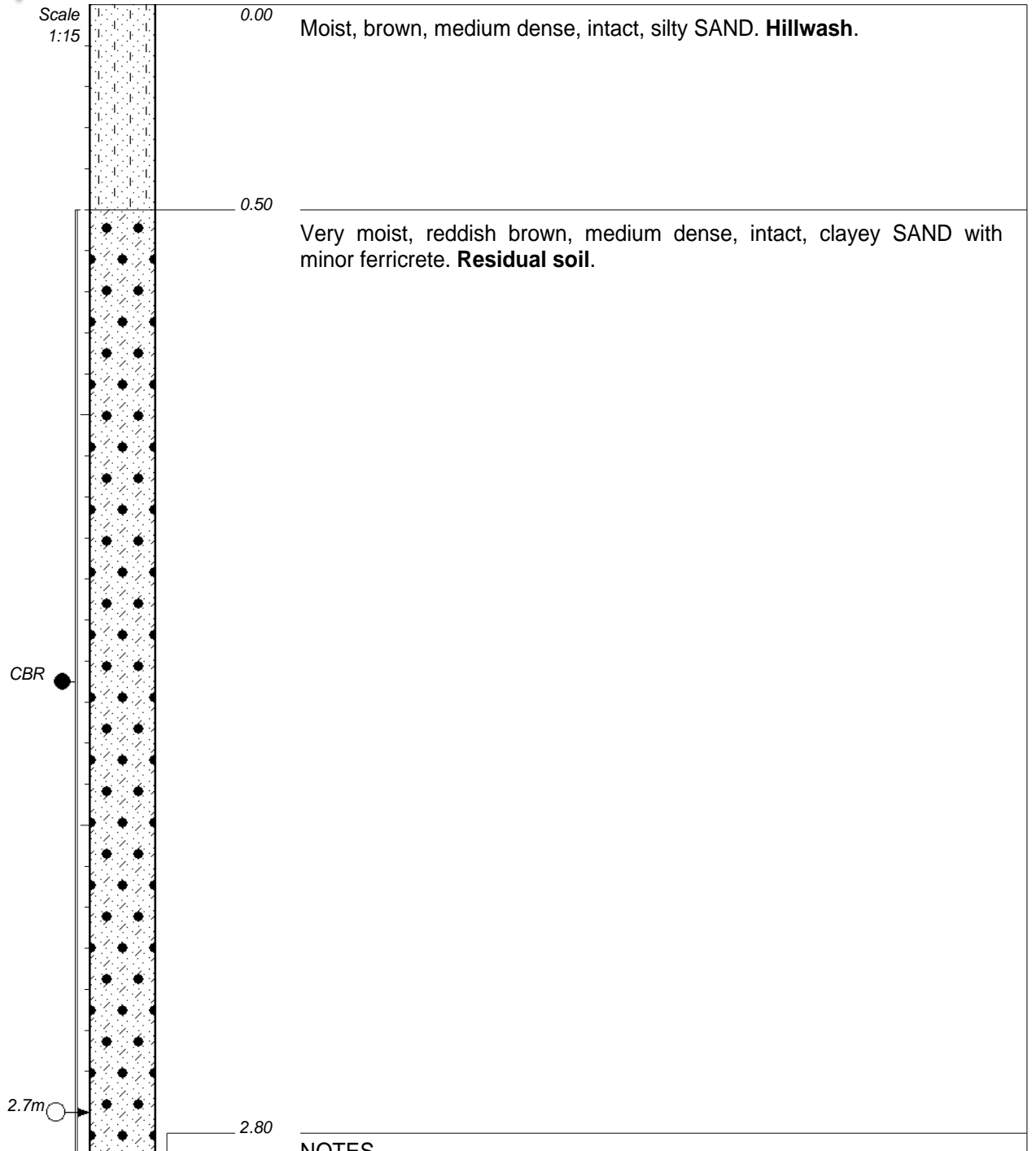
- 1) Excavation stopped at 2.2m due to required depth.
- 2) No groundwater seepage.
- 3) Sidewall stable.
- 4) Disturbed sample at 0.2--2.2m: F-IND.

CONTRACTOR :
MACHINE : BELL TLB
DRILLED BY : Xolani Shabangu
PROFILED BY : Andries Vukeya
TYPE SET BY : Andries Vukeya
SETUP FILE : STANDARD.SET

INCLINATION :
DIAM :
DATE : 22-08-2023
DATE : 20-09-2023
DATE : 06/12/2023 13:50
TEXT : ..ppendixASoilProfiles.txt

ELEVATION : 1660 m
X-COORD : 29.46242°E
Y-COORD : 26.10930°S

HOLE No: KTP28



NOTES

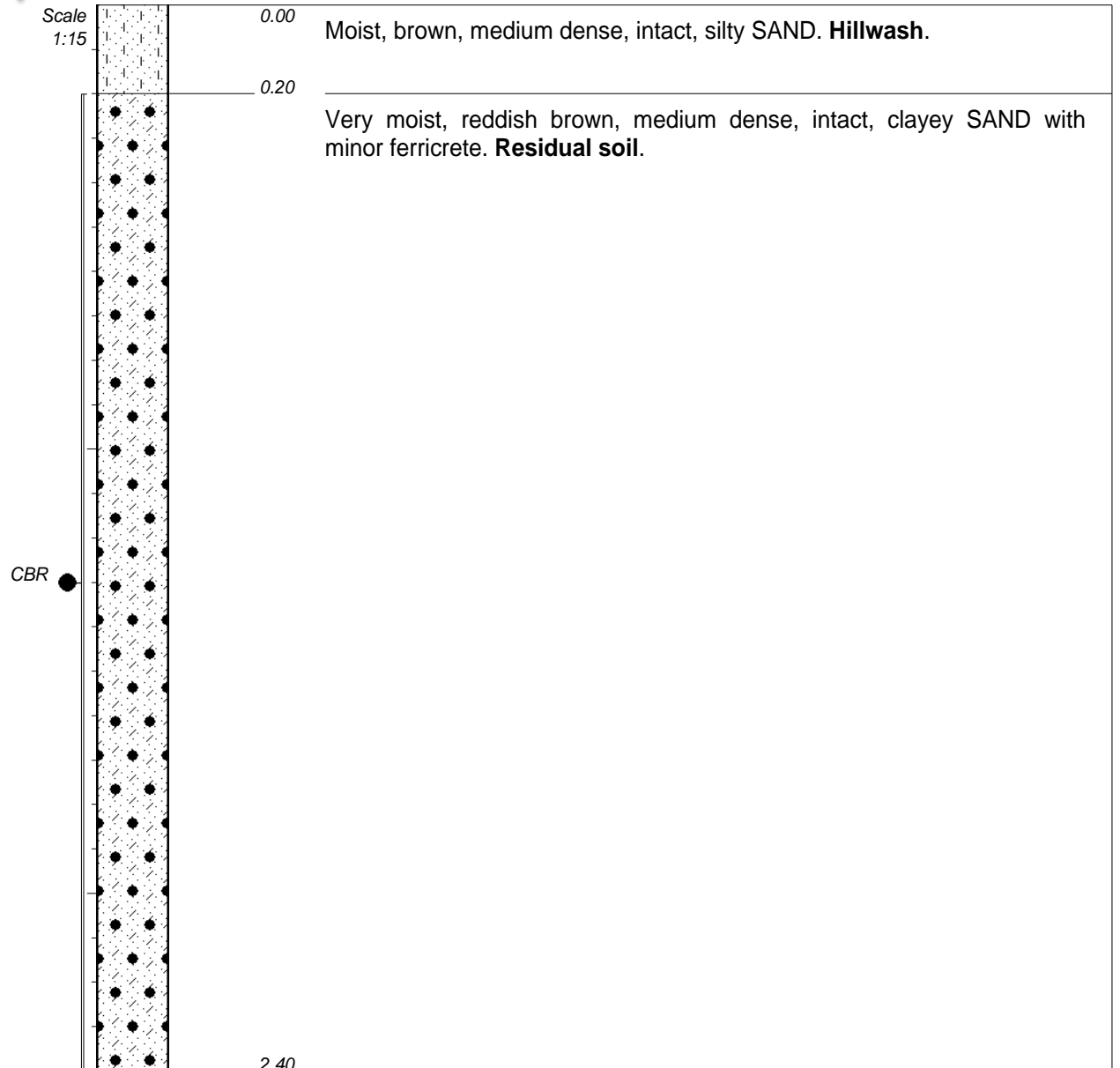
- 1) Excavation stopped at 2.8m due to required depth.
- 2) Groundwater seepage at 2.7m.
- 3) Sidewall stable.
- 4) Disturbed sample at 0.5--2.8m: CBR, F-IND.

CONTRACTOR :
MACHINE : BELL TLB
DRILLED BY : Xolani Shabangu
PROFILED BY : Andries Vukeya
TYPE SET BY : Andries Vukeya
SETUP FILE : STANDARD.SET

INCLINATION :
DIAM :
DATE : 22-08-2023
DATE : 20-09-2023
DATE : 06/12/2023 13:50
TEXT : ..ppendixASoilProfiles.txt

ELEVATION : 1657 m
X-COORD : 29.45976°E
Y-COORD : 26.10974°S

HOLE No: KTP29



NOTES

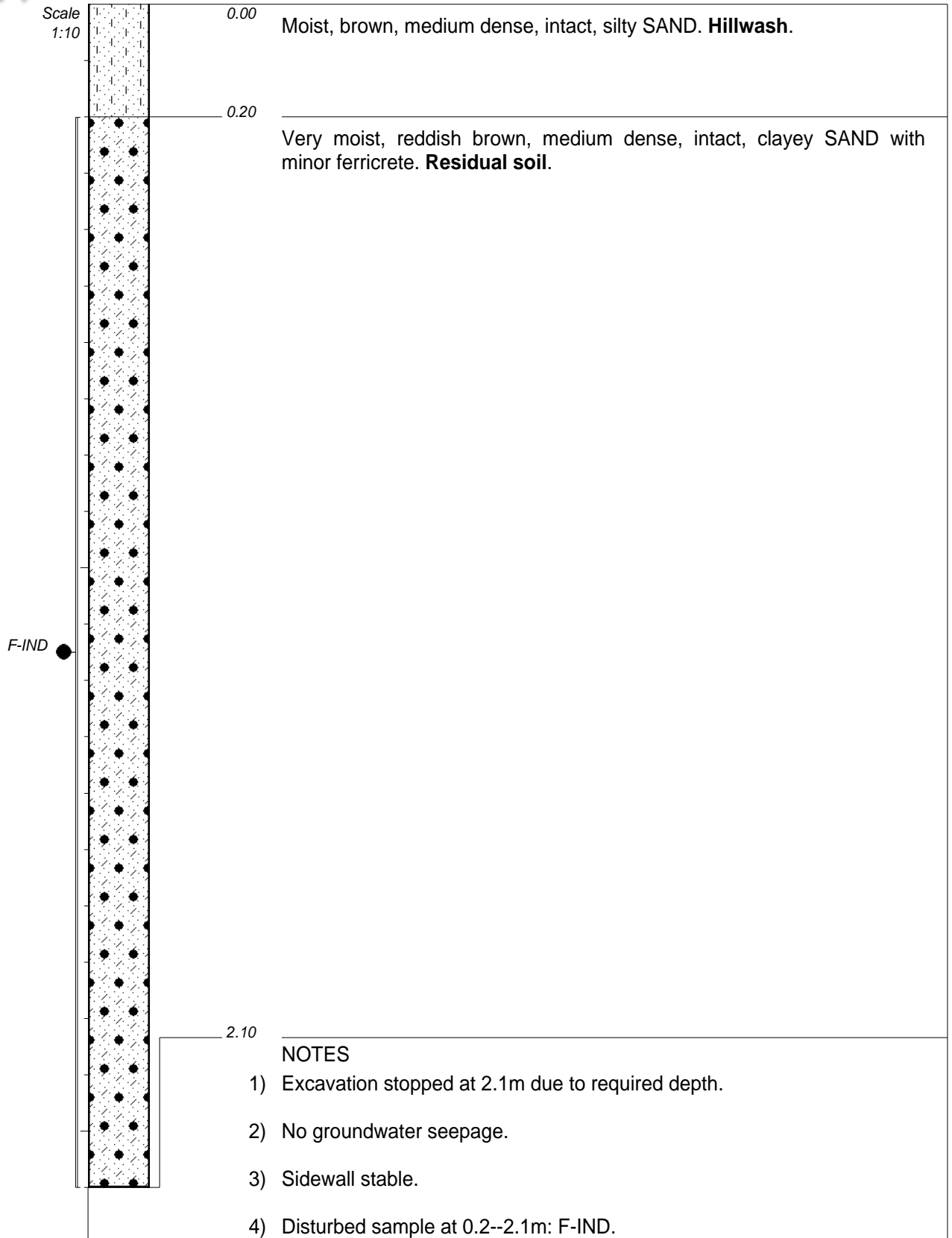
- 1) Excavation stopped at 2.4m due to required depth.
- 2) No groundwater seepage.
- 3) Sidewall stable.
- 4) Disturbed sample at 0.2--2.4m: CBR, F-IND.

CONTRACTOR :
MACHINE : BELL TLB
DRILLED BY : Xolani Shabangu
PROFILED BY : Andries Vukeya
TYPE SET BY : Andries Vukeya
SETUP FILE : STANDARD.SET

INCLINATION :
DIAM :
DATE : 22-08-2023
DATE : 20-09-2023
DATE : 06/12/2023 13:50
TEXT : ..ppendixASoilProfiles.txt

ELEVATION : 1649 m
X-COORD : 29.45623°E
Y-COORD : 26.10929°S

HOLE No: KTP30

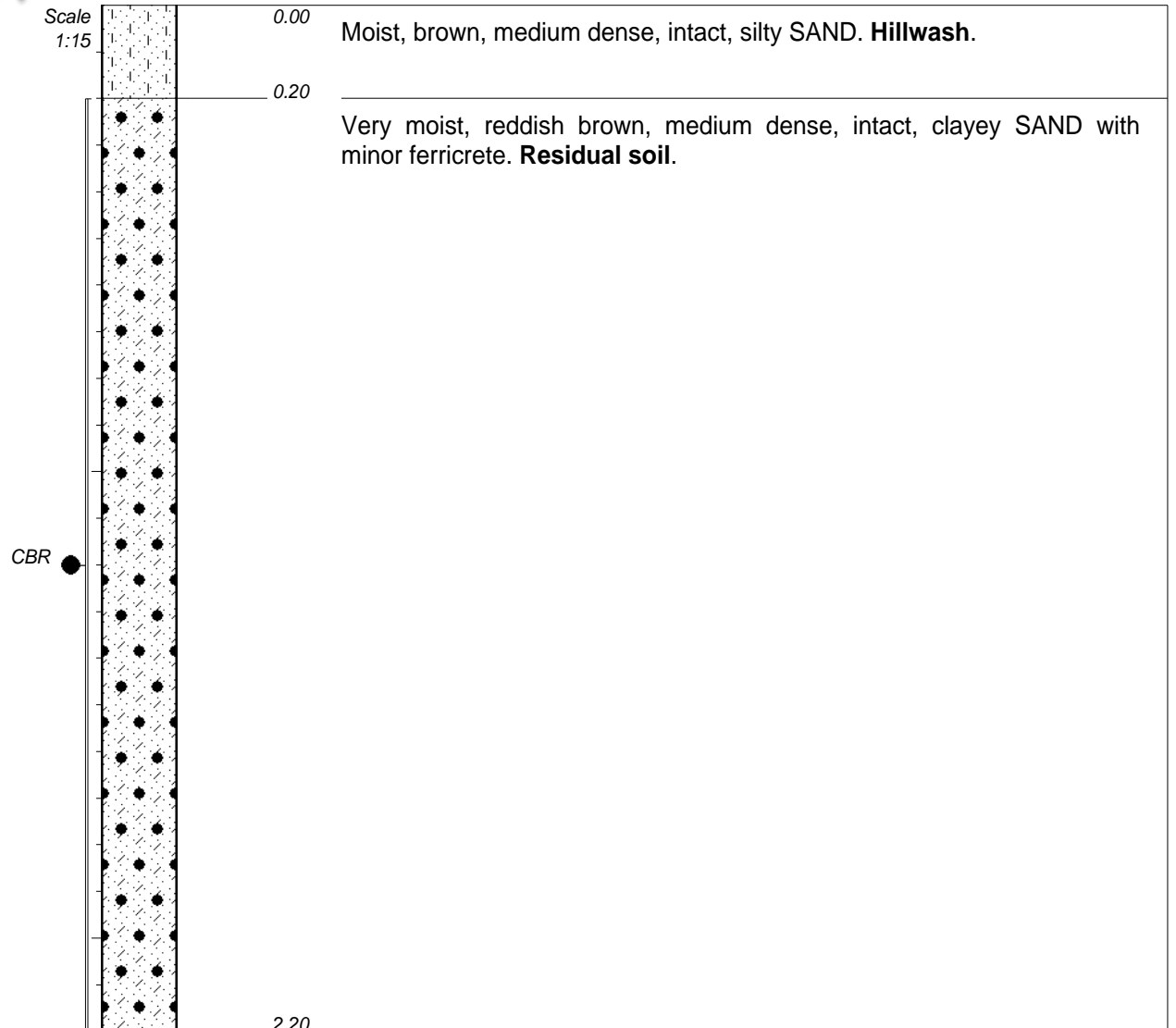


CONTRACTOR :
MACHINE : BELL TLB
DRILLED BY : Xolani Shabangu
PROFILED BY : Andries Vukeya
TYPE SET BY : Andries Vukeya
SETUP FILE : STANDARD.SET

INCLINATION :
DIAM :
DATE : 22-08-2023
DATE : 20-09-2023
DATE : 06/12/2023 13:50
TEXT : ..ppendixASoilProfiles.txt

ELEVATION : 1641 m
X-COORD : 29.45309°E
Y-COORD : 26.10831°S

HOLE No: KTP31



NOTES

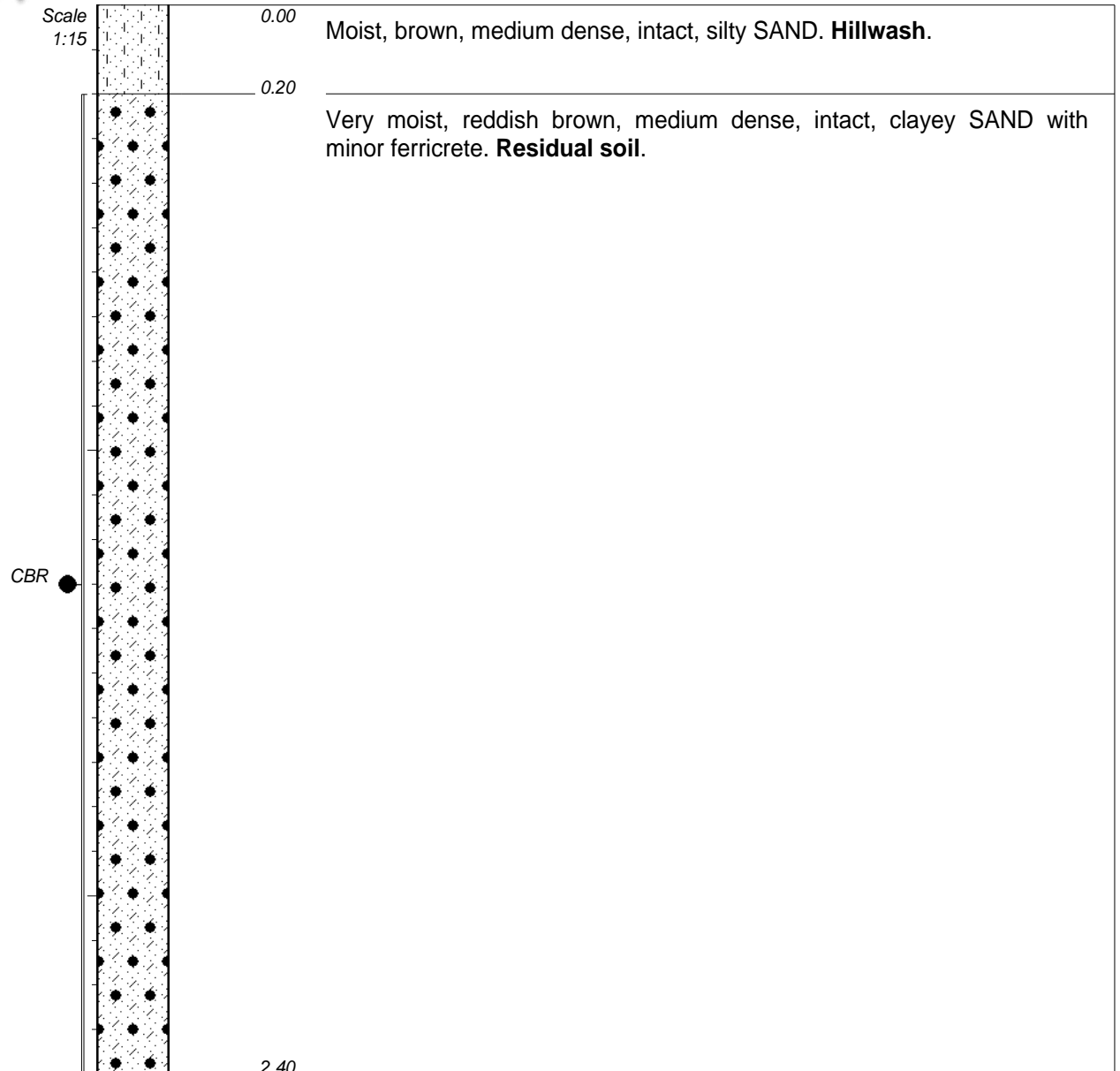
- 1) Excavation stopped at 2.2m due to required depth.
- 2) No groundwater seepage.
- 3) Sidewall stable.
- 4) Disturbed sample at 0.2--2.2m: CBR, F-IND.

CONTRACTOR :
MACHINE : BELL TLB
DRILLED BY : Xolani Shabangu
PROFILED BY : Andries Vukeya
TYPE SET BY : Andries Vukeya
SETUP FILE : STANDARD.SET

INCLINATION :
DIAM :
DATE : 22-08-2023
DATE : 20-09-2023
DATE : 06/12/2023 13:50
TEXT : ..ppendixASoilProfiles.txt

ELEVATION : 1646 m
X-COORD : 29.45509°E
Y-COORD : 26.10666°S

HOLE No: KTP32



NOTES

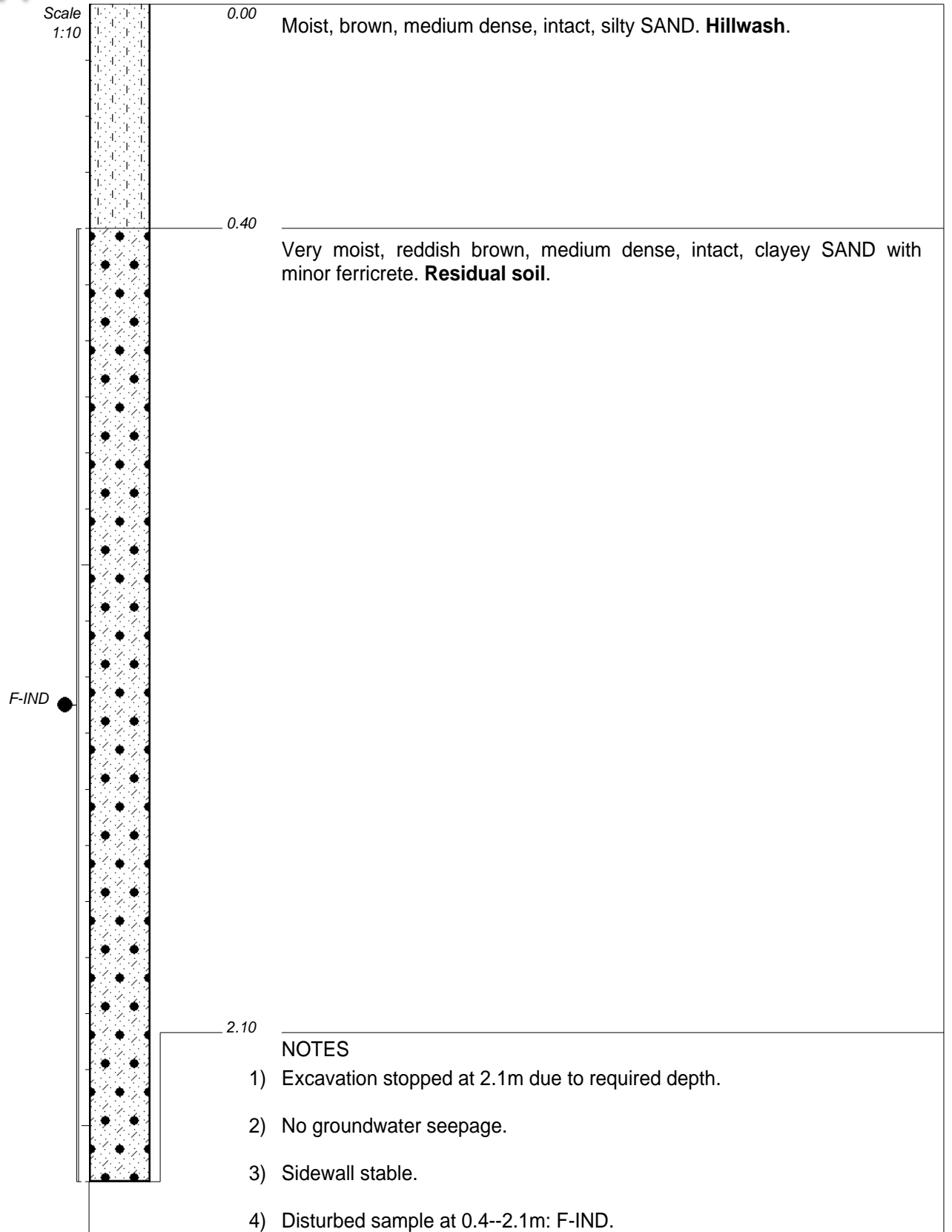
- 1) Excavation stopped at 2.4m due to required depth.
- 2) No groundwater seepage.
- 3) Sidewall stable.
- 4) Disturbed sample at 0.2--2.4m: CBR, F-IND.

CONTRACTOR :
MACHINE : BELL TLB
DRILLED BY : Xolani Shabangu
PROFILED BY : Andries Vukeya
TYPE SET BY : Andries Vukeya
SETUP FILE : STANDARD.SET

INCLINATION :
DIAM :
DATE : 22-08-2023
DATE : 20-09-2023
DATE : 06/12/2023 13:50
TEXT : ..ppendixASoilProfiles.txt

ELEVATION : 1654 m
X-COORD : 29.45810°E
Y-COORD : 26.10634°S

HOLE No: KTP33

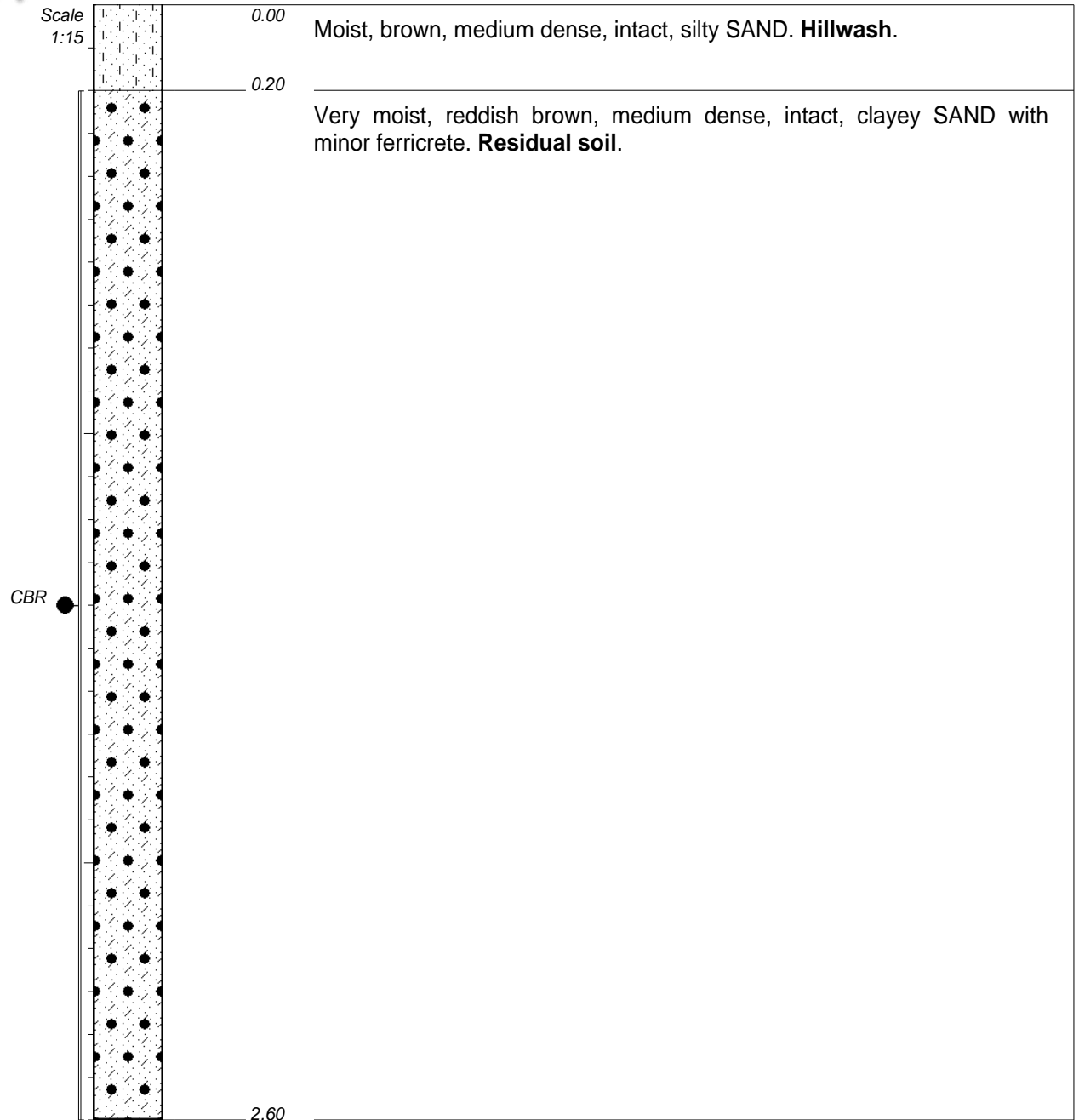


CONTRACTOR :
MACHINE : BELL TLB
DRILLED BY : Xolani Shabangu
PROFILED BY : Andries Vukeya
TYPE SET BY : Andries Vukeya
SETUP FILE : STANDARD.SET

INCLINATION :
DIAM :
DATE : 22-08-2023
DATE : 20-09-2023
DATE : 06/12/2023 13:50
TEXT : ..ppendixASoilProfiles.txt

ELEVATION : 1659 m
X-COORD : 29.46017°E
Y-COORD : 26.10734°S

HOLE No: KTP34



NOTES

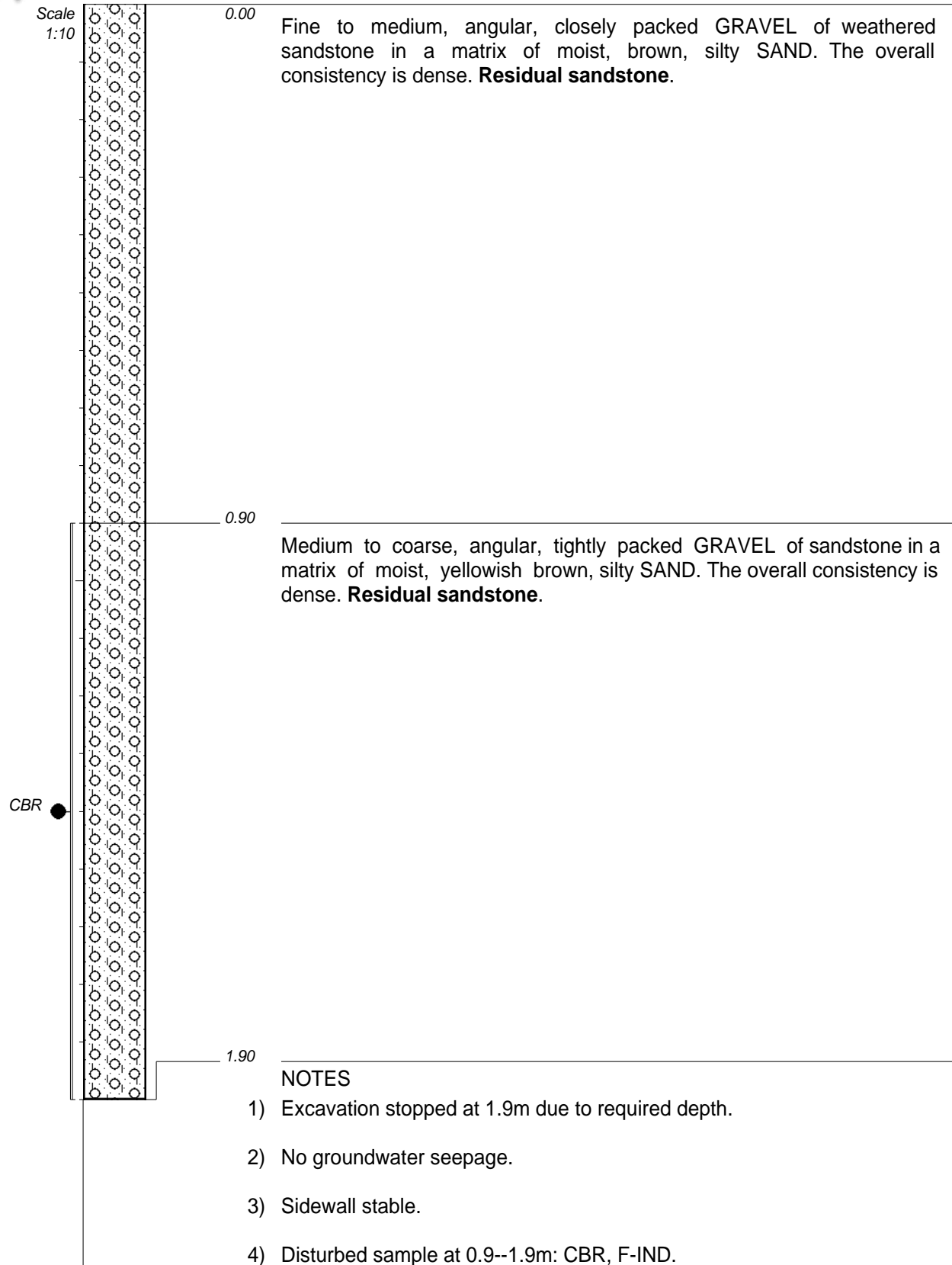
- 1) Excavation stopped at 2.6m due to required depth.
- 2) No groundwater seepage.
- 3) Sidewall stable.
- 4) Disturbed sample at 0.2--2.6m: CBR, F-IND.

CONTRACTOR :
MACHINE : BELL TLB
DRILLED BY : Xolani Shabangu
PROFILED BY : Andries Vukeya
TYPE SET BY : Andries Vukeya
SETUP FILE : STANDARD.SET

INCLINATION :
DIAM :
DATE : 22-08-2023
DATE : 20-09-2023
DATE : 06/12/2023 13:50
TEXT : ..ppendixASoilProfiles.txt

ELEVATION : 1656 m
X-COORD : 29.46280°E
Y-COORD : 26.10676°S

HOLE No: KTP35

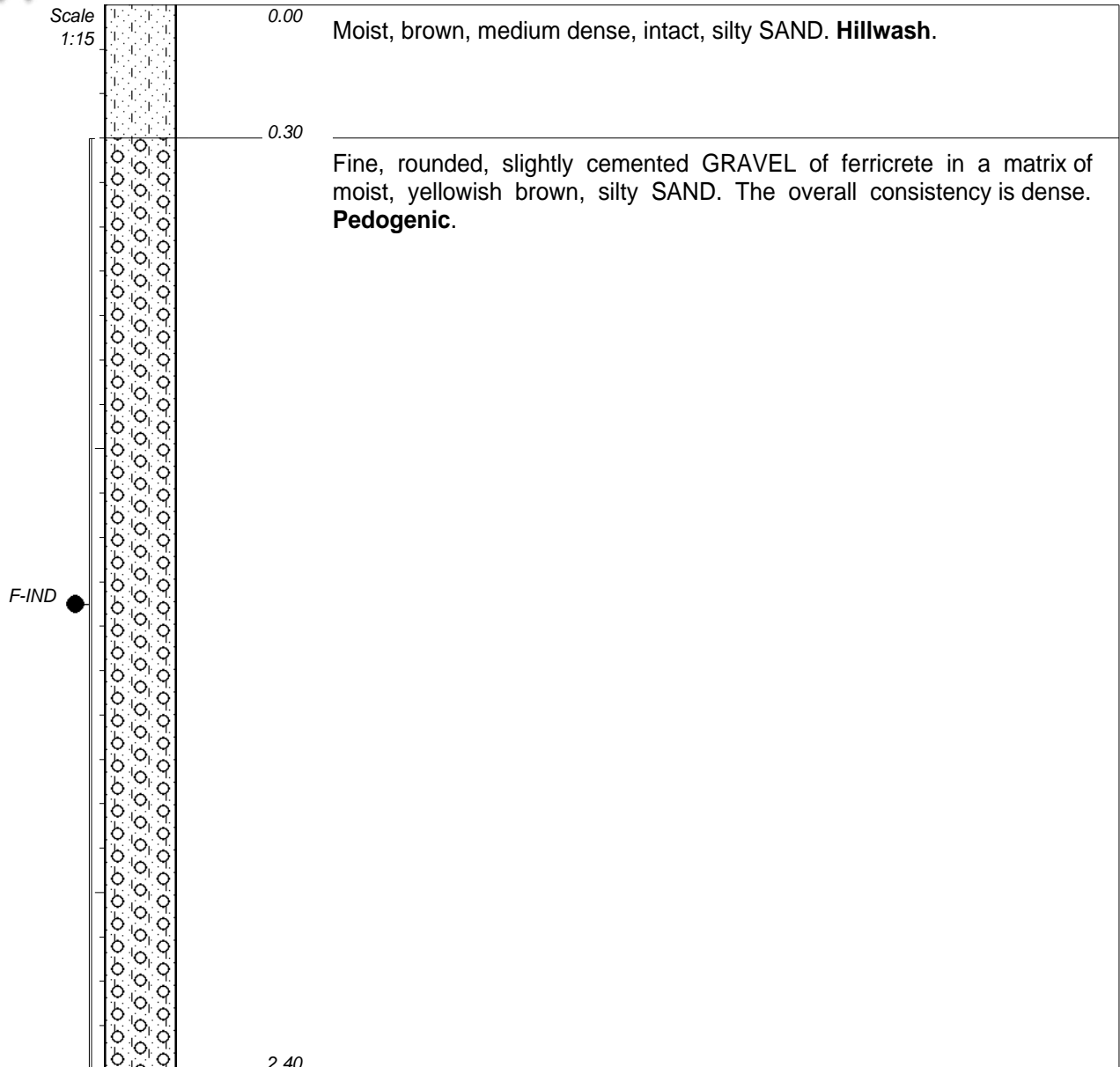


CONTRACTOR :
 MACHINE : BELL TLB
 DRILLED BY : Xolani Shabangu
 PROFILED BY : Andries Vukeya
 TYPE SET BY : Andries Vukeya
 SETUP FILE : STANDARD.SET

INCLINATION :
 DIAM :
 DATE : 22-08-2023
 DATE : 20-09-2023
 DATE : 06/12/2023 13:50
 TEXT : ..ppendixASoilProfiles.txt

ELEVATION : 1638 m
 X-COORD : 29.46236°E
 Y-COORD : 26.10323°S

HOLE No: KTP36



NOTES

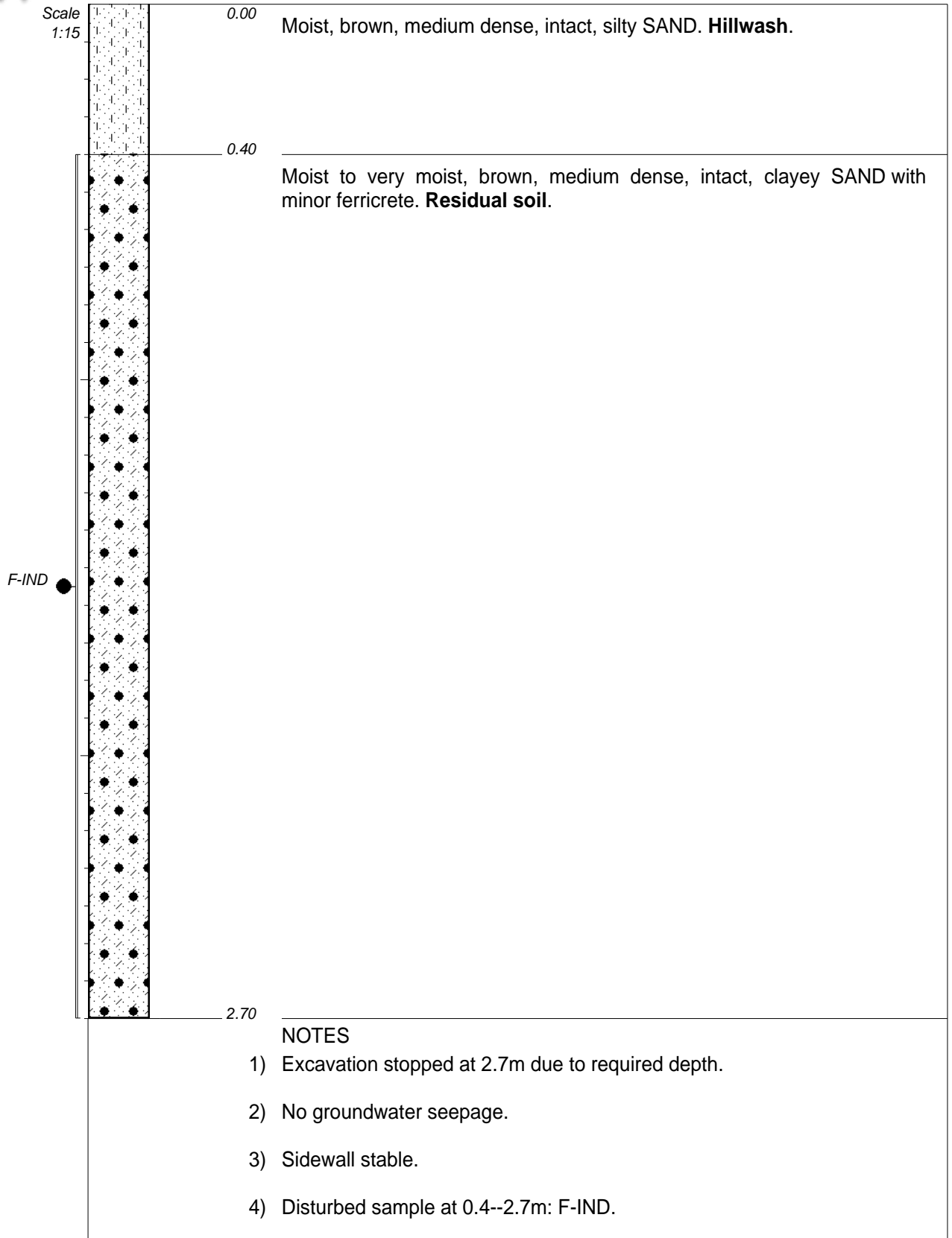
- 1) Excavation stopped at 2.4m due to required depth.
- 2) No groundwater seepage.
- 3) Sidewall stable.
- 4) Disturbed sample at 0.3--2.4m: F-IND.

CONTRACTOR :
MACHINE : BELL TLB
DRILLED BY : Xolani Shabangu
PROFILED BY : Andries Vukeya
TYPE SET BY : Andries Vukeya
SETUP FILE : STANDARD.SET

INCLINATION :
DIAM :
DATE : 22-08-2023
DATE : 20-09-2023
DATE : 06/12/2023 13:50
TEXT : ..ppendixASoilProfiles.txt

ELEVATION : 1648 m
X-COORD : 29.45538°E
Y-COORD : 26.10548°S

HOLE No: KTP37

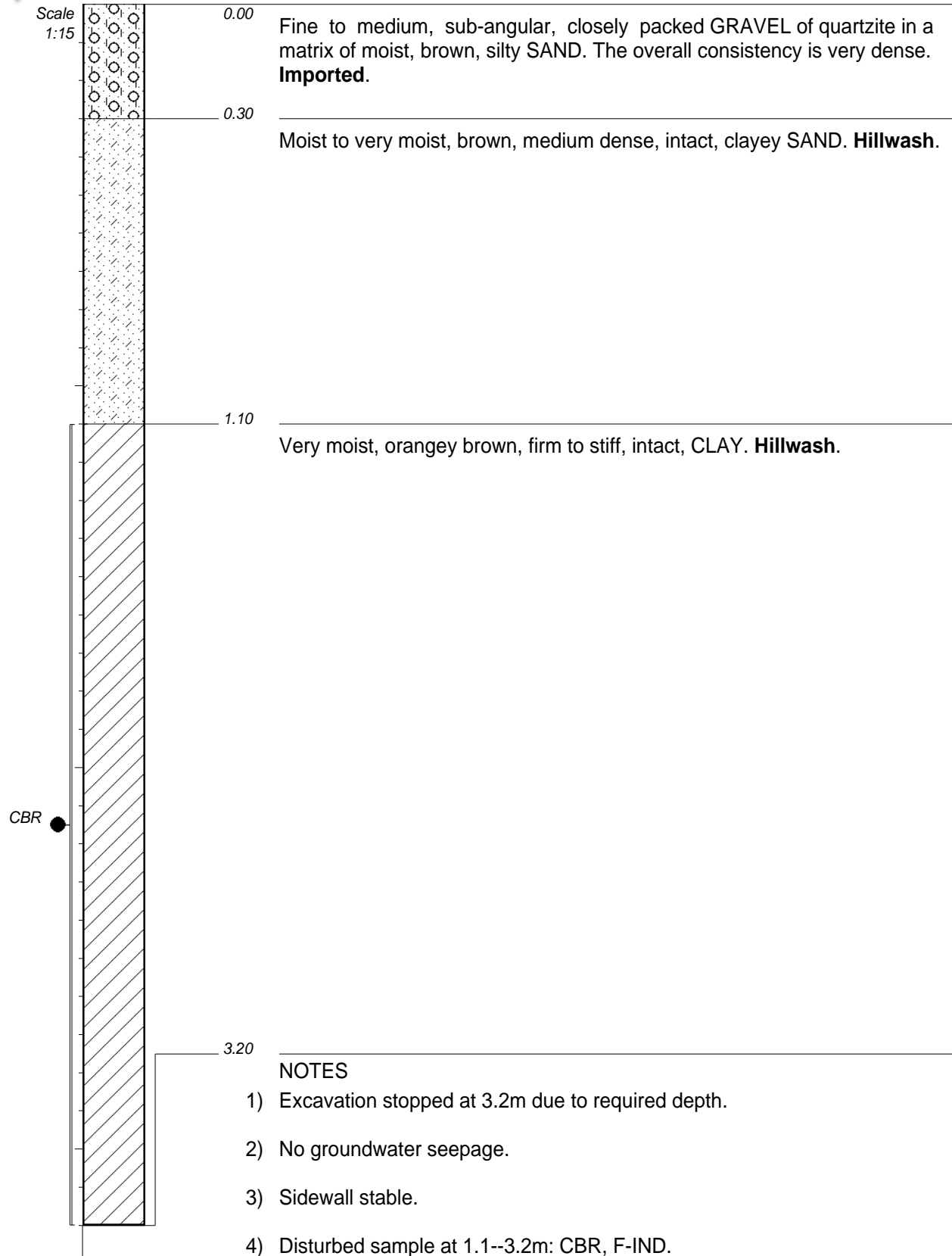


CONTRACTOR :
MACHINE : BELL TLB
DRILLED BY : Xolani Shabangu
PROFILED BY : Andries Vukeya
TYPE SET BY : Andries Vukeya
SETUP FILE : STANDARD.SET

INCLINATION :
DIAM :
DATE : 22-08-2023
DATE : 20-09-2023
DATE : 06/12/2023 13:50
TEXT : ..ppendixASoilProfiles.txt

ELEVATION : 1656 m
X-COORD : 29.45743°E
Y-COORD : 26.10744°S

HOLE No: KTP38

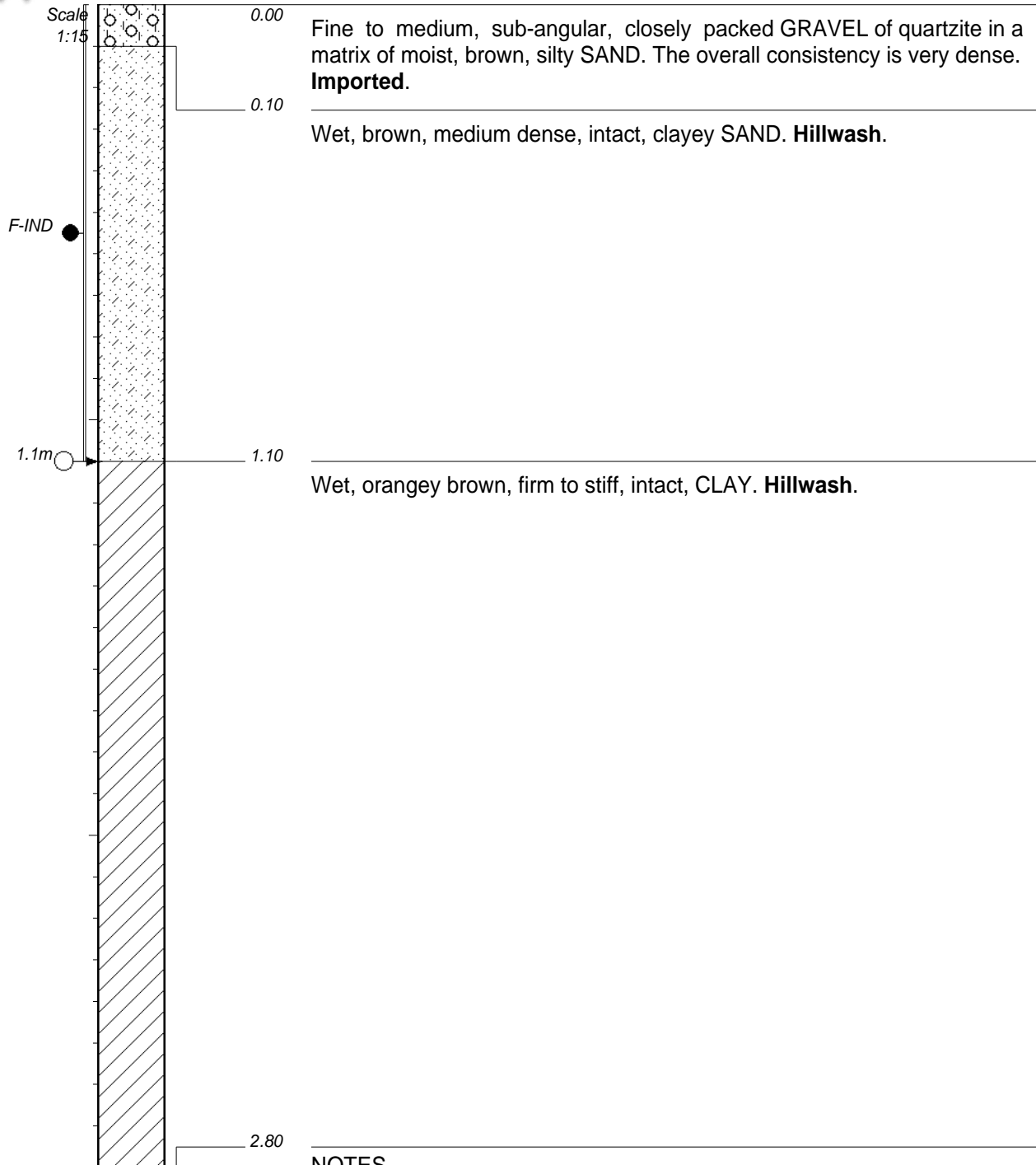


CONTRACTOR :
MACHINE : BELL TLB
DRILLED BY : Xolani Shabangu
PROFILED BY : Andries Vukeya
TYPE SET BY : Andries Vukeya
SETUP FILE : STANDARD.SET

INCLINATION :
DIAM :
DATE : 22-08-2023
DATE : 20-09-2023
DATE : 06/12/2023 13:50
TEXT : ..ppendixASoilProfiles.txt

ELEVATION : 1619 m
X-COORD : 29.47603°E
Y-COORD : 26.09237°S

HOLE No: KTP39



NOTES

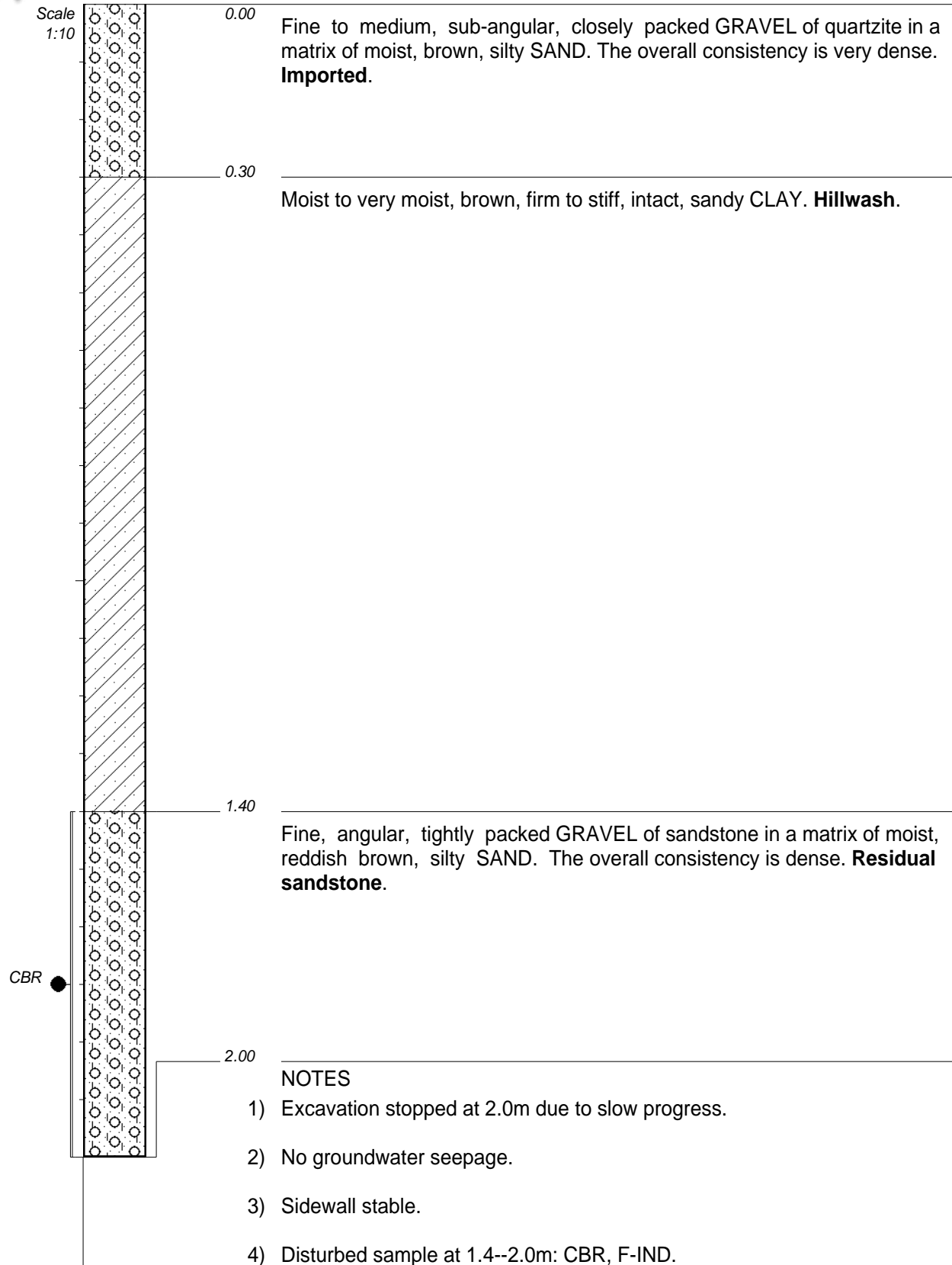
- 1) Excavation stopped at 2.8m due to required depth.
- 2) Groundwater seepage at 1.1m.
- 3) Sidewall stable.
- 4) Disturbed sample at 0.0--1.1m: F-IND.

CONTRACTOR :
MACHINE : BELL TLB
DRILLED BY : Xolani Shabangu
PROFILED BY : Andries Vukeya
TYPE SET BY : Andries Vukeya
SETUP FILE : STANDARD.SET

INCLINATION :
DIAM :
DATE : 22-08-2023
DATE : 20-09-2023
DATE : 06/12/2023 13:50
TEXT : ..ppendixASoilProfiles.txt

ELEVATION : 1623 m
X-COORD : 29.47676°E
Y-COORD : 26.09259°S

HOLE No: KTP40

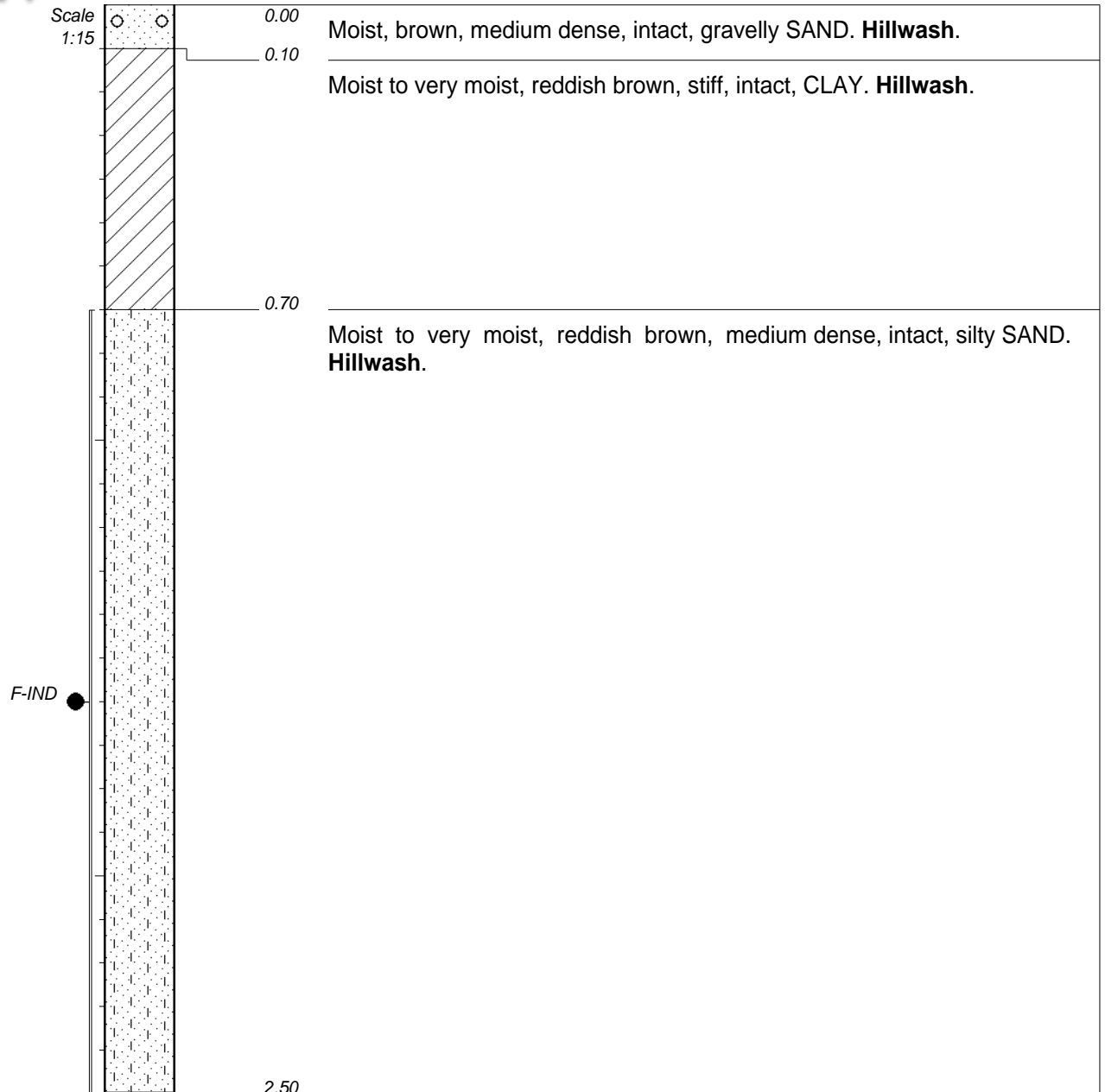


CONTRACTOR :
MACHINE : BELL TLB
DRILLED BY : Xolani Shabangu
PROFILED BY : Andries Vukeya
TYPE SET BY : Andries Vukeya
SETUP FILE : STANDARD.SET

INCLINATION :
DIAM :
DATE : 22-08-2023
DATE : 20-09-2023
DATE : 06/12/2023 13:50
TEXT : ..ppendixASoilProfiles.txt

ELEVATION : 1617 m
X-COORD : 29.47651°E
Y-COORD : 26.09168°S

HOLE No: KTP41


NOTES

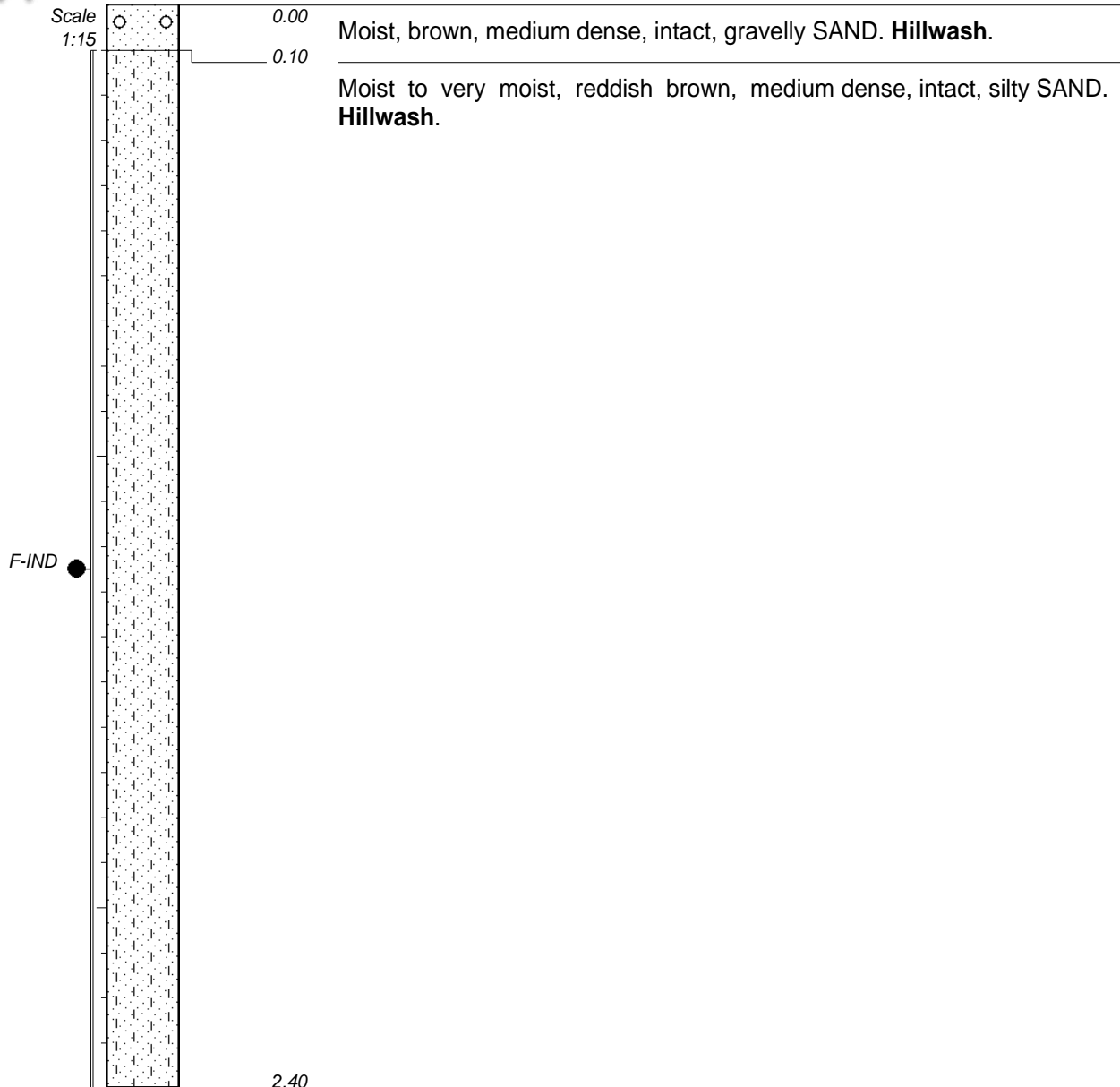
- 1) Excavation stopped at 2.5m due to required depth.
- 2) No groundwater seepage.
- 3) Sidewall stable.
- 4) Disturbed sample at 0.7--2.5m: F-IND.

CONTRACTOR :
 MACHINE : BELL TLB
 DRILLED BY : Xolani Shabangu
 PROFILED BY : Andries Vukeya
 TYPE SET BY : Andries Vukeya
 SETUP FILE : STANDARD.SET

INCLINATION :
 DIAM :
 DATE : 22-08-2023
 DATE : 20-09-2023
 DATE : 06/12/2023 13:50
 TEXT : ..ppendixASoilProfiles.txt

ELEVATION : 1616 m
 X-COORD : 29.46811°E
 Y-COORD : 26.09134°S

HOLE No: KTP42


NOTES

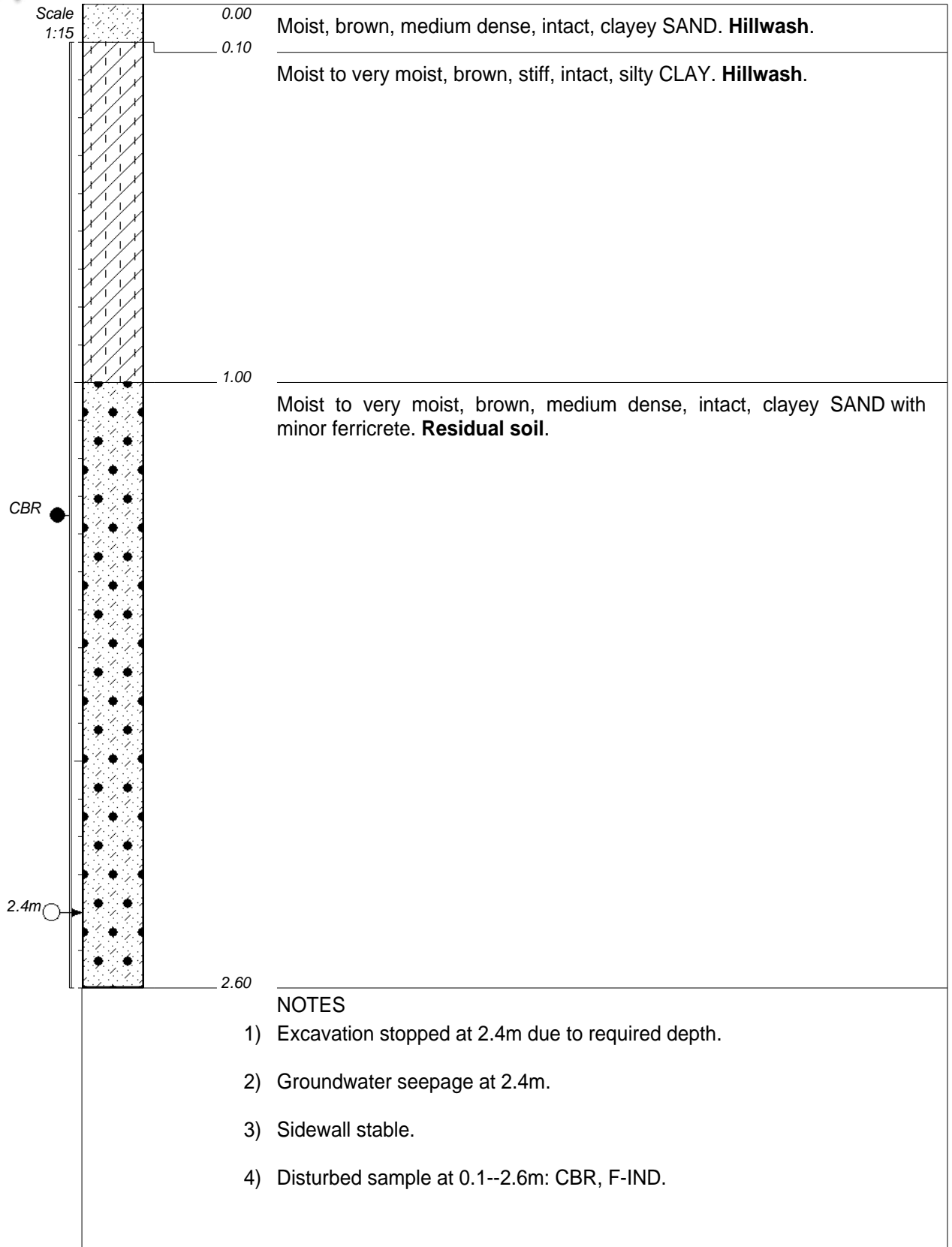
- 1) Excavation stopped at 2.4m due to required depth.
- 2) No groundwater seepage.
- 3) Sidewall stable.
- 4) Disturbed sample at 0.1--2.4m: F-IND.

CONTRACTOR :
 MACHINE : BELL TLB
 DRILLED BY : Xolani Shabangu
 PROFILED BY : Andries Vukeya
 TYPE SET BY : Andries Vukeya
 SETUP FILE : STANDARD.SET

INCLINATION :
 DIAM :
 DATE : 22-08-2023
 DATE : 20-09-2023
 DATE : 06/12/2023 13:50
 TEXT : ..ppendixASoilProfiles.txt

ELEVATION : 1617 m
 X-COORD : 29.46880°E
 Y-COORD : 26.09128°S

HOLE No: KTP43

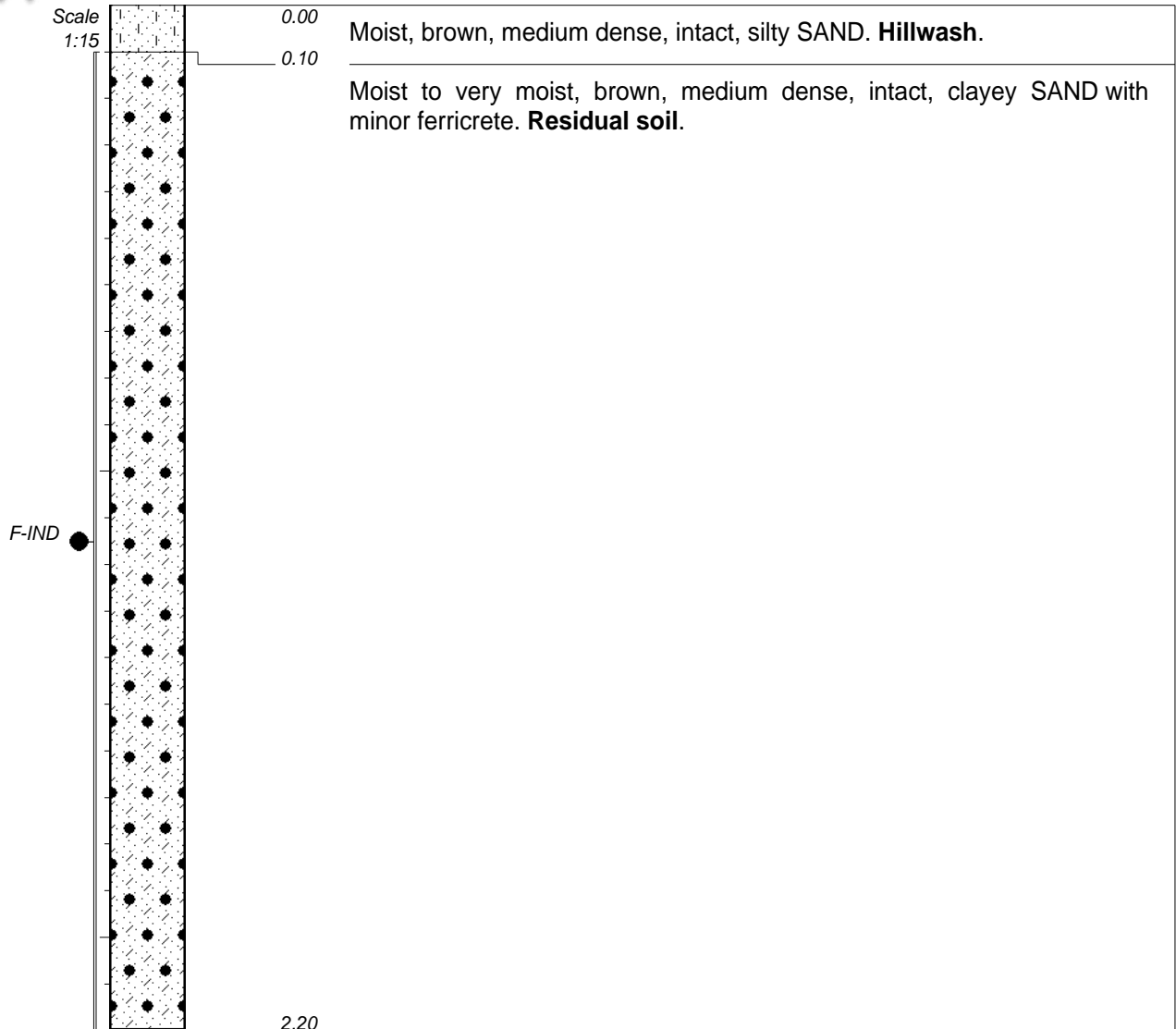


CONTRACTOR :
MACHINE : BELL TLB
DRILLED BY : Xolani Shabangu
PROFILED BY : Andries Vukeya
TYPE SET BY : Andries Vukeya
SETUP FILE : STANDARD.SET

INCLINATION :
DIAM :
DATE : 22-08-2023
DATE : 20-09-2023
DATE : 06/12/2023 13:50
TEXT : ..ppendixASoilProfiles.txt

ELEVATION : 1609 m
X-COORD : 29.46712°E
Y-COORD : 26.09124°S

HOLE No: KTP44



NOTES

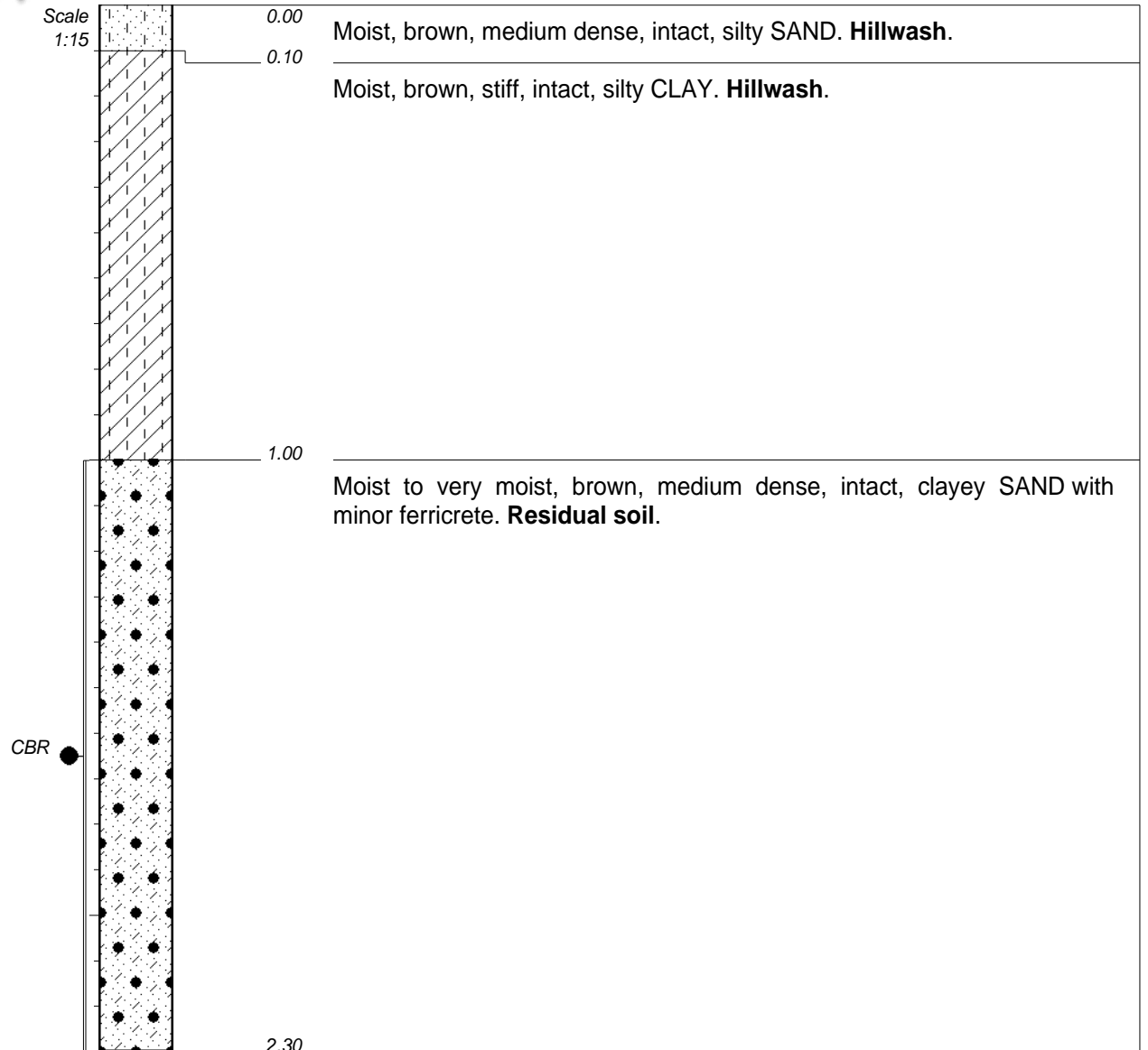
- 1) Excavation stopped at 2.2m due to required depth.
- 2) No groundwater seepage.
- 3) Sidewall stable.
- 4) Disturbed sample at 0.1--2.2m: F-IND.

CONTRACTOR :
MACHINE : BELL TLB
DRILLED BY : Xolani Shabangu
PROFILED BY : Andries Vukeya
TYPE SET BY : Andries Vukeya
SETUP FILE : STANDARD.SET

INCLINATION :
DIAM :
DATE : 22-08-2023
DATE : 20-09-2023
DATE : 06/12/2023 13:50
TEXT : ..ppendixASoilProfiles.txt

ELEVATION : 1622 m
X-COORD : 29.46832°E
Y-COORD : 26.09263°S

HOLE No: KTP45



NOTES

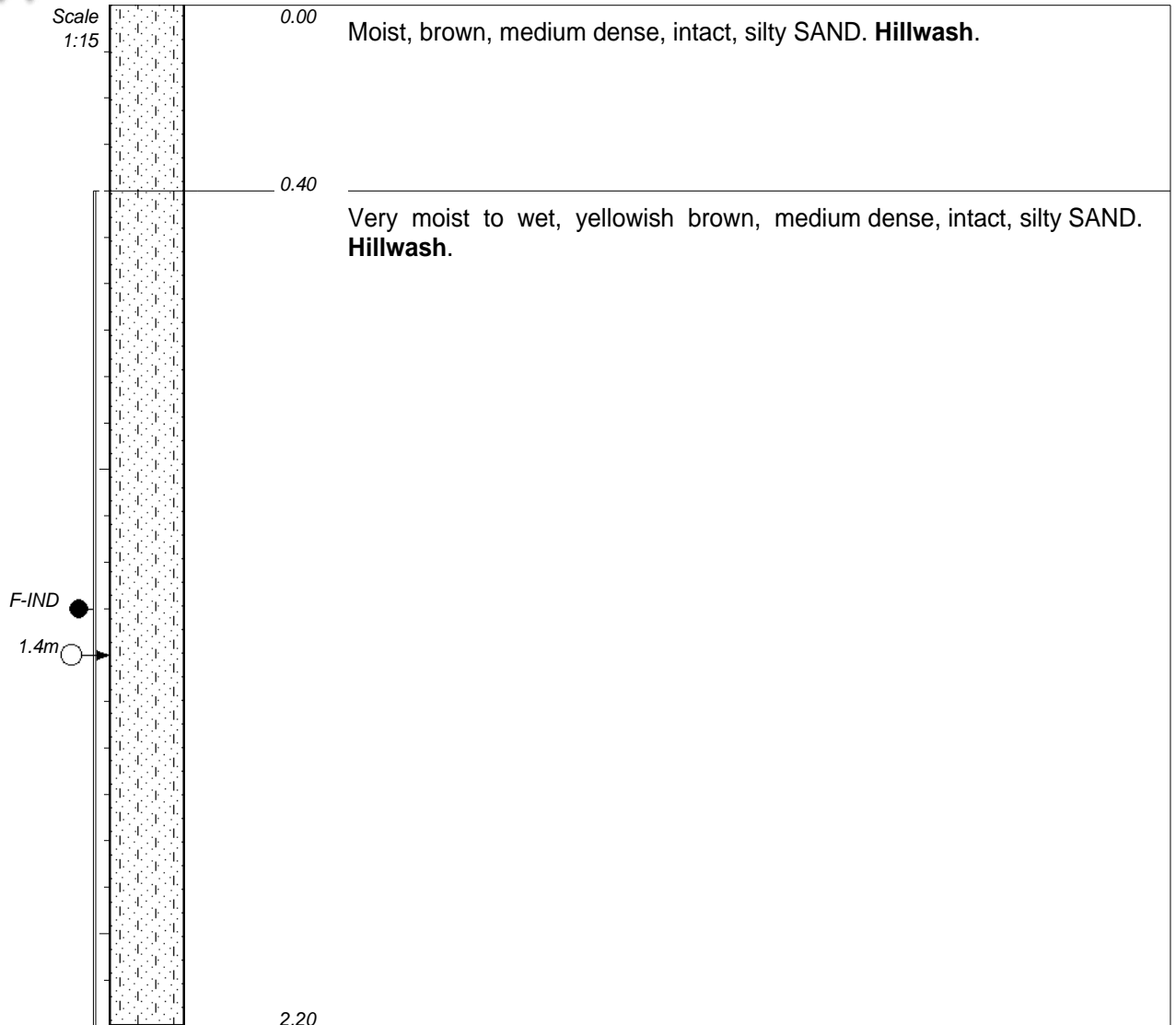
- 1) Excavation stopped at 2.3m due to required depth.
- 2) No groundwater seepage.
- 3) Sidewall stable.
- 4) Disturbed sample at 1.0--2.3m: CBR, F-IND.

CONTRACTOR :
MACHINE : BELL TLB
DRILLED BY : Xolani Shabangu
PROFILED BY : Andries Vukeya
TYPE SET BY : Andries Vukeya
SETUP FILE : STANDARD.SET

INCLINATION :
DIAM :
DATE : 22-08-2023
DATE : 20-09-2023
DATE : 06/12/2023 13:50
TEXT : ..ppendixASoilProfiles.txt

ELEVATION : 1617 m
X-COORD : 29.46762°E
Y-COORD : 26.09255°S

HOLE No: KTP46



NOTES

- 1) Excavation stopped at 2.2m due to required depth.
- 2) Groundwater seepage at 1.4m.
- 3) Sidewall stable.
- 4) Disturbed sample at 0.4--2.2m: F-IND.

CONTRACTOR :
MACHINE : BELL TLB
DRILLED BY : Xolani Shabangu
PROFILED BY : Andries Vukeya
TYPE SET BY : Andries Vukeya
SETUP FILE : STANDARD.SET

INCLINATION :
DIAM :
DATE : 22-08-2023
DATE : 20-09-2023
DATE : 06/12/2023 13:50
TEXT : ..ppendixASoilProfiles.txt

ELEVATION : 1610 m
X-COORD : 29.46650°E
Y-COORD : 26.09230°S

HOLE No: KTP47

APPENDIX B: DYNAMIC CONE PENETROMETER TEST RESULTS

(Use and interpretation of the dynamic cone penetrometer test (DCP), Paige-Green et al (2009)

TEST REPORT FOR DYNAMIC CONE PENETROMETER

BY TEST METHOD TMH6-ST6

CLIENT: ESKOM

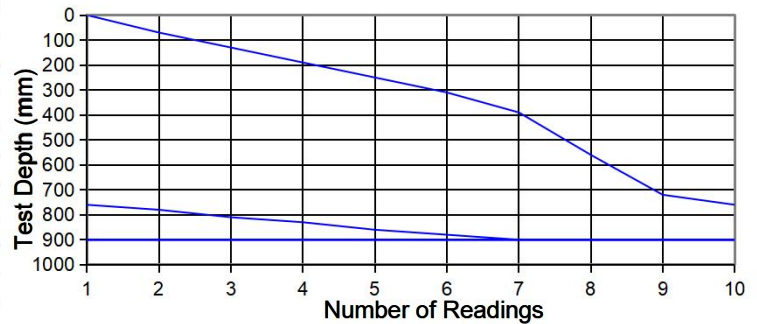
DATE REPORTED: 22/08/2023
 OUR REF: MAK1520823

PROJECT: Geotechnical Investigation: Komati Power Station
 ATTENTION: Eskom
 Tel/Email: 031 350 3370

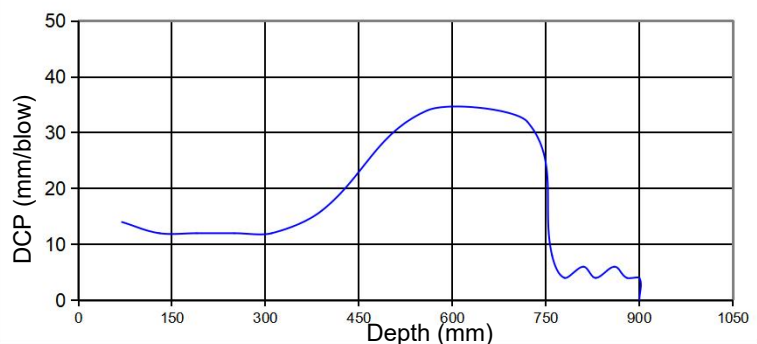
DESCRIPTION: DCP test was done from existing ground level
 DCP No: DCP1 at TP1

DATE TESTED: 22/08/2023

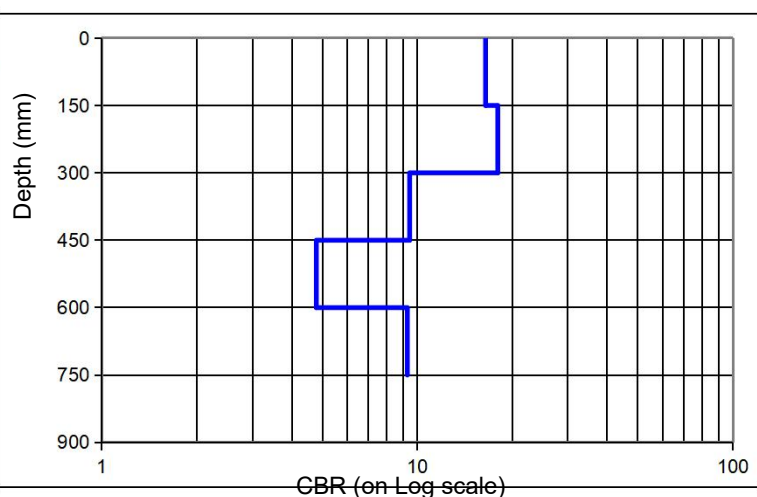
DCP Readings		Blows per reading: 5							
no.	mm	no.	mm	no.	mm	no.	mm	no.	mm
1	70	11	810	21		31		41	
2	130	12	830	22		32		42	
3	190	13	860	23		33		43	
4	250	14	880	24		34		44	
5	310	15	900	25		35		45	
6	390	16		26		36		46	
7	560	17		27		37		47	
8	720	18		28		38		48	
9	760	19		29		39		49	
10	780	20		30		40		50	



DCP number (mm / Blow) DN									
DN	mm	DN	mm	DN	mm	DN	mm	DN	mm
14	70	6	810						
12	130	4	830						
12	190	6	860						
12	250	4	880						
12	310	4	900						
16	390								
34	560								
32	720								
8	760								
4	780								



Depth (mm)				In situ		Blows/mm	
no.	From	-	To	DN	CBR	150mm	300mm
1	1	-	150	12,9	16	12	12,1
2	151	-	300	12,0	18	13	
3	301	-	450	19,7	9	8	
4	451	-	600	33,4	5	4	6,0
5	601	-	750	20,0	9	8	
6	751	-	900				
8	901	-	1050				
9	1051	-	1200				
10	1201	-	1350				
11	1351	-	1500				
12	1501	-	1650				
13	1651	-	1800				
14	1801	-	1950				
15	1951	-	2100				



REMARKS: Max penetration depth 900 mm

Where values are presented as continued "cont", refer to the adjacent graphs where DN values vs depth and penetration vs depths can be read.

Signature 

TEST REPORT FOR DYNAMIC CONE PENETROMETER

BY TEST METHOD TMH6-ST6

CLIENT: ESKOM

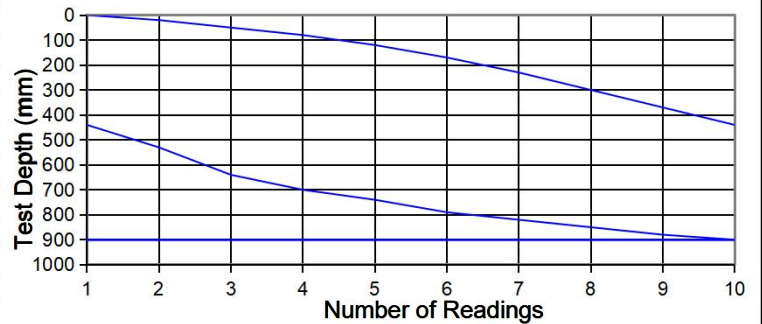
DATE REPORTED: 22/08/2023
 OUR REF: MAK1520823

PROJECT: Geotechnical Investigation: Komati Power Station
 ATTENTION: Eskom
 Tel/Email: 031 350 3370

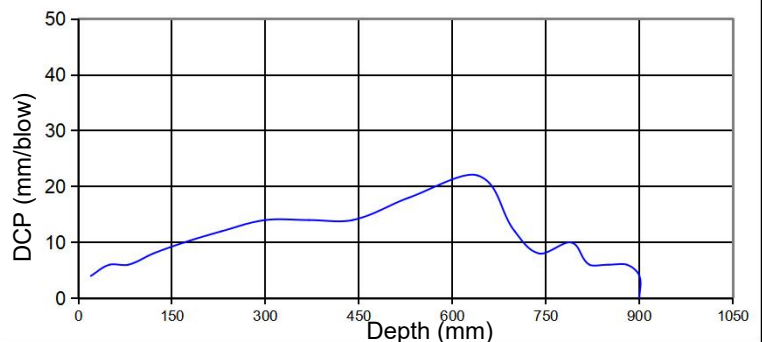
DESCRIPTION: DCP test was done from existing ground level
 DCP No: DCP2 at TP2

DATE TESTED: 22/08/2023

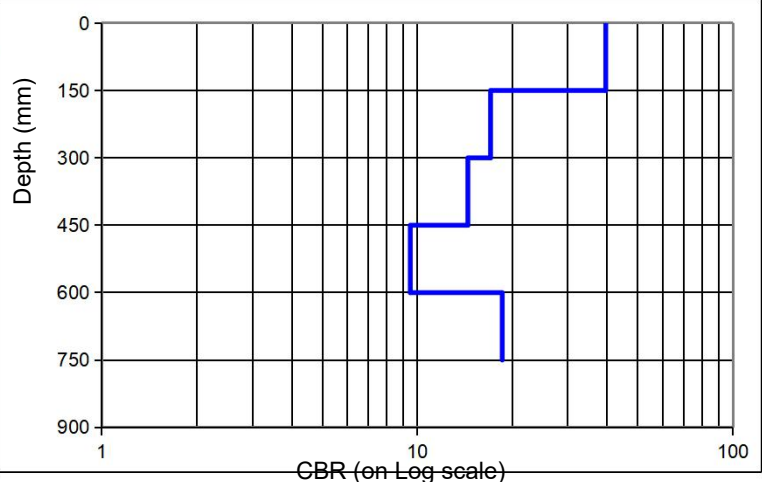
DCP Readings		Blows per reading: 5							
no.	mm	no.	mm	no.	mm	no.	mm	no.	mm
1	20	11	640	21		31		41	
2	50	12	700	22		32		42	
3	80	13	740	23		33		43	
4	120	14	790	24		34		44	
5	170	15	820	25		35		45	
6	230	16	850	26		36		46	
7	300	17	880	27		37		47	
8	370	18	900	28		38		48	
9	440	19		29		39		49	
10	530	20		30		40		50	



DCP number (mm / Blow) DN									
DN	mm	DN	mm	DN	mm	DN	mm	DN	mm
4	20	22	640						
6	50	12	700						
6	80	8	740						
8	120	10	790						
10	170	6	820						
12	230	6	850						
14	300	6	880						
14	370	4	900						
14	440								
18	530								



Depth (mm)				In situ		Blows/mm	
no.	From	-	To	DN	CBR	150mm	300mm
1	1	-	150	6,5	40	23	17,5
2	151	-	300	12,5	17	12	
3	301	-	450	14,2	14	11	9,1
4	451	-	600	19,7	9	8	
5	601	-	750	11,7	19	13	
6	751	-	900				
8	901	-	1050				
9	1051	-	1200				
10	1201	-	1350				
11	1351	-	1500				
12	1501	-	1650				
13	1651	-	1800				
14	1801	-	1950				
15	1951	-	2100				



REMARKS: Max penetration depth 900 mm

Where values are presented as continued "cont", refer to the adjacent graphs where DN values vs depth and penetration vs depths can be read.

Signature 

TEST REPORT FOR DYNAMIC CONE PENETROMETER

BY TEST METHOD TMH6-ST6

CLIENT: ESKOM

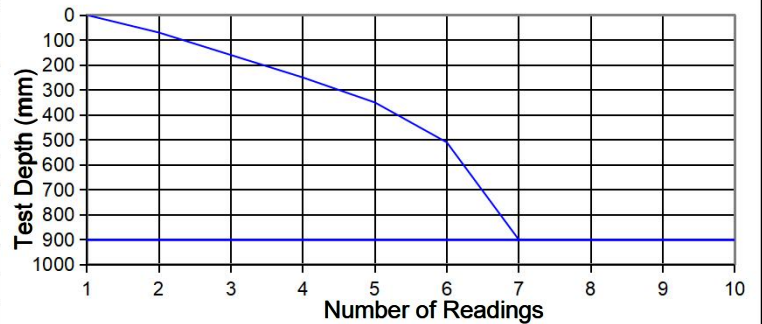
DATE REPORTED: 22/08/2023
 OUR REF: MAK1520823

PROJECT: Geotechnical Investigation: Komati Power Station
 ATTENTION: Eskom
 Tel/Email: 031 350 3370

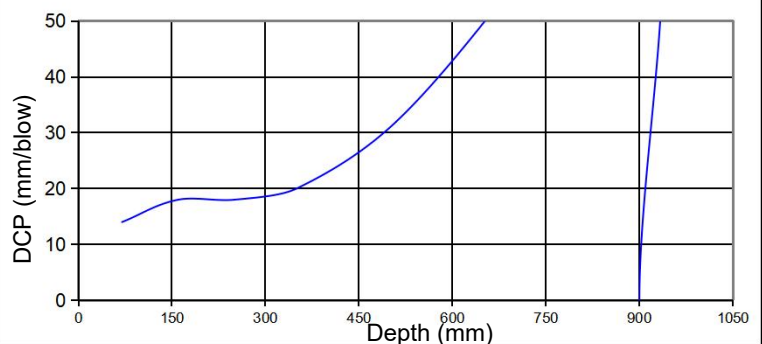
DESCRIPTION: DCP test was done from existing ground level
 DCP No: DCP3 at TP3

DATE TESTED: 22/08/2023

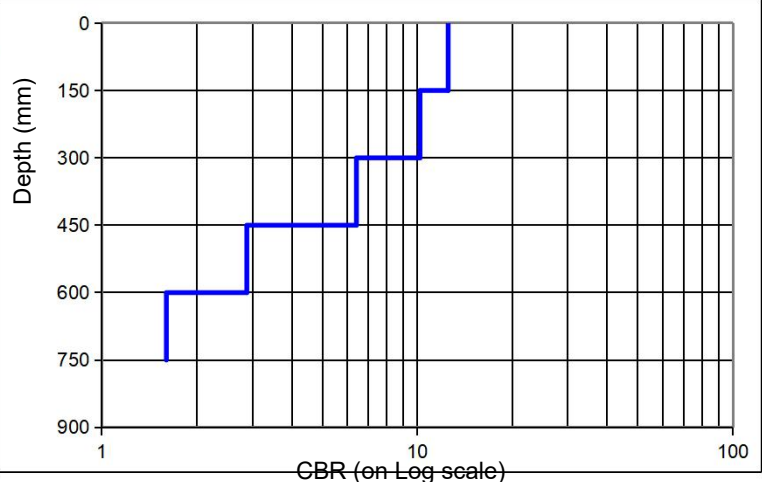
DCP Readings		Blows per reading: 5							
no.	mm	no.	mm	no.	mm	no.	mm	no.	mm
1	70	11		21		31		41	
2	160	12		22		32		42	
3	250	13		23		33		43	
4	350	14		24		34		44	
5	510	15		25		35		45	
6	900	16		26		36		46	
7		17		27		37		47	
8		18		28		38		48	
9		19		29		39		49	
10		20		30		40		50	



DCP number (mm / Blow) DN									
DN	mm	DN	mm	DN	mm	DN	mm	DN	mm
14	70								
18	160								
18	250								
20	350								
32	510								
78	900								



Depth (mm)				In situ		Blows/mm	
no.	From	-	To	DN	CBR	150mm	300mm
1	1	-	150	15,9	13	9	8,8
2	151	-	300	18,6	10	8	
3	301	-	450	26,7	6	6	4,3
4	451	-	600	49,5	3	3	
5	601	-	750	78,0	2	2	
6	751	-	900				
8	901	-	1050				
9	1051	-	1200				
10	1201	-	1350				
11	1351	-	1500				
12	1501	-	1650				
13	1651	-	1800				
14	1801	-	1950				
15	1951	-	2100				



REMARKS: Max penetration depth 900 mm

Where values are presented as continued "cont", refer to the adjacent graphs where DN values vs depth and penetration vs depths can be read.

Signature 

TEST REPORT FOR DYNAMIC CONE PENETROMETER

BY TEST METHOD TMH6-ST6

CLIENT: ESKOM

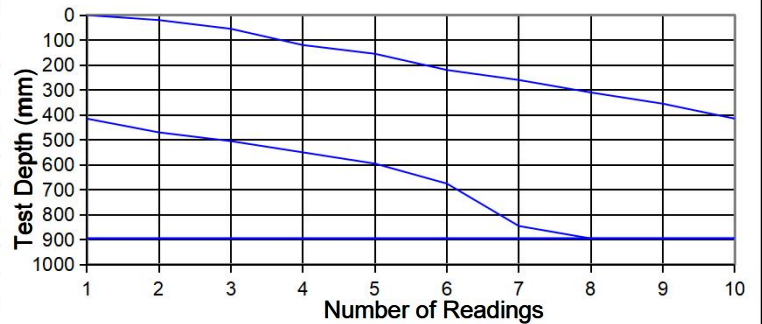
DATE REPORTED: 22/08/2023
 OUR REF: MAK1520823

PROJECT: Geotechnical Investigation: Komati Power Station
 ATTENTION: Eskom
 Tel/Email: 031 350 3370

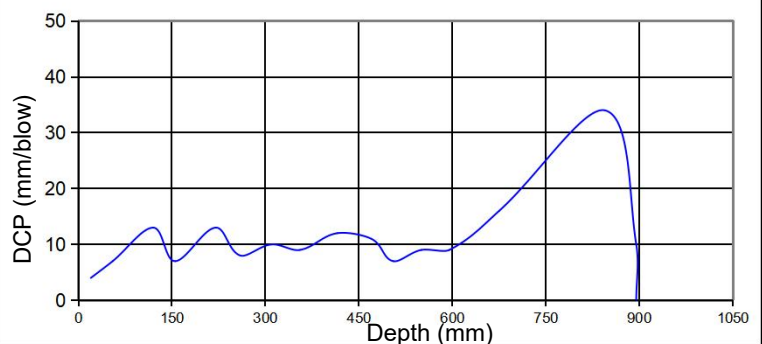
DESCRIPTION: DCP test was done from existing ground level
 DCP No: DCP4 at TP4

DATE TESTED: 22/08/2023

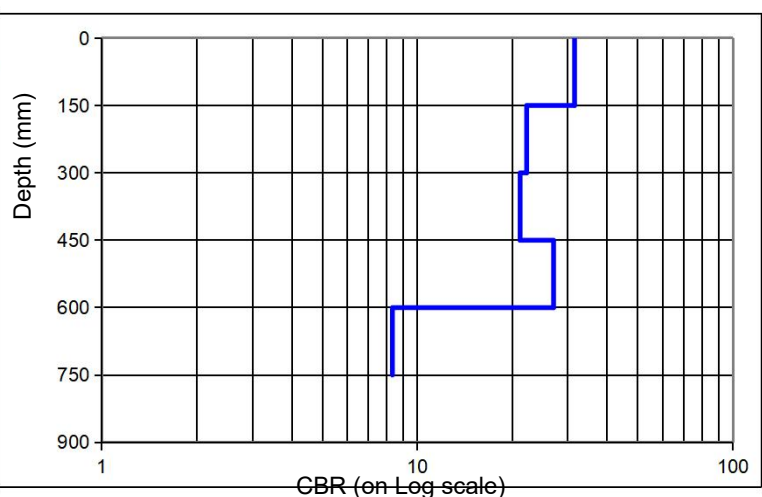
DCP Readings		Blows per reading: 5							
no.	mm	no.	mm	no.	mm	no.	mm	no.	mm
1	20	11	505	21		31		41	
2	55	12	550	22		32		42	
3	120	13	595	23		33		43	
4	155	14	675	24		34		44	
5	220	15	845	25		35		45	
6	260	16	895	26		36		46	
7	310	17		27		37		47	
8	355	18		28		38		48	
9	415	19		29		39		49	
10	470	20		30		40		50	



DCP number (mm / Blow) DN									
DN	mm	DN	mm	DN	mm	DN	mm	DN	mm
4	20	7	505						
7	55	9	550						
13	120	9	595						
7	155	16	675						
13	220	34	845						
8	260	10	895						
10	310								
9	355								
12	415								
11	470								



Depth (mm)				In situ		Blows/mm	
no.	From	-	To	DN	CBR	150mm	300mm
1	1	-	150	7,8	32	19	17,0
2	151	-	300	10,2	22	15	
3	301	-	450	10,6	21	14	
4	451	-	600	8,8	27	17	15,7
5	601	-	750	21,8	8	7	
6	751	-	900				
8	901	-	1050				
9	1051	-	1200				
10	1201	-	1350				
11	1351	-	1500				
12	1501	-	1650				
13	1651	-	1800				
14	1801	-	1950				
15	1951	-	2100				



REMARKS:

Max penetration depth **895 mm**

Where values are presented as continued "cont", refer to the adjacent graphs where DN values vs depth and penetration vs depths can be read.

Signature

TEST REPORT FOR DYNAMIC CONE PENETROMETER

BY TEST METHOD TMH6-ST6

CLIENT: ESKOM

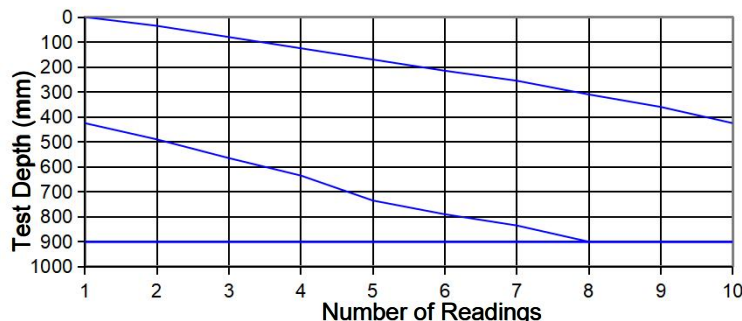
DATE REPORTED: 22/08/2023
 OUR REF: MAK1520823

PROJECT: Geotechnical Investigation: Komati Power Station
 ATTENTION: Eskom
 Tel/Email: 031 350 3370

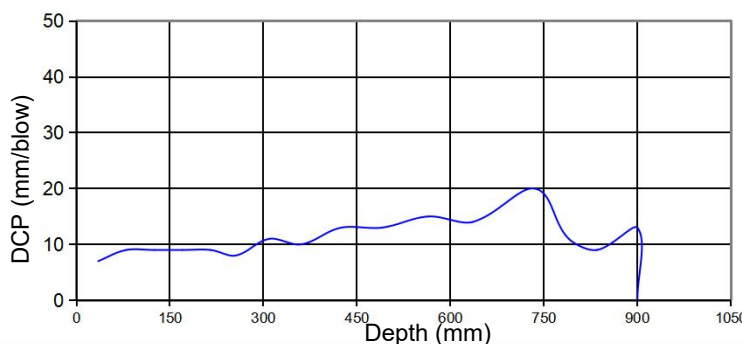
DESCRIPTION: DCP test was done from existing ground level
 DCP No: DCP5 at TP5

DATE TESTED: 22/08/2023

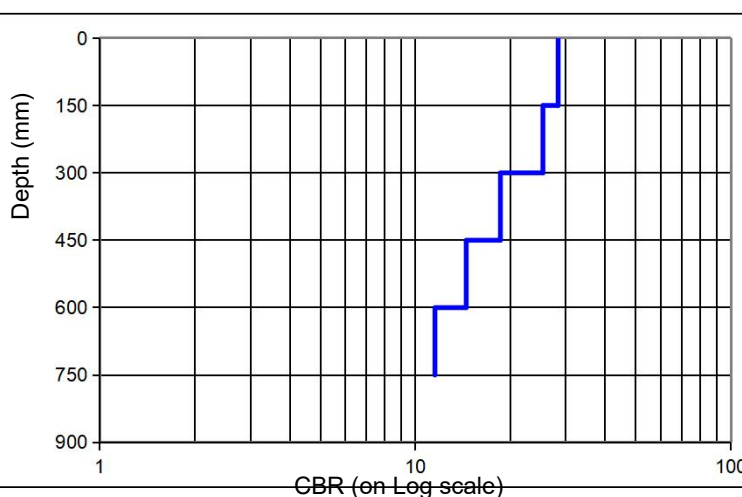
DCP Readings		Blows per reading: 5							
no.	mm	no.	mm	no.	mm	no.	mm	no.	mm
1	35	11	565	21		31		41	
2	80	12	635	22		32		42	
3	125	13	735	23		33		43	
4	170	14	790	24		34		44	
5	215	15	835	25		35		45	
6	255	16	900	26		36		46	
7	310	17		27		37		47	
8	360	18		28		38		48	
9	425	19		29		39		49	
10	490	20		30		40		50	



DCP number (mm / Blow) DN									
DN	mm	DN	mm	DN	mm	DN	mm	DN	mm
7	35	15	565						
9	80	14	635						
9	125	20	735						
9	170	11	790						
9	215	9	835						
8	255	13	900						
11	310								
10	360								
13	425								
13	490								



Depth (mm)				In situ		Blows/mm	
no.	From	-	To	DN	CBR	150mm	300mm
1	1	-	150	8,4	28	18	17,0
2	151	-	300	9,2	25	16	
3	301	-	450	11,7	19	13	11,7
4	451	-	600	14,2	14	11	
5	601	-	750	16,9	12	9	
6	751	-	900				
8	901	-	1050				
9	1051	-	1200				
10	1201	-	1350				
11	1351	-	1500				
12	1501	-	1650				
13	1651	-	1800				
14	1801	-	1950				
15	1951	-	2100				



REMARKS:

Max penetration depth 900 mm

Where values are presented as continued "cont", refer to the adjacent graphs where DN values vs depth and penetration vs depths can be read.

Signature

TEST REPORT FOR DYNAMIC CONE PENETROMETER

BY TEST METHOD TMH6-ST6

CLIENT: ESKOM

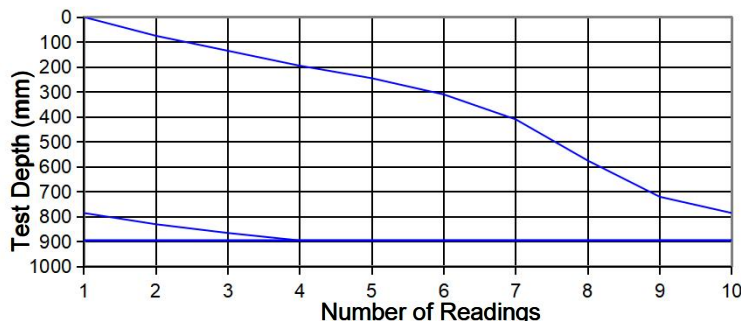
DATE REPORTED: 22/08/2023
 OUR REF: MAK1520823

PROJECT: Geotechnical Investigation: Komati Power Station
 ATTENTION: Eskom
 Tel/Email: 031 350 3370

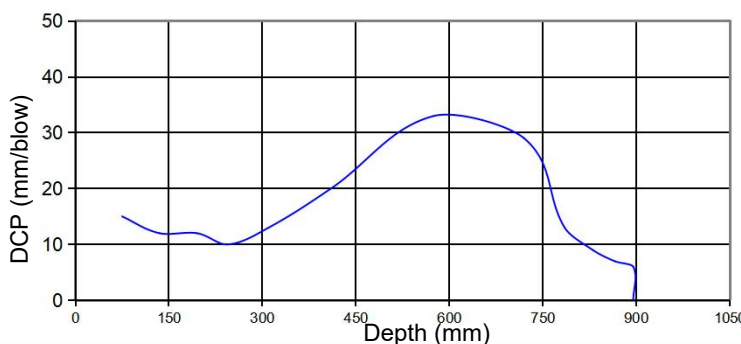
DESCRIPTION: DCP test was done from existing ground level
 DCP No: DCP6 at TP6

DATE TESTED: 22/08/2023

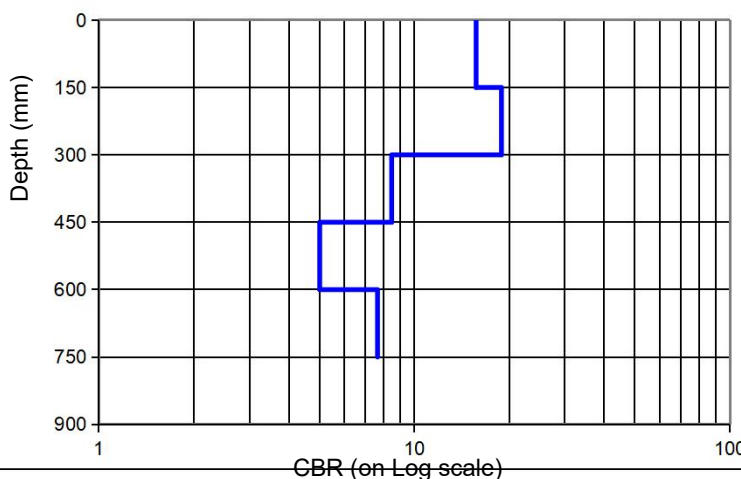
DCP Readings		Blows per reading: 5							
no.	mm	no.	mm	no.	mm	no.	mm	no.	mm
1	75	11	865	21		31		41	
2	135	12	895	22		32		42	
3	195	13		23		33		43	
4	245	14		24		34		44	
5	310	15		25		35		45	
6	410	16		26		36		46	
7	575	17		27		37		47	
8	720	18		28		38		48	
9	785	19		29		39		49	
10	830	20		30		40		50	



DCP number (mm / Blow) DN									
DN	mm	DN	mm	DN	mm	DN	mm	DN	mm
15	75	7	865						
12	135	6	895						
12	195								
10	245								
13	310								
20	410								
33	575								
29	720								
13	785								
9	830								



Depth (mm)				In situ		Blows/mm	
no.	From	-	To	DN	CBR	150mm	300mm
1	1	-	150	13,3	16	11	12,1
2	151	-	300	11,6	19	13	
3	301	-	450	21,5	8	7	5,8
4	451	-	600	32,3	5	5	
5	601	-	750	23,3	8	6	
6	751	-	900				
8	901	-	1050				
9	1051	-	1200				
10	1201	-	1350				
11	1351	-	1500				
12	1501	-	1650				
13	1651	-	1800				
14	1801	-	1950				
15	1951	-	2100				



REMARKS:

Max penetration depth **895 mm**

Where values are presented as continued "cont", refer to the adjacent graphs where DN values vs depth and penetration vs depths can be read.

Signature

TEST REPORT FOR DYNAMIC CONE PENETROMETER

BY TEST METHOD TMH6-ST6

CLIENT: ESKOM

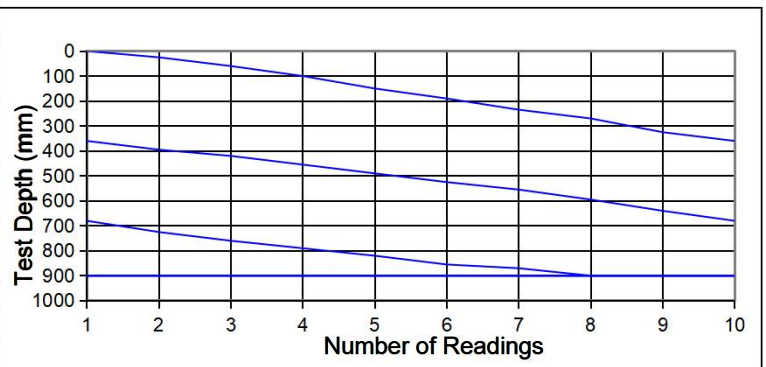
DATE REPORTED: 22/08/2023
 OUR REF: MAK1520823

PROJECT: Geotechnical Investigation: Komati Power Station
 ATTENTION: Eskom
 Tel/Email: 031 350 3370

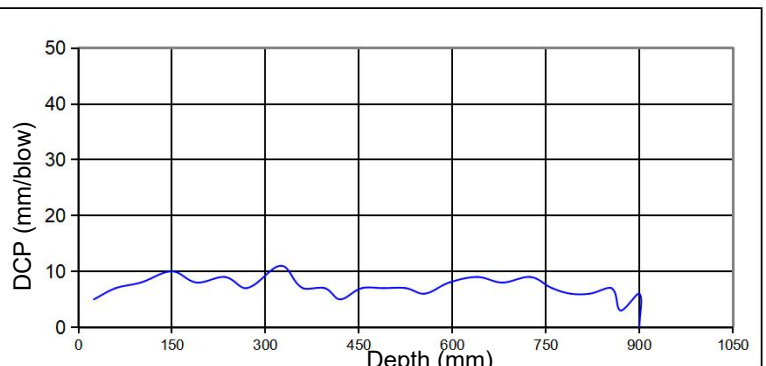
DESCRIPTION: DCP test was done from existing ground level
 DCP No: DCP7 at TP7

DATE TESTED: 22/08/2023

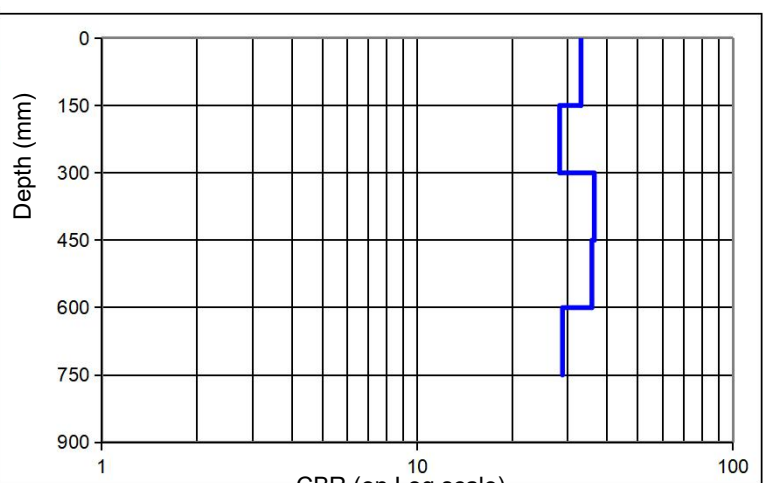
DCP Readings		Blows per reading: 5							
no.	mm	no.	mm	no.	mm	no.	mm	no.	mm
1	25	11	420	21	790	31		41	
2	60	12	455	22	820	32		42	
3	100	13	490	23	855	33		43	
4	150	14	525	24	870	34		44	
5	190	15	555	25	900	35		45	
6	235	16	595	26		36		46	
7	270	17	640	27		37		47	
8	325	18	680	28		38		48	
9	360	19	725	29		39		49	
10	395	20	760	30		40		50	



DCP number (mm / Blow) DN									
DN	mm	DN	mm	DN	mm	DN	mm	DN	mm
5	25	5	420	6	790				
7	60	7	455	6	820				
8	100	7	490	7	855				
10	150	7	525	3	870				
8	190	6	555	6	900				
9	235	8	595						
7	270	9	640						
11	325	8	680						
7	360	9	725						
7	395	7	760						



Depth (mm)				In situ		Blows/mm	
no.	From	-	To	DN	CBR	150mm	300mm
1	1	-	150	7,5	33	20	18,9
2	151	-	300	8,5	28	18	
3	301	-	450	7,0	36	22	21,4
4	451	-	600	7,1	36	21	
5	601	-	750	8,3	29	18	
6	751	-	900				
8	901	-	1050				
9	1051	-	1200				
10	1201	-	1350				
11	1351	-	1500				
12	1501	-	1650				
13	1651	-	1800				
14	1801	-	1950				
15	1951	-	2100				



REMARKS: Max penetration depth 900 mm

Where values are presented as continued "cont", refer to the adjacent graphs where DN values vs depth and penetration vs depths can be read.

Signature 

TEST REPORT FOR DYNAMIC CONE PENETROMETER

BY TEST METHOD TMH6-ST6

CLIENT: ESKOM

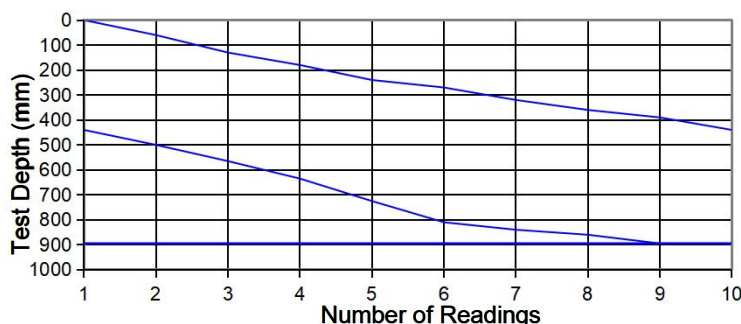
DATE REPORTED: 22/08/2023
 OUR REF: MAK1520823

PROJECT: Geotechnical Investigation: Komati Power Station
 ATTENTION: Eskom
 Tel/Email: 031 350 3370

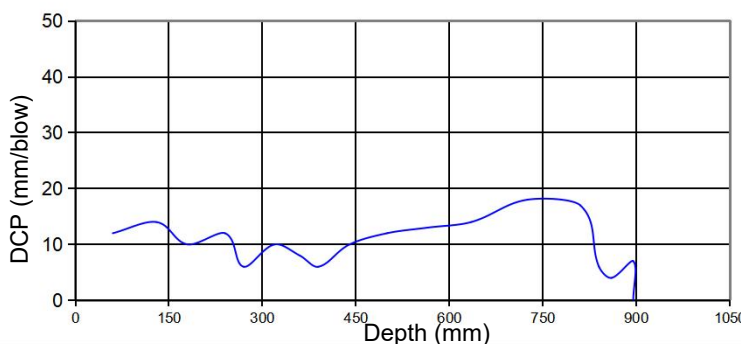
DESCRIPTION: DCP test was done from existing ground level
 DCP No: DCP8 at TP8

DATE TESTED: 22/08/2023

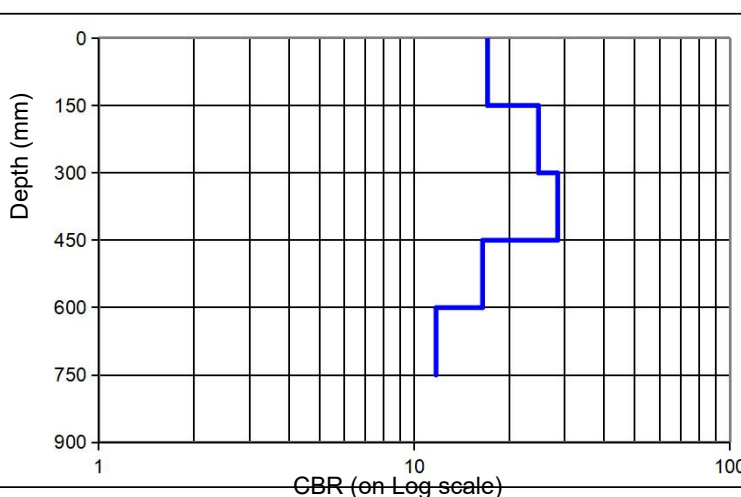
DCP Readings		Blows per reading: 5							
no.	mm	no.	mm	no.	mm	no.	mm	no.	mm
1	60	11	565	21		31		41	
2	130	12	635	22		32		42	
3	180	13	725	23		33		43	
4	240	14	810	24		34		44	
5	270	15	840	25		35		45	
6	320	16	860	26		36		46	
7	360	17	895	27		37		47	
8	390	18		28		38		48	
9	440	19		29		39		49	
10	500	20		30		40		50	



DCP number (mm / Blow) DN									
DN	mm	DN	mm	DN	mm	DN	mm	DN	mm
12	60	13	565						
14	130	14	635						
10	180	18	725						
12	240	17	810						
6	270	6	840						
10	320	4	860						
8	360	7	895						
6	390								
10	440								
12	500								



Depth (mm)				In situ		Blows/mm	
no.	From	-	To	DN	CBR	150mm	300mm
1	1	-	150	12,5	17	12	14,0
2	151	-	300	9,4	25	16	
3	301	-	450	8,4	28	18	14,8
4	451	-	600	12,9	16	12	
5	601	-	750	16,7	12	9	
6	751	-	900				
8	901	-	1050				
9	1051	-	1200				
10	1201	-	1350				
11	1351	-	1500				
12	1501	-	1650				
13	1651	-	1800				
14	1801	-	1950				
15	1951	-	2100				



REMARKS:

Max penetration depth **895 mm**

Where values are presented as continued "cont", refer to the adjacent graphs where DN values vs depth and penetration vs depths can be read.

Signature

TEST REPORT FOR DYNAMIC CONE PENETROMETER

BY TEST METHOD TMH6-ST6

CLIENT: ESKOM

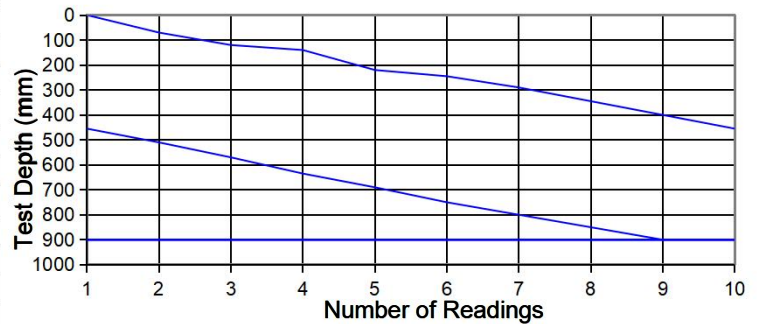
DATE REPORTED: 22/08/2023
 OUR REF: MAK1520823

PROJECT: Geotechnical Investigation: Komati Power Station
 ATTENTION: Eskom
 Tel/Email: 031 350 3370

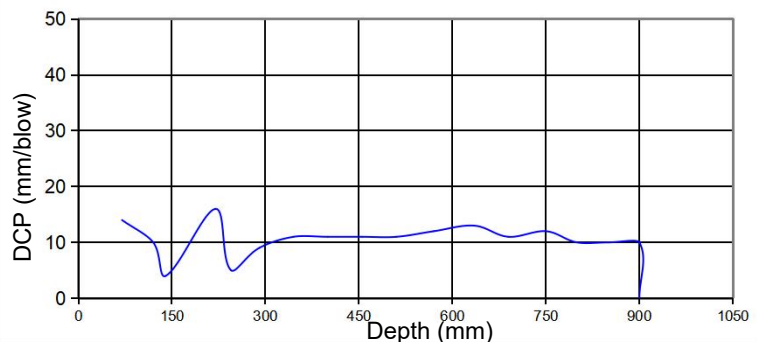
DESCRIPTION: DCP test was done from existing ground level
 DCP No: DCP9 at TP9

DATE TESTED: 22/08/2023

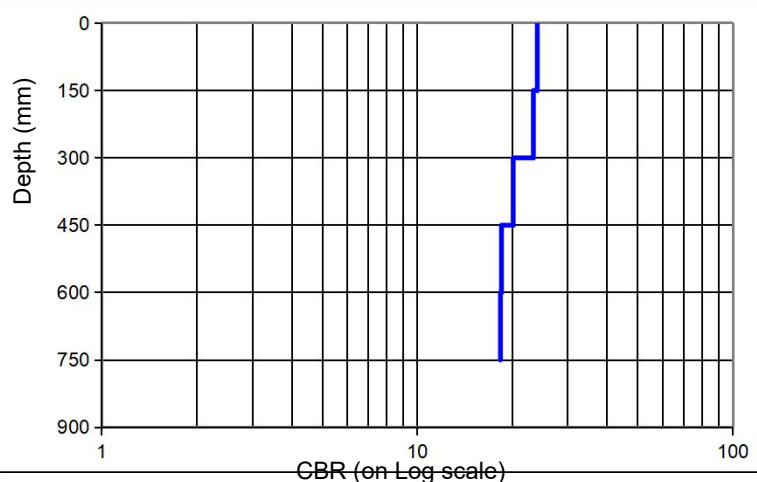
DCP Readings		Blows per reading: 5							
no.	mm	no.	mm	no.	mm	no.	mm	no.	mm
1	70	11	570	21		31		41	
2	120	12	635	22		32		42	
3	140	13	690	23		33		43	
4	220	14	750	24		34		44	
5	245	15	800	25		35		45	
6	290	16	850	26		36		46	
7	345	17	900	27		37		47	
8	400	18		28		38		48	
9	455	19		29		39		49	
10	510	20		30		40		50	



DCP number (mm / Blow) DN									
DN	mm	DN	mm	DN	mm	DN	mm	DN	mm
14	70	12	570						
10	120	13	635						
4	140	11	690						
16	220	12	750						
5	245	10	800						
9	290	10	850						
11	345	10	900						
11	400								
11	455								
11	510								



Depth (mm)				In situ		Blows/mm	
no.	From	-	To	DN	CBR	150mm	300mm
1	1	-	150	9,6	24	16	15,5
2	151	-	300	9,8	23	15	
3	301	-	450	11,0	20	14	13,2
4	451	-	600	11,8	18	13	
5	601	-	750	11,8	18	13	
6	751	-	900				
8	901	-	1050				
9	1051	-	1200				
10	1201	-	1350				
11	1351	-	1500				
12	1501	-	1650				
13	1651	-	1800				
14	1801	-	1950				
15	1951	-	2100				



REMARKS: Max penetration depth 900 mm

Where values are presented as continued "cont", refer to the adjacent graphs where DN values vs depth and penetration vs depths can be read.

Signature 

TEST REPORT FOR DYNAMIC CONE PENETROMETER

BY TEST METHOD TMH6-ST6

CLIENT: ESKOM

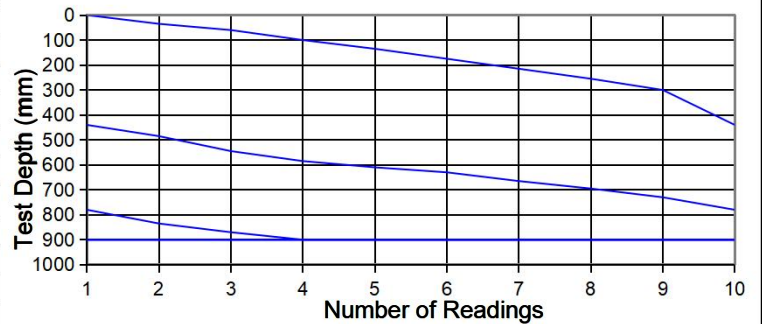
DATE REPORTED: 22/08/2023
 OUR REF: MAK1520823

PROJECT: Geotechnical Investigation: Komati Power Station
 ATTENTION: Eskom
 Tel/Email: 031 350 3370

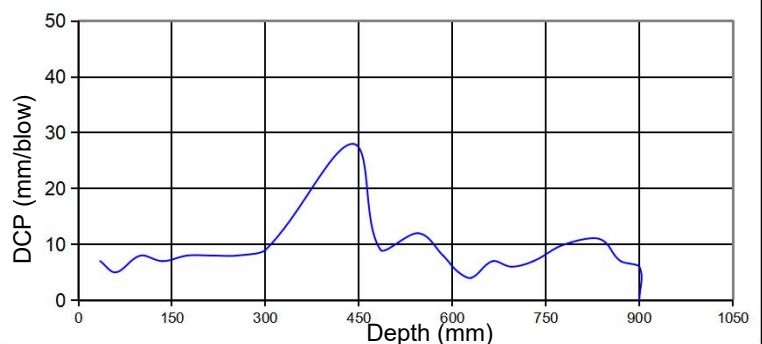
DESCRIPTION: DCP test was done from existing ground level
 DCP No: DCP10 at TP10

DATE TESTED: 22/08/2023

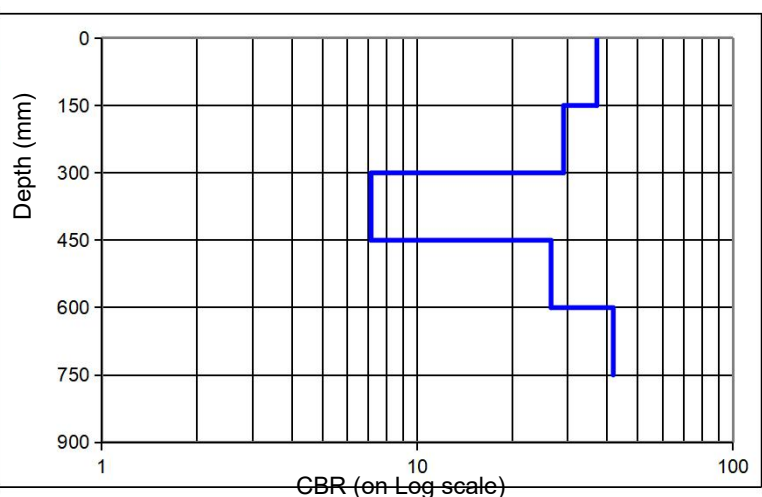
DCP Readings		Blows per reading: 5							
no.	mm	no.	mm	no.	mm	no.	mm	no.	mm
1	35	11	545	21	900	31		41	
2	60	12	585	22		32		42	
3	100	13	610	23		33		43	
4	135	14	630	24		34		44	
5	175	15	665	25		35		45	
6	215	16	695	26		36		46	
7	255	17	730	27		37		47	
8	300	18	780	28		38		48	
9	440	19	835	29		39		49	
10	485	20	870	30		40		50	



DCP number (mm / Blow) DN									
DN	mm	DN	mm	DN	mm	DN	mm	DN	mm
7	35	12	545	6	900				
5	60	8	585						
8	100	5	610						
7	135	4	630						
8	175	7	665						
8	215	6	695						
8	255	7	730						
9	300	10	780						
28	440	11	835						
9	485	7	870						



Depth (mm)				In situ		Blows/mm	
no.	From	-	To	DN	CBR	150mm	300mm
1	1	-	150	6,9	37	22	20,0
2	151	-	300	8,3	29	18	
3	301	-	450	24,5	7	6	11,5
4	451	-	600	8,9	27	17	
5	601	-	750	6,3	42	24	
6	751	-	900				
8	901	-	1050				
9	1051	-	1200				
10	1201	-	1350				
11	1351	-	1500				
12	1501	-	1650				
13	1651	-	1800				
14	1801	-	1950				
15	1951	-	2100				



REMARKS: Max penetration depth 900 mm

Where values are presented as continued "cont", refer to the adjacent graphs where DN values vs depth and penetration vs depths can be read.

Signature 

TEST REPORT FOR DYNAMIC CONE PENETROMETER

BY TEST METHOD TMH6-ST6

CLIENT: ESKOM

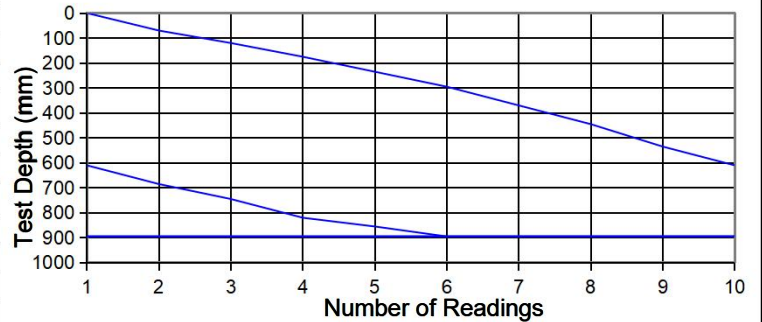
DATE REPORTED: 22/08/2023
 OUR REF: MAK1520823

PROJECT: Geotechnical Investigation: Komati Power Station
 ATTENTION: Eskom
 Tel/Email: 031 350 3370

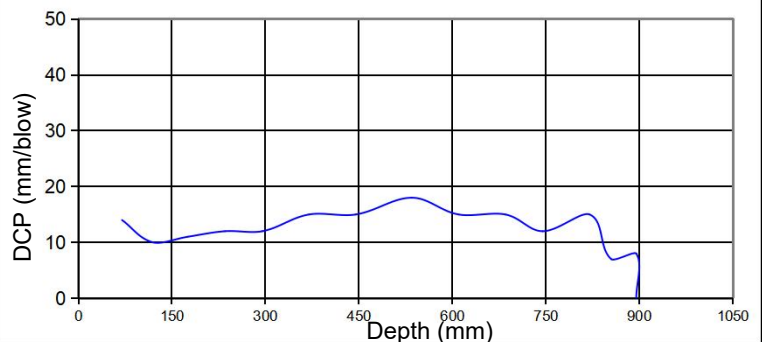
DESCRIPTION: DCP test was done from existing ground level
 DCP No: DCP11 at TP11

DATE TESTED: 22/08/2023

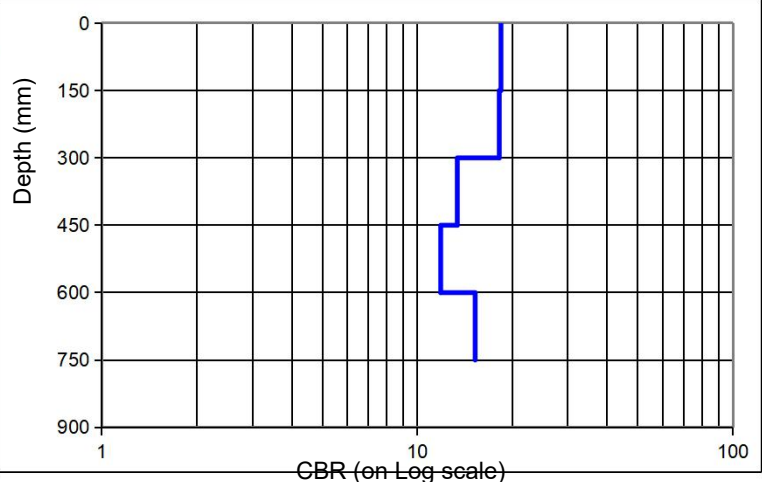
DCP Readings		Blows per reading: 5							
no.	mm	no.	mm	no.	mm	no.	mm	no.	mm
1	70	11	745	21		31		41	
2	120	12	820	22		32		42	
3	175	13	855	23		33		43	
4	235	14	895	24		34		44	
5	295	15		25		35		45	
6	370	16		26		36		46	
7	445	17		27		37		47	
8	535	18		28		38		48	
9	610	19		29		39		49	
10	685	20		30		40		50	



DCP number (mm / Blow) DN									
DN	mm	DN	mm	DN	mm	DN	mm	DN	mm
14	70	12	745						
10	120	15	820						
11	175	7	855						
12	235	8	895						
12	295								
15	370								
15	445								
18	535								
15	610								
15	685								



Depth (mm)				In situ		Blows/mm	
no.	From	-	To	DN	CBR	150mm	300mm
1	1	-	150	11,8	18	13	12,7
2	151	-	300	11,9	18	13	
3	301	-	450	15,1	13	10	9,5
4	451	-	600	16,6	12	9	
5	601	-	750	13,6	15	11	
6	751	-	900				
8	901	-	1050				
9	1051	-	1200				
10	1201	-	1350				
11	1351	-	1500				
12	1501	-	1650				
13	1651	-	1800				
14	1801	-	1950				
15	1951	-	2100				



REMARKS:

Max penetration depth **895** mm

Where values are presented as continued "cont", refer to the adjacent graphs where DN values vs depth and penetration vs depths can be read.

Signature

TEST REPORT FOR DYNAMIC CONE PENETROMETER

BY TEST METHOD TMH6-ST6

CLIENT: ESKOM

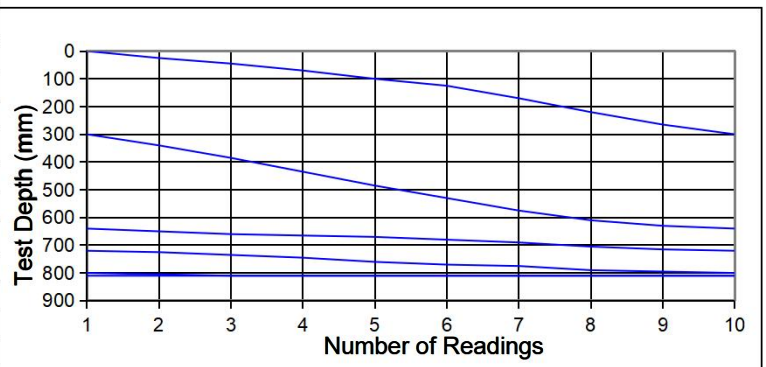
DATE REPORTED: 22/08/2023
 OUR REF: MAK1520823

PROJECT: Geotechnical Investigation: Komati Power Station
 ATTENTION: Eskom
 Tel/Email: 031 350 3370

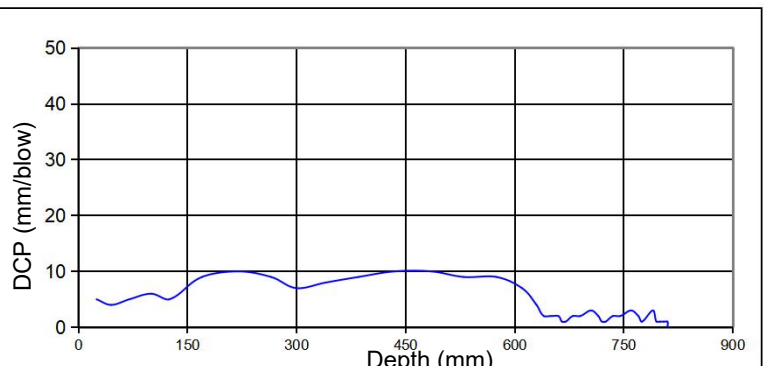
DESCRIPTION: DCP test was done from existing ground level
 DCP No: DCP12 at TP12

DATE TESTED: 22/08/2023

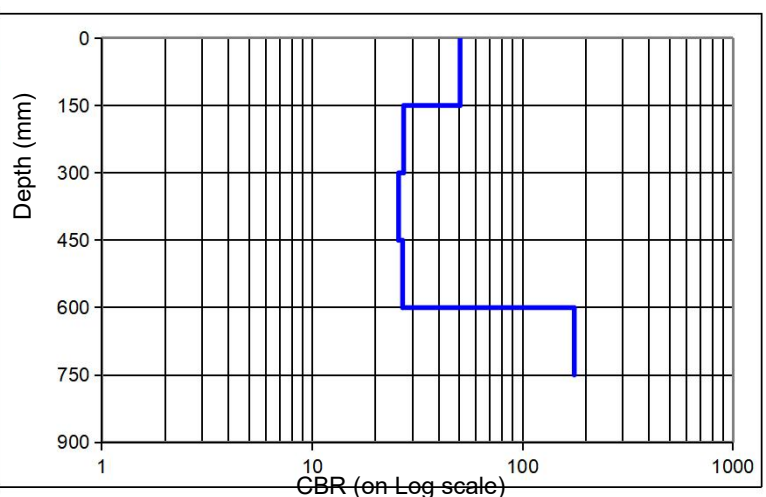
DCP Readings		Blows per reading: 5							
no.	mm	no.	mm	no.	mm	no.	mm	no.	mm
1	25	11	385	21	665	31	760	41	
2	45	12	435	22	670	32	770	42	
3	70	13	485	23	680	33	775	43	
4	100	14	530	24	690	34	790	44	
5	125	15	575	25	705	35	795	45	
6	170	16	610	26	715	36	800	46	
7	220	17	630	27	720	37	805	47	
8	265	18	640	28	725	38	810	48	
9	300	19	650	29	735	39		49	
10	340	20	660	30	745	40		50	



DCP number (mm / Blow) DN									
DN	mm	DN	mm	DN	mm	DN	mm	DN	mm
5	25	9	385	1	665	3	760		
4	45	10	435	1	670	2	770		
5	70	10	485	2	680	1	775		
6	100	9	530	2	690	3	790		
5	125	9	575	3	705	1	795		
9	170	7	610	2	715	1	800		
10	220	4	630	1	720	1	805		
9	265	2	640	1	725	1	810		
7	300	2	650	2	735				
8	340	2	660	2	745				



Depth (mm)				In situ		Blows/mm	
no.	From	-	To	DN	CBR	150mm	300mm
1	1	-	150	5,4	50	28	22,5
2	151	-	300	8,7	27	17	
3	301	-	450	9,1	26	17	
4	451	-	600	8,8	27	17	
5	601	-	750	2,1	176	73	16,8
6	751	-	900				
8	901	-	1050				
9	1051	-	1200				
10	1201	-	1350				
11	1351	-	1500				
12	1501	-	1650				
13	1651	-	1800				
14	1801	-	1950				
15	1951	-	2100				



REMARKS:

Max penetration depth **810 mm**

Where values are presented as continued "cont", refer to the adjacent graphs where DN values vs depth and penetration vs depths can be read.

Signature 

TEST REPORT FOR DYNAMIC CONE PENETROMETER

BY TEST METHOD TMH6-ST6

CLIENT: ESKOM

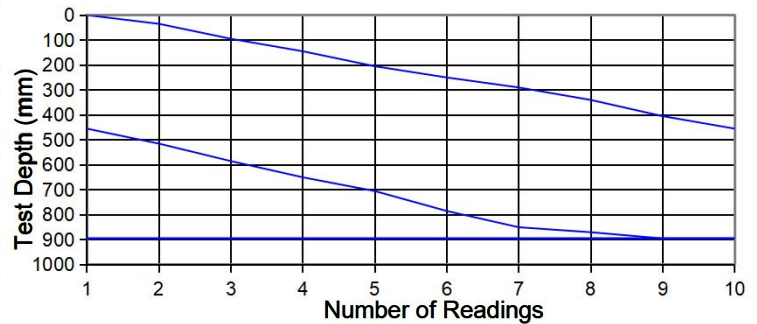
DATE REPORTED: 22/08/2023
 OUR REF: MAK1520823

PROJECT: Geotechnical Investigation: Komati Power Station
 ATTENTION: Eskom
 Tel/Email: 031 350 3370

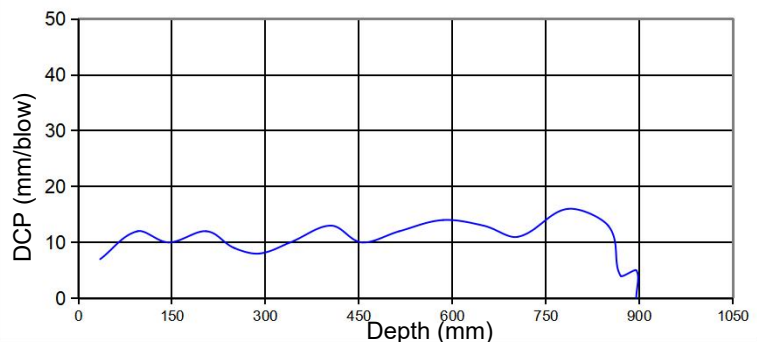
DESCRIPTION: DCP test was done from existing ground level
 DCP No: DCP13 at TP13

DATE TESTED: 22/08/2023

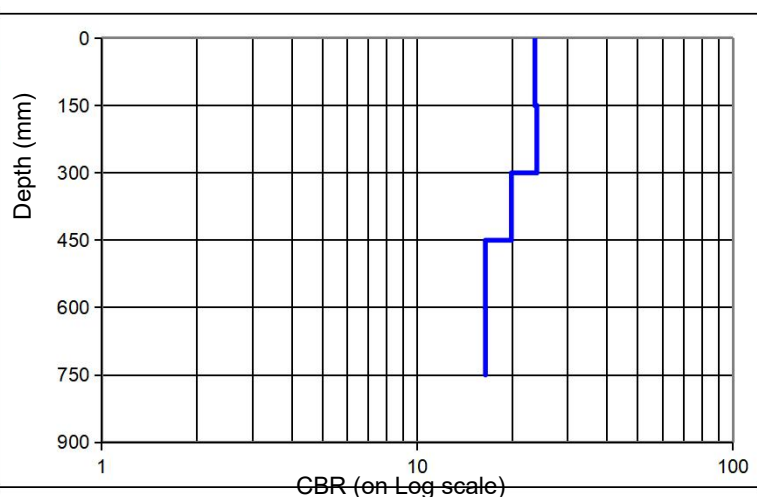
DCP Readings		Blows per reading: 5							
no.	mm	no.	mm	no.	mm	no.	mm	no.	mm
1	35	11	585	21		31		41	
2	95	12	650	22		32		42	
3	145	13	705	23		33		43	
4	205	14	785	24		34		44	
5	250	15	850	25		35		45	
6	290	16	870	26		36		46	
7	340	17	895	27		37		47	
8	405	18		28		38		48	
9	455	19		29		39		49	
10	515	20		30		40		50	



DCP number (mm / Blow) DN									
DN	mm	DN	mm	DN	mm	DN	mm	DN	mm
7	35	14	585						
12	95	13	650						
10	145	11	705						
12	205	16	785						
9	250	13	850						
8	290	4	870						
10	340	5	895						
13	405								
10	455								
12	515								



Depth (mm)				In situ		Blows/mm	
no.	From	-	To	DN	CBR	150mm	300mm
1	1	-	150	9,7	24	15	15,5
2	151	-	300	9,6	24	16	
3	301	-	450	11,1	20	14	12,6
4	451	-	600	12,9	16	12	
5	601	-	750	12,9	16	12	
6	751	-	900				
8	901	-	1050				
9	1051	-	1200				
10	1201	-	1350				
11	1351	-	1500				
12	1501	-	1650				
13	1651	-	1800				
14	1801	-	1950				
15	1951	-	2100				



REMARKS:

Max penetration depth **895 mm**

Where values are presented as continued "cont", refer to the adjacent graphs where DN values vs depth and penetration vs depths can be read.

Signature

TEST REPORT FOR DYNAMIC CONE PENETROMETER BY TEST METHOD TMH6-ST6

CLIENT: ESKOM

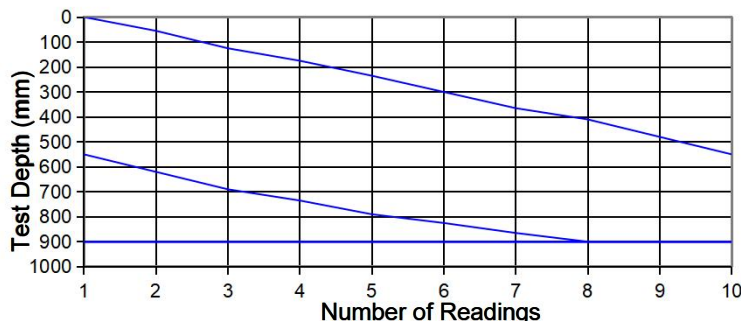
DATE REPORTED: 22/08/2023
 OUR REF: MAK1520823

PROJECT: Geotechnical Investigation: Komati Power Station
 ATTENTION: Eskom
 Tel/Email: 031 350 3370

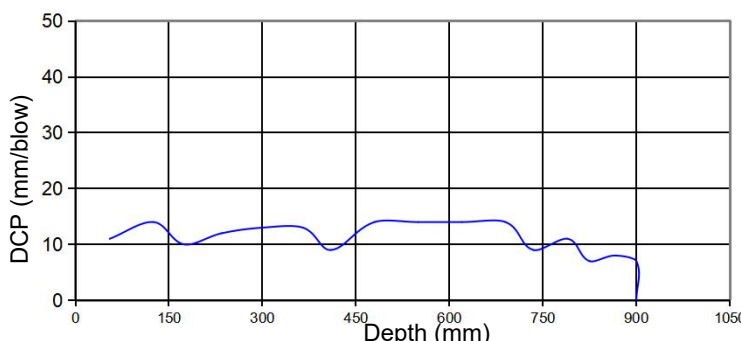
DESCRIPTION: DCP test was done from existing ground level
 DCP No: DCP14 at TP14

DATE TESTED: 22/08/2023

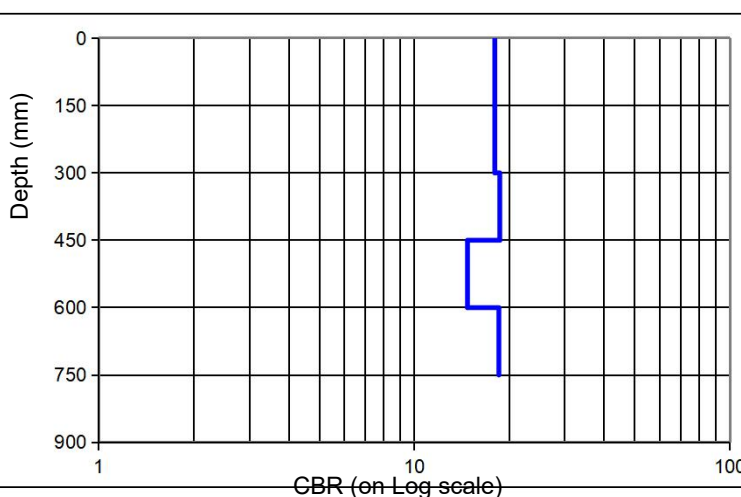
DCP Readings		Blows per reading: 5							
no.	mm	no.	mm	no.	mm	no.	mm	no.	mm
1	55	11	690	21		31		41	
2	125	12	735	22		32		42	
3	175	13	790	23		33		43	
4	235	14	825	24		34		44	
5	300	15	865	25		35		45	
6	365	16	900	26		36		46	
7	410	17		27		37		47	
8	480	18		28		38		48	
9	550	19		29		39		49	
10	620	20		30		40		50	



DCP number (mm / Blow) DN									
DN	mm	DN	mm	DN	mm	DN	mm	DN	mm
11	55	14	690						
14	125	9	735						
10	175	11	790						
12	235	7	825						
13	300	8	865						
13	365	7	900						
9	410								
14	480								
14	550								
14	620								



Depth (mm)				In situ		Blows/mm	
no.	From	-	To	DN	CBR	150mm	300mm
1	1	-	150	12,0	18	13	12,5
2	151	-	300	12,0	18	13	
3	301	-	450	11,7	19	13	11,8
4	451	-	600	14,0	15	11	
5	601	-	750	11,7	19	13	
6	751	-	900				
8	901	-	1050				
9	1051	-	1200				
10	1201	-	1350				
11	1351	-	1500				
12	1501	-	1650				
13	1651	-	1800				
14	1801	-	1950				
15	1951	-	2100				



REMARKS: Max penetration depth 900 mm

Where values are presented as continued "cont", refer to the adjacent graphs where DN values vs depth and penetration vs depths can be read.

Signature 

TEST REPORT FOR DYNAMIC CONE PENETROMETER BY TEST METHOD TMH6-ST6

CLIENT: ESKOM

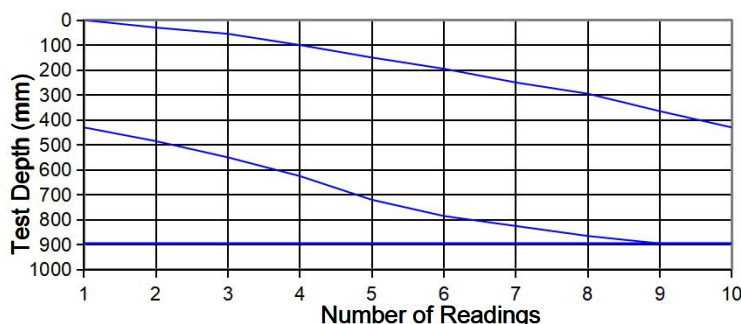
DATE REPORTED: 22/08/2023
 OUR REF: MAK1520823

PROJECT: Geotechnical Investigation: Komati Power Station
 ATTENTION: Eskom
 Tel/Email: 031 350 3370

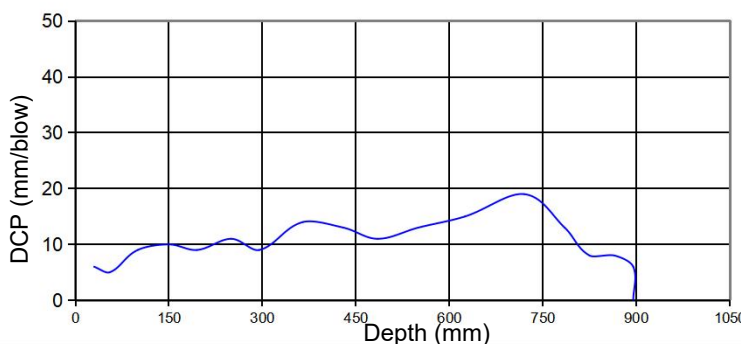
DESCRIPTION: DCP test was done from existing ground level
 DCP No: DCP15 at TP15

DATE TESTED: 22/08/2023

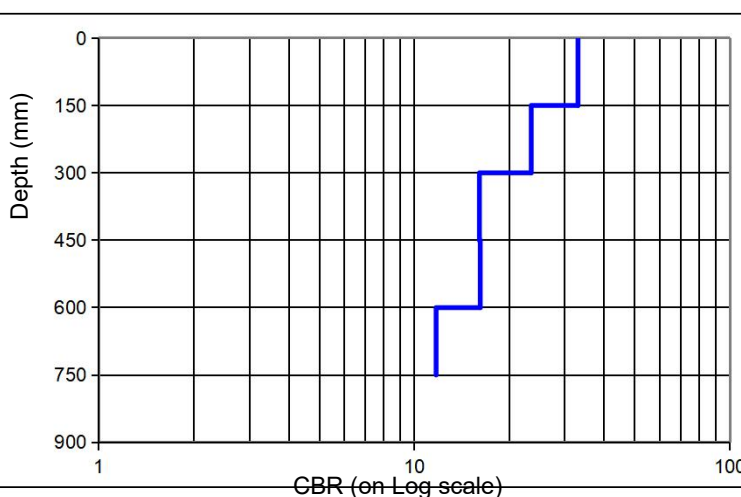
DCP Readings		Blows per reading: 5							
no.	mm	no.	mm	no.	mm	no.	mm	no.	mm
1	30	11	550	21		31		41	
2	55	12	625	22		32		42	
3	100	13	720	23		33		43	
4	150	14	785	24		34		44	
5	195	15	825	25		35		45	
6	250	16	865	26		36		46	
7	295	17	895	27		37		47	
8	365	18		28		38		48	
9	430	19		29		39		49	
10	485	20		30		40		50	



DCP number (mm / Blow) DN									
DN	mm	DN	mm	DN	mm	DN	mm	DN	mm
6	30	13	550						
5	55	15	625						
9	100	19	720						
10	150	13	785						
9	195	8	825						
11	250	8	865						
9	295	6	895						
14	365								
13	430								
11	485								



Depth (mm)				In situ		Blows/mm	
no.	From	-	To	DN	CBR	150mm	300mm
1	1	-	150	7,5	33	20	17,7
2	151	-	300	9,8	23	15	
3	301	-	450	13,1	16	11	11,5
4	451	-	600	13,0	16	12	
5	601	-	750	16,7	12	9	
6	751	-	900				
8	901	-	1050				
9	1051	-	1200				
10	1201	-	1350				
11	1351	-	1500				
12	1501	-	1650				
13	1651	-	1800				
14	1801	-	1950				
15	1951	-	2100				



REMARKS:

Max penetration depth **895 mm**

Where values are presented as continued "cont", refer to the adjacent graphs where DN values vs depth and penetration vs depths can be read.

Signature 

TEST REPORT FOR DYNAMIC CONE PENETROMETER

BY TEST METHOD TMH6-ST6

CLIENT: ESKOM

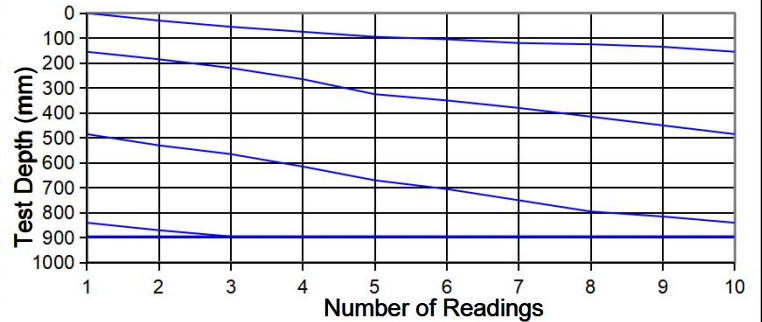
DATE REPORTED: 22/08/2023
 OUR REF: MAK1520823

PROJECT: Geotechnical Investigation: Komati Power Station
 ATTENTION: Eskom
 Tel/Email: 031 350 3370

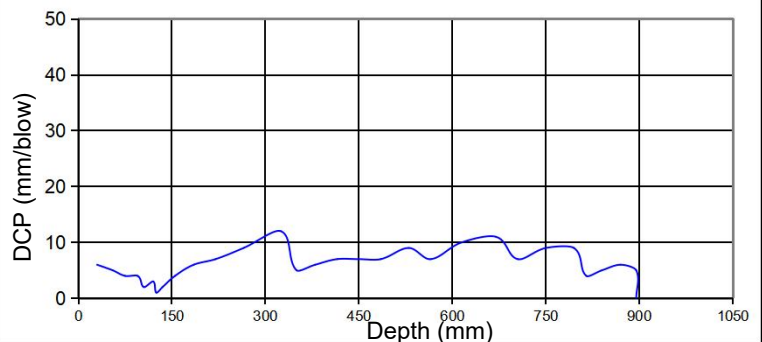
DESCRIPTION: DCP test was done from existing ground level
 DCP No: DCP16 at TP16

DATE TESTED: 22/08/2023

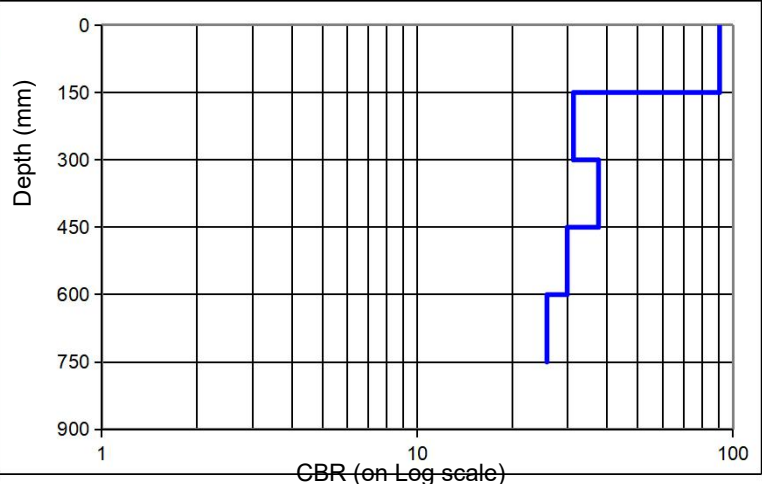
DCP Readings		Blows per reading: 5							
no.	mm	no.	mm	no.	mm	no.	mm	no.	mm
1	30	11	220	21	615	31		41	
2	55	12	265	22	670	32		42	
3	75	13	325	23	705	33		43	
4	95	14	350	24	750	34		44	
5	105	15	380	25	795	35		45	
6	120	16	415	26	815	36		46	
7	125	17	450	27	840	37		47	
8	135	18	485	28	870	38		48	
9	155	19	530	29	895	39		49	
10	185	20	565	30		40		50	



DCP number (mm / Blow) DN									
DN	mm	DN	mm	DN	mm	DN	mm	DN	mm
6	30	7	220	10	615				
5	55	9	265	11	670				
4	75	12	325	7	705				
4	95	5	350	9	750				
2	105	6	380	9	795				
3	120	7	415	4	815				
1	125	7	450	5	840				
2	135	7	485	6	870				
4	155	9	530	5	895				
6	185	7	565						



Depth (mm)				In situ		Blows/mm	
no.	From	-	To	DN	CBR	150mm	300mm
1	1	-	150	3,4	91	44	31,5
2	151	-	300	7,8	31	19	
3	301	-	450	6,8	38	22	20,3
4	451	-	600	8,1	30	19	
5	601	-	750	9,1	26	17	
6	751	-	900				
8	901	-	1050				
9	1051	-	1200				
10	1201	-	1350				
11	1351	-	1500				
12	1501	-	1650				
13	1651	-	1800				
14	1801	-	1950				
15	1951	-	2100				



REMARKS:

Max penetration depth **895 mm**

Where values are presented as continued "cont", refer to the adjacent graphs where DN values vs depth and penetration vs depths can be read.

Signature

TEST REPORT FOR DYNAMIC CONE PENETROMETER

BY TEST METHOD TMH6-ST6

CLIENT: ESKOM

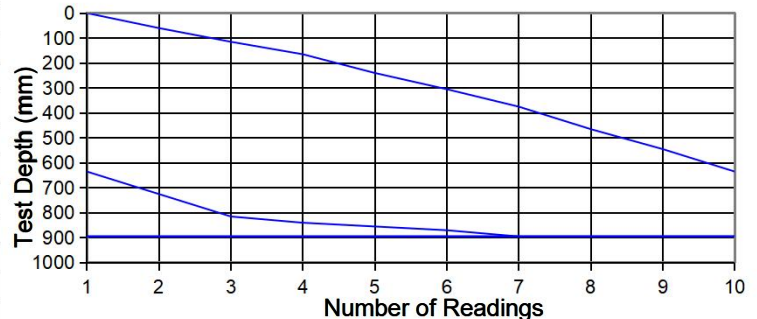
DATE REPORTED: 22/08/2023
 OUR REF: MAK1520823

PROJECT: Geotechnical Investigation: Komati Power Station
 ATTENTION: Eskom
 Tel/Email: 031 350 3370

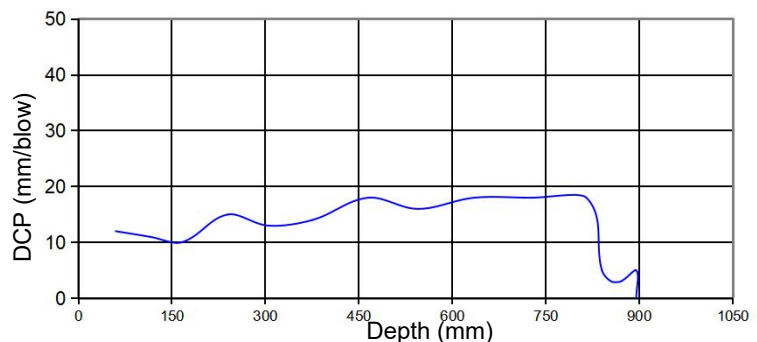
DESCRIPTION: DCP test was done from existing ground level
 DCP No: DCP17 at TP17

DATE TESTED: 22/08/2023

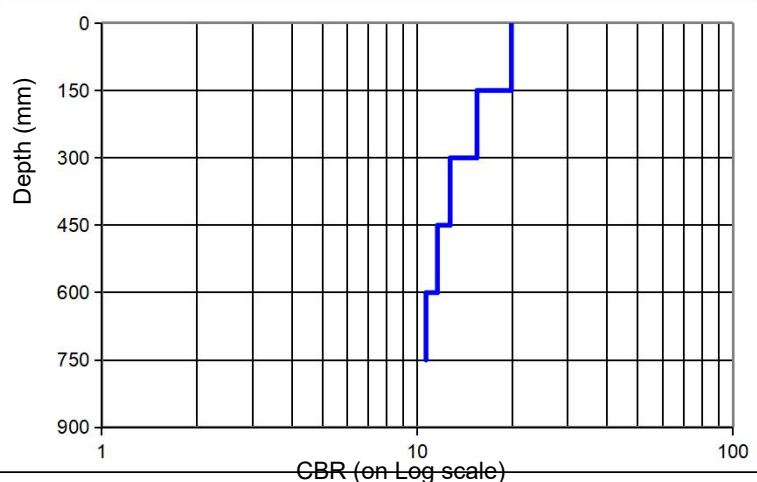
DCP Readings		Blows per reading: 5							
no.	mm	no.	mm	no.	mm	no.	mm	no.	mm
1	60	11	815	21		31		41	
2	115	12	840	22		32		42	
3	165	13	855	23		33		43	
4	240	14	870	24		34		44	
5	305	15	895	25		35		45	
6	375	16		26		36		46	
7	465	17		27		37		47	
8	545	18		28		38		48	
9	635	19		29		39		49	
10	725	20		30		40		50	



DCP number (mm / Blow) DN									
DN	mm	DN	mm	DN	mm	DN	mm	DN	mm
12	60	18	815						
11	115	5	840						
10	165	3	855						
15	240	3	870						
13	305	5	895						
14	375								
18	465								
16	545								
18	635								
18	725								



Depth (mm)				In situ		Blows/mm	
no.	From	-	To	DN	CBR	150mm	300mm
1	1	-	150	11,1	20	14	12,3
2	151	-	300	13,5	15	11	
3	301	-	450	15,7	13	10	9,2
4	451	-	600	16,9	12	9	
5	601	-	750	18,0	11	8	
6	751	-	900				
8	901	-	1050				
9	1051	-	1200				
10	1201	-	1350				
11	1351	-	1500				
12	1501	-	1650				
13	1651	-	1800				
14	1801	-	1950				
15	1951	-	2100				



REMARKS:

Max penetration depth **895 mm**

Where values are presented as continued "cont", refer to the adjacent graphs where DN values vs depth and penetration vs depths can be read.

Signature

TEST REPORT FOR DYNAMIC CONE PENETROMETER

BY TEST METHOD TMH6-ST6

CLIENT: ESKOM

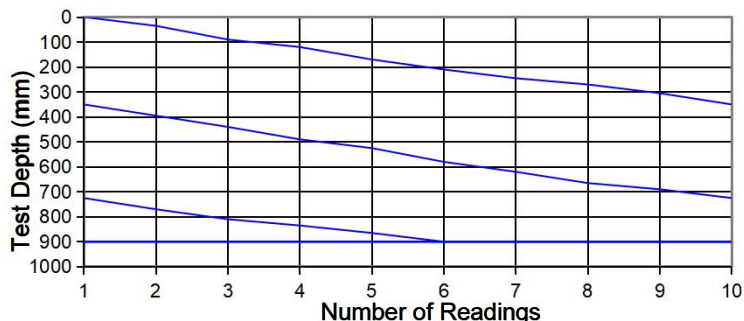
DATE REPORTED: 22/08/2023
 OUR REF: MAK1520823

PROJECT: Geotechnical Investigation: Komati Power Station
 ATTENTION: Eskom
 Tel/Email: 031 350 3370

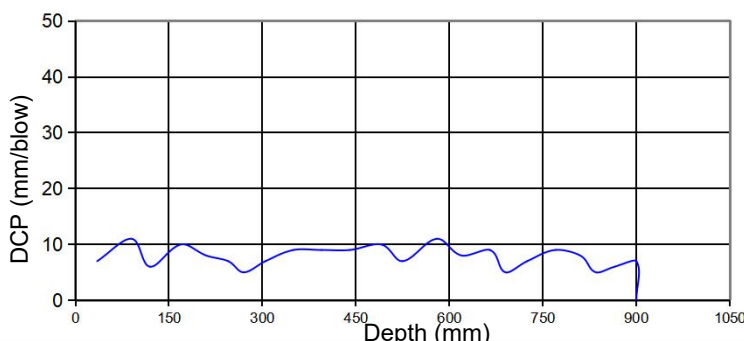
DESCRIPTION: DCP test was done from existing ground level
 DCP No: DCP1 at TP1

DATE TESTED: 22/08/2023

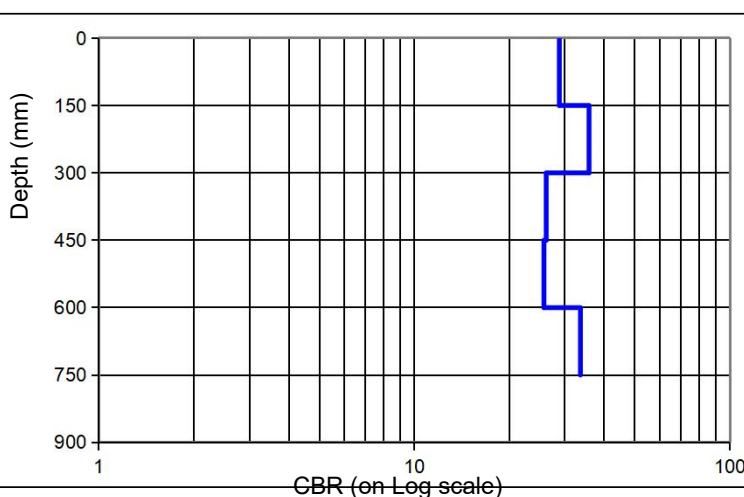
DCP Readings		Blows per reading: 5							
no.	mm	no.	mm	no.	mm	no.	mm	no.	mm
1	35	11	440	21	835	31		41	
2	90	12	490	22	865	32		42	
3	120	13	525	23	900	33		43	
4	170	14	580	24		34		44	
5	210	15	620	25		35		45	
6	245	16	665	26		36		46	
7	270	17	690	27		37		47	
8	305	18	725	28		38		48	
9	350	19	770	29		39		49	
10	395	20	810	30		40		50	



DCP number (mm / Blow) DN									
DN	mm	DN	mm	DN	mm	DN	mm	DN	mm
7	35	9	440	5	835				
11	90	10	490	6	865				
6	120	7	525	7	900				
10	170	11	580						
8	210	8	620						
7	245	9	665						
5	270	5	690						
7	305	7	725						
9	350	9	770						
9	395	8	810						



Depth (mm)				In situ		Blows/mm	
no.	From	-	To	DN	CBR	150mm	300mm
1	1	-	150	8,3	29	18	19,6
2	151	-	300	7,0	36	21	
3	301	-	450	9,0	26	17	16,6
4	451	-	600	9,1	26	17	
5	601	-	750	7,4	34	20	
6	751	-	900				
8	901	-	1050				
9	1051	-	1200				
10	1201	-	1350				
11	1351	-	1500				
12	1501	-	1650				
13	1651	-	1800				
14	1801	-	1950				
15	1951	-	2100				



REMARKS: Max penetration depth 900 mm

Where values are presented as continued "cont", refer to the adjacent graphs where DN values vs depth and penetration vs depths can be read.

Signature 

TEST REPORT FOR DYNAMIC CONE PENETROMETER

BY TEST METHOD TMH6-ST6

CLIENT: ESKOM

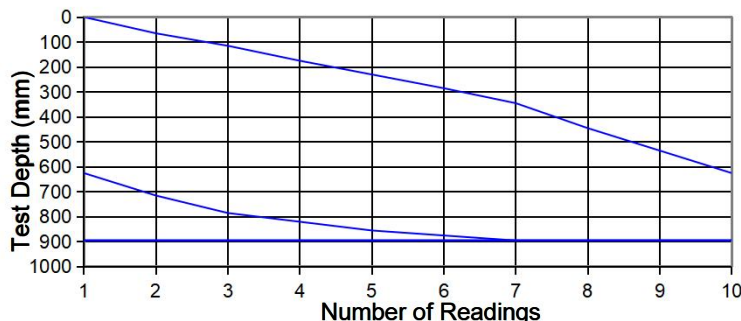
DATE REPORTED: 22/08/2023
 OUR REF: MAK1520823

PROJECT: Geotechnical Investigation: Komati Power Station
 ATTENTION: Eskom
 Tel/Email: 031 350 3370

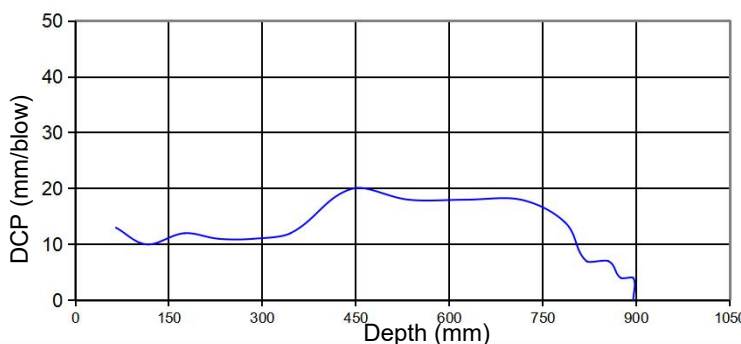
DESCRIPTION: DCP test was done from existing ground level
 DCP No: DCP19 at TP19

DATE TESTED: 22/08/2023

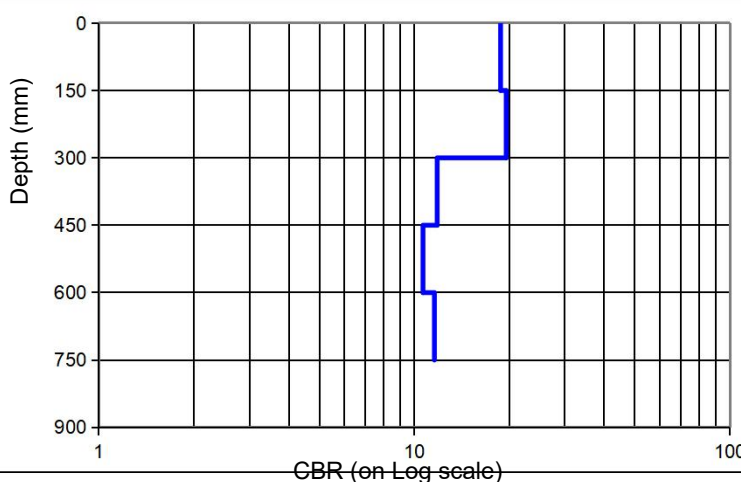
DCP Readings		Blows per reading: 5							
no.	mm	no.	mm	no.	mm	no.	mm	no.	mm
1	65	11	785	21		31		41	
2	115	12	820	22		32		42	
3	175	13	855	23		33		43	
4	230	14	875	24		34		44	
5	285	15	895	25		35		45	
6	345	16		26		36		46	
7	445	17		27		37		47	
8	535	18		28		38		48	
9	625	19		29		39		49	
10	715	20		30		40		50	



DCP number (mm / Blow) DN									
DN	mm	DN	mm	DN	mm	DN	mm	DN	mm
13	65	14	785						
10	115	7	820						
12	175	7	855						
11	230	4	875						
11	285	4	895						
12	345								
20	445								
18	535								
18	625								
18	715								



Depth (mm)				In situ		Blows/mm	
no.	From	-	To	DN	CBR	150mm	300mm
1	1	-	150	11,6	19	13	13,1
2	151	-	300	11,3	20	13	
3	301	-	450	16,6	12	9	
4	451	-	600	18,0	11	8	
5	601	-	750	16,9	12	9	8,7
6	751	-	900				
8	901	-	1050				
9	1051	-	1200				
10	1201	-	1350				
11	1351	-	1500				
12	1501	-	1650				
13	1651	-	1800				
14	1801	-	1950				
15	1951	-	2100				



REMARKS:

Max penetration depth **895 mm**

Where values are presented as continued "cont", refer to the adjacent graphs where DN values vs depth and penetration vs depths can be read.

Signature

TEST REPORT FOR DYNAMIC CONE PENETROMETER

BY TEST METHOD TMH6-ST6

CLIENT: ESKOM

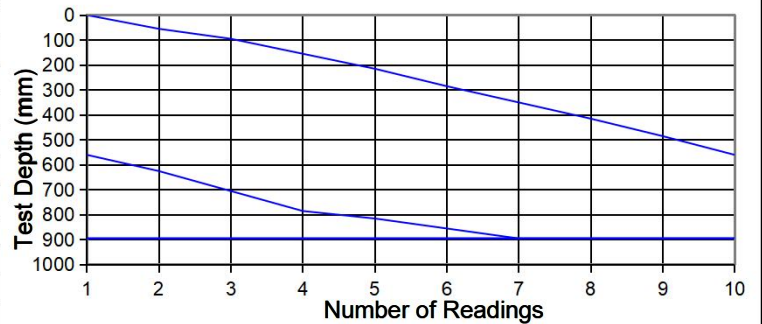
DATE REPORTED: 22/08/2023
 OUR REF: MAK1520823

PROJECT: Geotechnical Investigation: Komati Power Station
 ATTENTION: Eskom
 Tel/Email: 031 350 3370

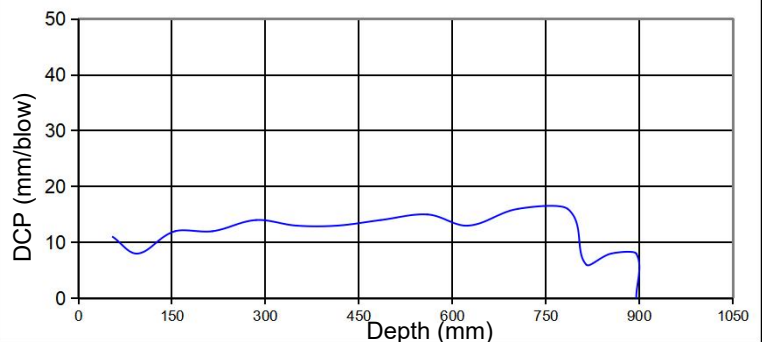
DESCRIPTION: DCP test was done from existing ground level
 DCP No: DCP20 at TP20

DATE TESTED: 22/08/2023

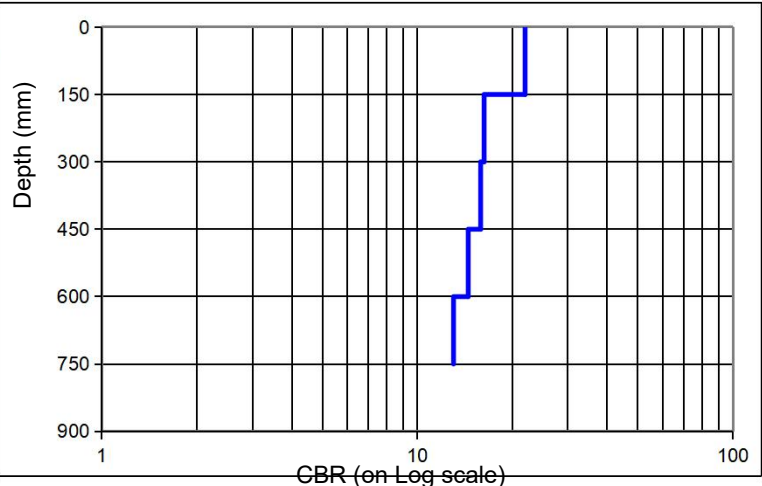
DCP Readings		Blows per reading: 5							
no.	mm	no.	mm	no.	mm	no.	mm	no.	mm
1	55	11	705	21		31		41	
2	95	12	785	22		32		42	
3	155	13	815	23		33		43	
4	215	14	855	24		34		44	
5	285	15	895	25		35		45	
6	350	16		26		36		46	
7	415	17		27		37		47	
8	485	18		28		38		48	
9	560	19		29		39		49	
10	625	20		30		40		50	



DCP number (mm / Blow) DN									
DN	mm	DN	mm	DN	mm	DN	mm	DN	mm
11	55	16	705						
8	95	16	785						
12	155	6	815						
12	215	8	855						
14	285	8	895						
13	350								
13	415								
14	485								
15	560								
13	625								



Depth (mm)				In situ		Blows/mm	
no.	From	-	To	DN	CBR	150mm	300mm
1	1	-	150	10,3	22	15	13,1
2	151	-	300	13,0	16	12	
3	301	-	450	13,2	16	11	11,0
4	451	-	600	14,2	14	11	
5	601	-	750	15,4	13	10	
6	751	-	900				
8	901	-	1050				
9	1051	-	1200				
10	1201	-	1350				
11	1351	-	1500				
12	1501	-	1650				
13	1651	-	1800				
14	1801	-	1950				
15	1951	-	2100				



REMARKS:

Max penetration depth **895 mm**

Where values are presented as continued "cont", refer to the adjacent graphs where DN values vs depth and penetration vs depths can be read.

Signature

TEST REPORT FOR DYNAMIC CONE PENETROMETER

BY TEST METHOD TMH6-ST6

CLIENT: ESKOM

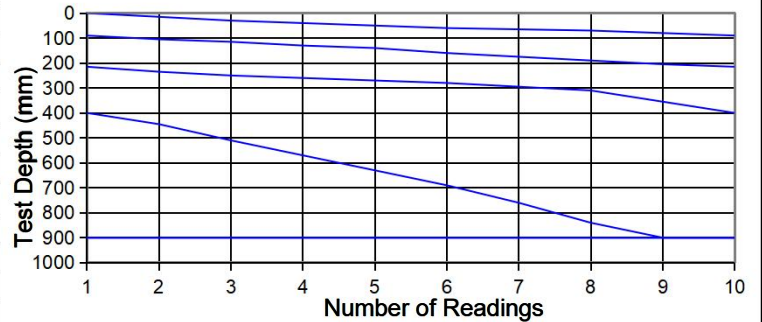
DATE REPORTED: 22/08/2023
 OUR REF: MAK1520823

PROJECT: Geotechnical Investigation: Komati Power Station
 ATTENTION: Eskom
 Tel/Email: 031 350 3370

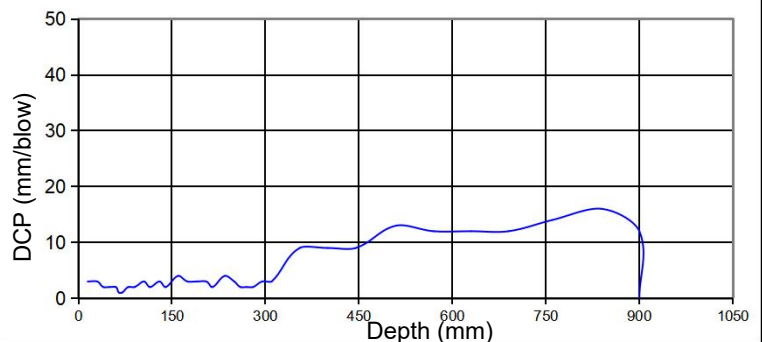
DESCRIPTION: DCP test was done from existing ground level
 DCP No: DCP41 at TP41

DATE TESTED: 22/08/2023

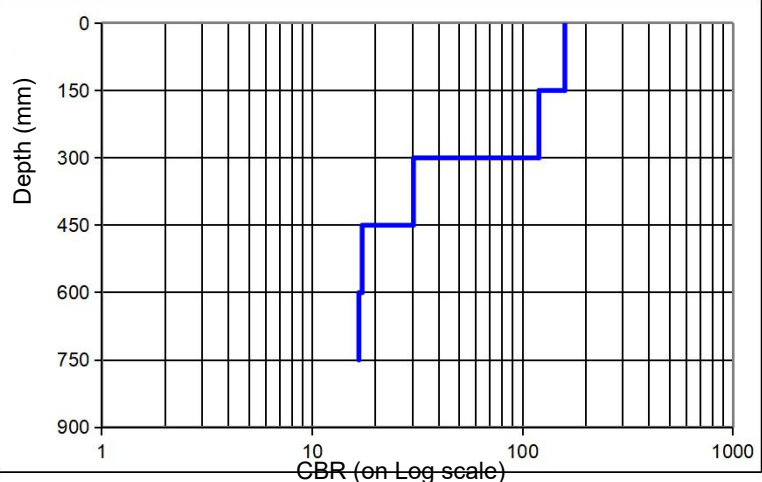
DCP Readings		Blows per reading: 5							
no.	mm	no.	mm	no.	mm	no.	mm	no.	mm
1	15	11	115	21	260	31	630	41	
2	30	12	130	22	270	32	690	42	
3	40	13	140	23	280	33	760	43	
4	50	14	160	24	295	34	840	44	
5	60	15	175	25	310	35	900	45	
6	65	16	190	26	355	36		46	
7	70	17	205	27	400	37		47	
8	80	18	215	28	445	38		48	
9	90	19	235	29	510	39		49	
10	105	20	250	30	570	40		50	



DCP number (mm / Blow) DN									
DN	mm	DN	mm	DN	mm	DN	mm	DN	mm
3	15	2	115	2	260	12	630		
3	30	3	130	2	270	12	690		
2	40	2	140	2	280	14	760		
2	50	4	160	3	295	16	840		
2	60	3	175	3	310	12	900		
1	65	3	190	9	355				
1	70	3	205	9	400				
2	80	2	215	9	445				
2	90	4	235	13	510				
3	105	3	250	12	570				



Depth (mm)				In situ		Blows/mm	
no.	From	-	To	DN	CBR	150mm	300mm
1	1	-	150	2,2	159	68	60,8
2	151	-	300	2,8	120	54	
3	301	-	450	8,0	30	19	15,4
4	451	-	600	12,4	17	12	
5	601	-	750	12,7	17	12	
6	751	-	900				
8	901	-	1050				
9	1051	-	1200				
10	1201	-	1350				
11	1351	-	1500				
12	1501	-	1650				
13	1651	-	1800				
14	1801	-	1950				
15	1951	-	2100				



REMARKS: Max penetration depth 900 mm

Where values are presented as continued "cont", refer to the adjacent graphs where DN values vs depth and penetration vs depths can be read.

Signature 

TEST REPORT FOR DYNAMIC CONE PENETROMETER

BY TEST METHOD TMH6-ST6

CLIENT: ESKOM

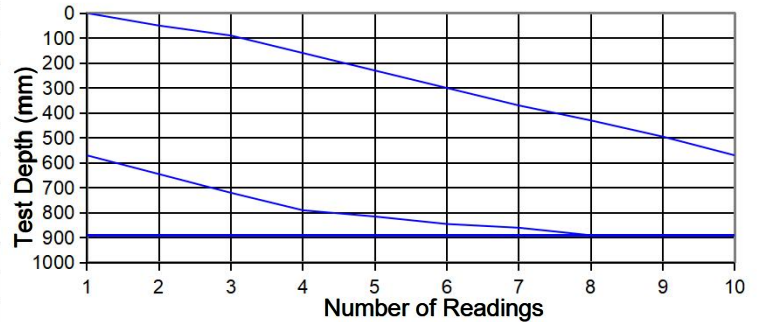
DATE REPORTED: 22/08/2023
 OUR REF: MAK1520823

PROJECT: Geotechnical Investigation: Komati Power Station
 ATTENTION: Eskom
 Tel/Email: 031 350 3370

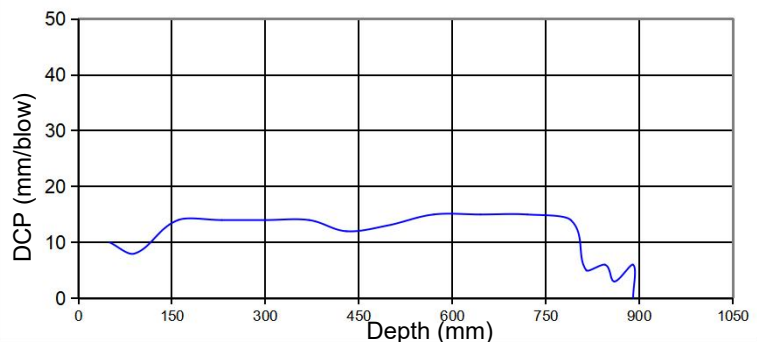
DESCRIPTION: DCP test was done from existing ground level
 DCP No: DCP42 at TP42

DATE TESTED: 22/08/2023

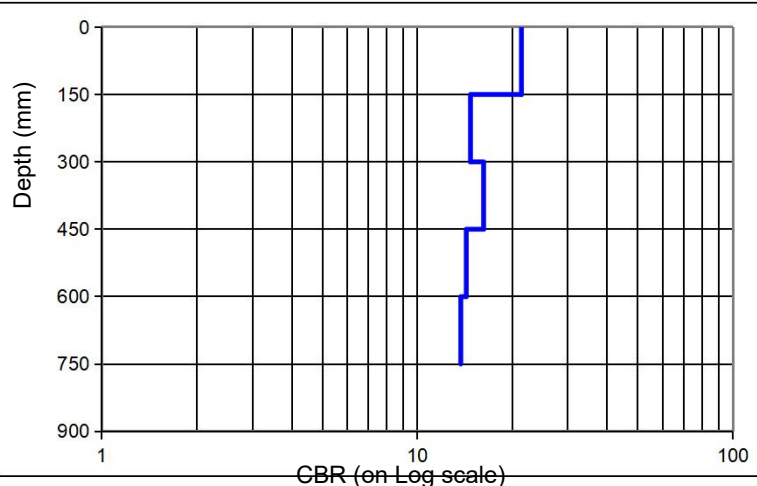
DCP Readings		Blows per reading: 5							
no.	mm	no.	mm	no.	mm	no.	mm	no.	mm
1	50	11	720	21		31		41	
2	90	12	790	22		32		42	
3	160	13	815	23		33		43	
4	230	14	845	24		34		44	
5	300	15	860	25		35		45	
6	370	16	890	26		36		46	
7	430	17		27		37		47	
8	495	18		28		38		48	
9	570	19		29		39		49	
10	645	20		30		40		50	



DCP number (mm / Blow) DN									
DN	mm	DN	mm	DN	mm	DN	mm	DN	mm
10	50	15	720						
8	90	14	790						
14	160	5	815						
14	230	6	845						
14	300	3	860						
14	370	6	890						
12	430								
13	495								
15	570								
15	645								



Depth (mm)				In situ		Blows/mm	
no.	From	-	To	DN	CBR	150mm	300mm
1	1	-	150	10,5	21	14	12,5
2	151	-	300	14,0	15	11	
3	301	-	450	13,0	16	12	11,0
4	451	-	600	14,3	14	10	
5	601	-	750	14,8	14	10	
6	751	-	900				
8	901	-	1050				
9	1051	-	1200				
10	1201	-	1350				
11	1351	-	1500				
12	1501	-	1650				
13	1651	-	1800				
14	1801	-	1950				
15	1951	-	2100				



REMARKS:

Max penetration depth **890 mm**

Where values are presented as continued "cont", refer to the adjacent graphs where DN values vs depth and penetration vs depths can be read.

Signature

TEST REPORT FOR DYNAMIC CONE PENETROMETER BY TEST METHOD TMH6-ST6

CLIENT: ESKOM

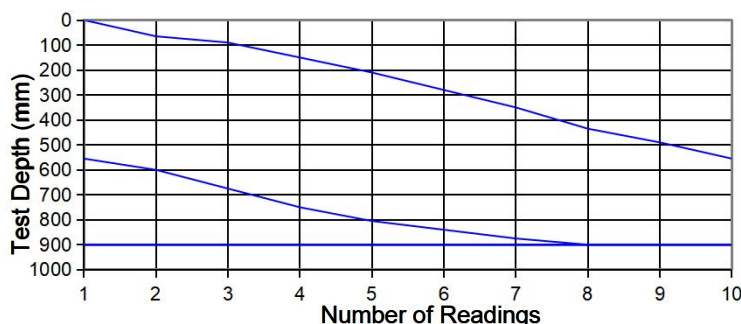
DATE REPORTED: 22/08/2023
 OUR REF: MAK1520823

PROJECT: Geotechnical Investigation: Komati Power Station
 ATTENTION: Eskom
 Tel/Email: 031 350 3370

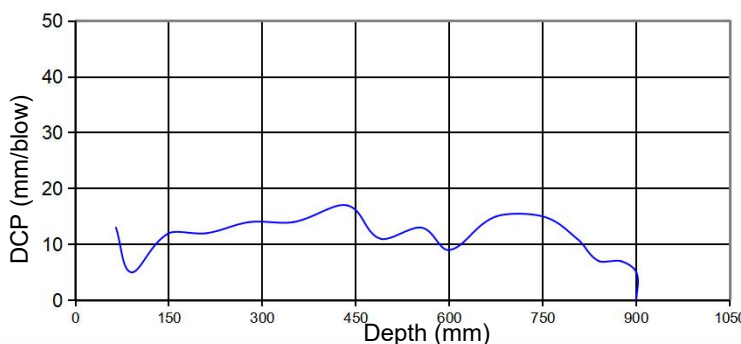
DESCRIPTION: DCP test was done from existing ground level
 DCP No: DCP43 at TP43

DATE TESTED: 22/08/2023

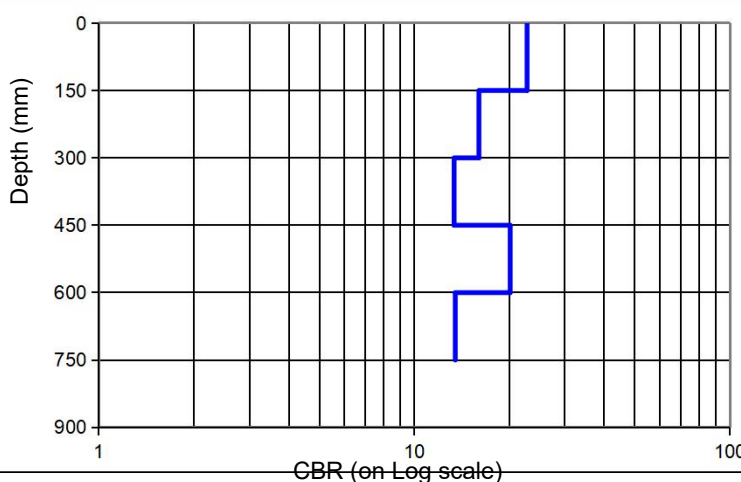
DCP Readings		Blows per reading: 5							
no.	mm	no.	mm	no.	mm	no.	mm	no.	mm
1	65	11	675	21		31		41	
2	90	12	750	22		32		42	
3	150	13	805	23		33		43	
4	210	14	840	24		34		44	
5	280	15	875	25		35		45	
6	350	16	900	26		36		46	
7	435	17		27		37		47	
8	490	18		28		38		48	
9	555	19		29		39		49	
10	600	20		30		40		50	



DCP number (mm / Blow) DN									
DN	mm	DN	mm	DN	mm	DN	mm	DN	mm
13	65	15	675						
5	90	15	750						
12	150	11	805						
12	210	7	840						
14	280	7	875						
14	350	5	900						
17	435								
11	490								
13	555								
9	600								



Depth (mm)				In situ		Blows/mm	
no.	From	-	To	DN	CBR	150mm	300mm
1	1	-	150	10,0	23	15	13,2
2	151	-	300	13,1	16	11	
3	301	-	450	15,1	13	10	11,8
4	451	-	600	11,0	20	14	
5	601	-	750	15,0	13	10	
6	751	-	900				
8	901	-	1050				
9	1051	-	1200				
10	1201	-	1350				
11	1351	-	1500				
12	1501	-	1650				
13	1651	-	1800				
14	1801	-	1950				
15	1951	-	2100				



REMARKS: Max penetration depth 900 mm

Where values are presented as continued "cont", refer to the adjacent graphs where DN values vs depth and penetration vs depths can be read.

Signature 

TEST REPORT FOR DYNAMIC CONE PENETROMETER

BY TEST METHOD TMH6-ST6

CLIENT: ESKOM

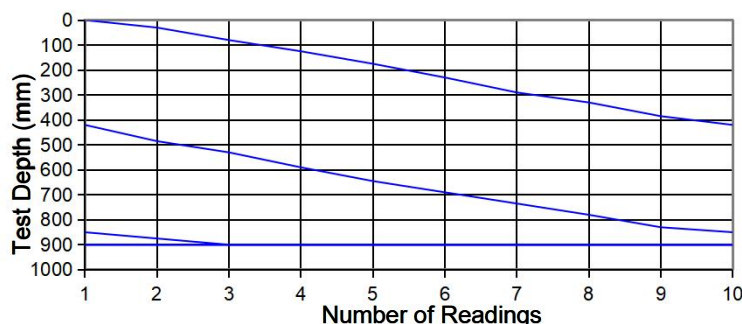
DATE REPORTED: 22/08/2023
 OUR REF: MAK1520823

PROJECT: Geotechnical Investigation: Komati Power Station
 ATTENTION: Eskom
 Tel/Email: 031 350 3370

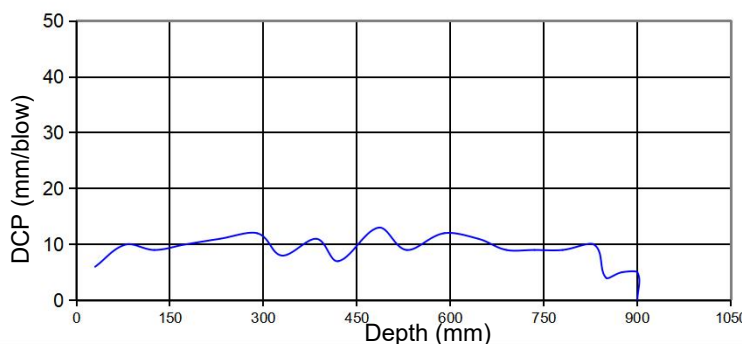
DESCRIPTION: DCP test was done from existing ground level
 DCP No: DCP44 at TP44

DATE TESTED: 22/08/2023

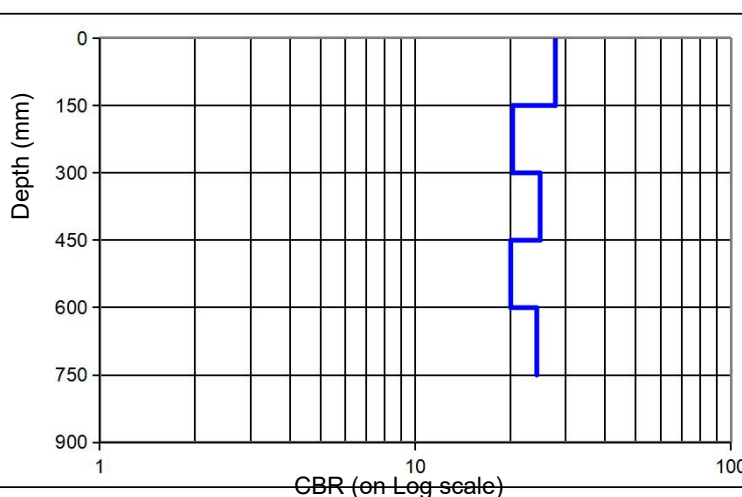
DCP Readings		Blows per reading: 5							
no.	mm	no.	mm	no.	mm	no.	mm	no.	mm
1	30	11	530	21		31		41	
2	80	12	590	22		32		42	
3	125	13	645	23		33		43	
4	175	14	690	24		34		44	
5	230	15	735	25		35		45	
6	290	16	780	26		36		46	
7	330	17	830	27		37		47	
8	385	18	850	28		38		48	
9	420	19	875	29		39		49	
10	485	20	900	30		40		50	



DCP number (mm / Blow) DN									
DN	mm	DN	mm	DN	mm	DN	mm	DN	mm
6	30	9	530						
10	80	12	590						
9	125	11	645						
10	175	9	690						
11	230	9	735						
12	290	9	780						
8	330	10	830						
11	385	4	850						
7	420	5	875						
13	485	5	900						



Depth (mm)				In situ		Blows/mm	
no.	From	-	To	DN	CBR	150mm	300mm
1	1	-	150	8,6	28	18	15,6
2	151	-	300	10,9	20	14	
3	301	-	450	9,3	25	16	14,8
4	451	-	600	11,0	20	14	
5	601	-	750	9,5	24	16	
6	751	-	900				
8	901	-	1050				
9	1051	-	1200				
10	1201	-	1350				
11	1351	-	1500				
12	1501	-	1650				
13	1651	-	1800				
14	1801	-	1950				
15	1951	-	2100				



REMARKS: Max penetration depth 900 mm

Where values are presented as continued "cont", refer to the adjacent graphs where DN values vs depth and penetration vs depths can be read.

Signature 

TEST REPORT FOR DYNAMIC CONE PENETROMETER

BY TEST METHOD TMH6-ST6

CLIENT: ESKOM

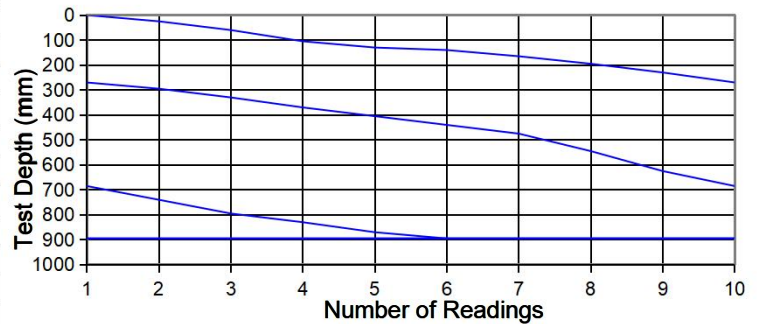
DATE REPORTED: 22/08/2023
 OUR REF: MAK1520823

PROJECT: Geotechnical Investigation: Komati Power Station
 ATTENTION: Eskom
 Tel/Email: 031 350 3370

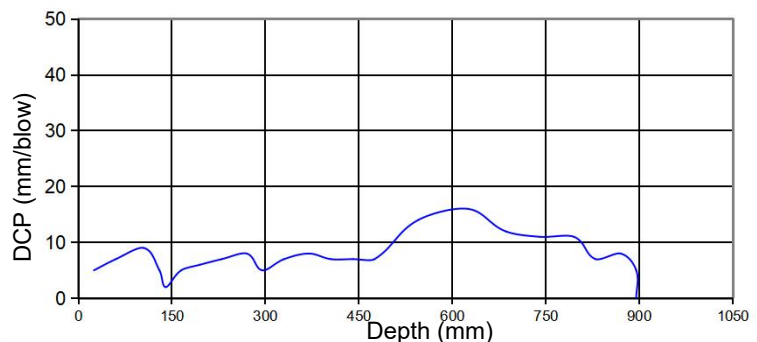
DESCRIPTION: DCP test was done from existing ground level
 DCP No: DCP45 at TP45

DATE TESTED: 22/08/2023

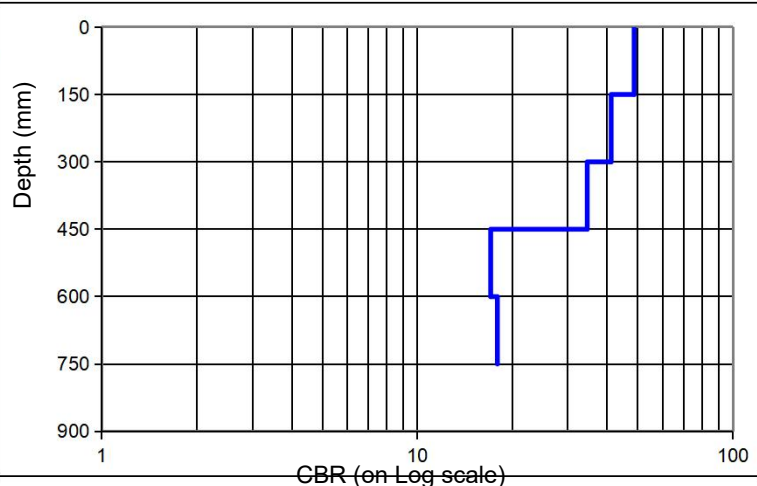
DCP Readings					Blows per reading: 5				
no.	mm	no.	mm	no.	mm	no.	mm	no.	mm
1	25	11	330	21	830	31		41	
2	60	12	370	22	870	32		42	
3	105	13	405	23	895	33		43	
4	130	14	440	24		34		44	
5	140	15	475	25		35		45	
6	165	16	545	26		36		46	
7	195	17	625	27		37		47	
8	230	18	685	28		38		48	
9	270	19	740	29		39		49	
10	295	20	795	30		40		50	



DCP number (mm / Blow) DN									
DN	mm	DN	mm	DN	mm	DN	mm	DN	mm
5	25	7	330	7	830				
7	60	8	370	8	870				
9	105	7	405	5	895				
5	130	7	440						
2	140	7	475						
5	165	14	545						
6	195	16	625						
7	230	12	685						
8	270	11	740						
5	295	11	795						



Depth (mm)				In situ		Blows/mm		
no.	From	-	To	DN	CBR	150mm	300mm	
1	1	-	150	5,6	49	27		25,4
2	151	-	300	6,3	41	24		
3	301	-	450	7,2	35	21		16,4
4	451	-	600	12,5	17	12		
5	601	-	750	12,0	18	12		
6	751	-	900					
8	901	-	1050					
9	1051	-	1200					
10	1201	-	1350					
11	1351	-	1500					
12	1501	-	1650					
13	1651	-	1800					
14	1801	-	1950					
15	1951	-	2100					



REMARKS:

Max penetration depth **895 mm**

Where values are presented as continued "cont", refer to the adjacent graphs where DN values vs depth and penetration vs depths can be read.

Signature

TEST REPORT FOR DYNAMIC CONE PENETROMETER

BY TEST METHOD TMH6-ST6

CLIENT: ESKOM

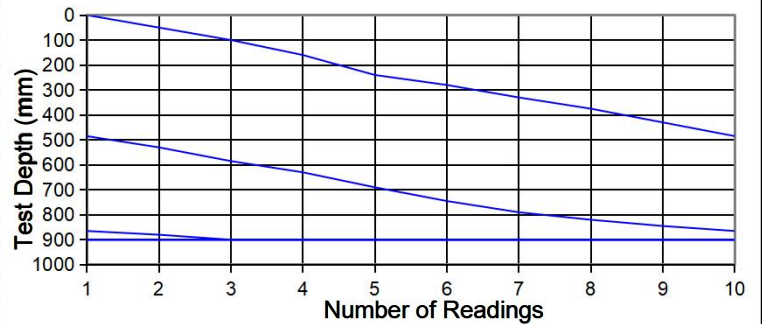
DATE REPORTED: 22/08/2023
 OUR REF: MAK1520823

PROJECT: Geotechnical Investigation: Komati Power Station
 ATTENTION: Eskom
 Tel/Email: 031 350 3370

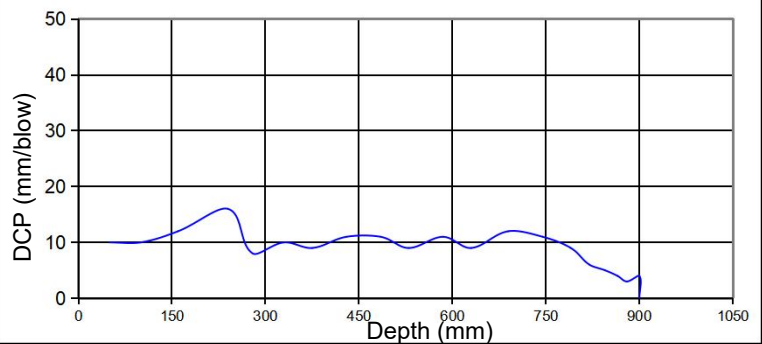
DESCRIPTION: DCP test was done from existing ground level
 DCP No: DCP46 at TP46

DATE TESTED: 22/08/2023

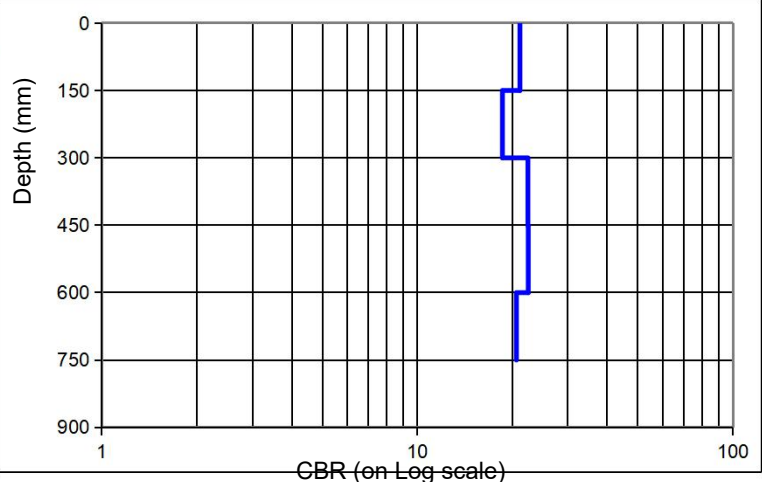
DCP Readings		Blows per reading: 5							
no.	mm	no.	mm	no.	mm	no.	mm	no.	mm
1	50	11	585	21		31		41	
2	100	12	630	22		32		42	
3	160	13	690	23		33		43	
4	240	14	745	24		34		44	
5	280	15	790	25		35		45	
6	330	16	820	26		36		46	
7	375	17	845	27		37		47	
8	430	18	865	28		38		48	
9	485	19	880	29		39		49	
10	530	20	900	30		40		50	



DCP number (mm / Blow) DN									
DN	mm	DN	mm	DN	mm	DN	mm	DN	mm
10	50	11	585						
10	100	9	630						
12	160	12	690						
16	240	11	745						
8	280	9	790						
10	330	6	820						
9	375	5	845						
11	430	4	865						
11	485	3	880						
9	530	4	900						



Depth (mm)				In situ		Blows/mm	
no.	From	-	To	DN	CBR	150mm	300mm
1	1	-	150	10,6	21	14	13,5
2	151	-	300	11,7	19	13	
3	301	-	450	10,1	22	15	14,8
4	451	-	600	10,1	22	15	
5	601	-	750	10,8	21	14	
6	751	-	900				
8	901	-	1050				
9	1051	-	1200				
10	1201	-	1350				
11	1351	-	1500				
12	1501	-	1650				
13	1651	-	1800				
14	1801	-	1950				
15	1951	-	2100				



REMARKS: Max penetration depth 900 mm

Where values are presented as continued "cont", refer to the adjacent graphs where DN values vs depth and penetration vs depths can be read.

Signature 

TEST REPORT FOR DYNAMIC CONE PENETROMETER

BY TEST METHOD TMH6-ST6

CLIENT: ESKOM

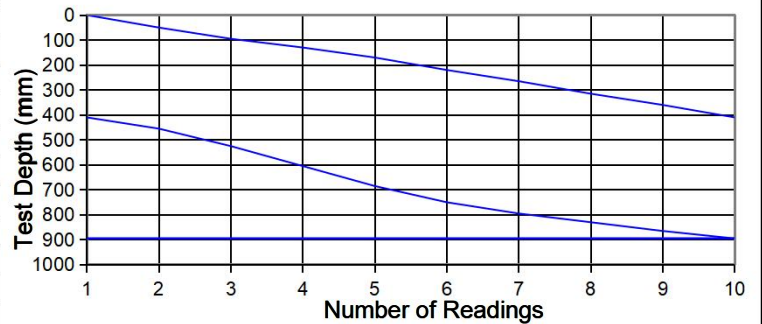
DATE REPORTED: 22/08/2023
 OUR REF: MAK1520823

PROJECT: Geotechnical Investigation: Komati Power Station
 ATTENTION: Eskom
 Tel/Email: 031 350 3370

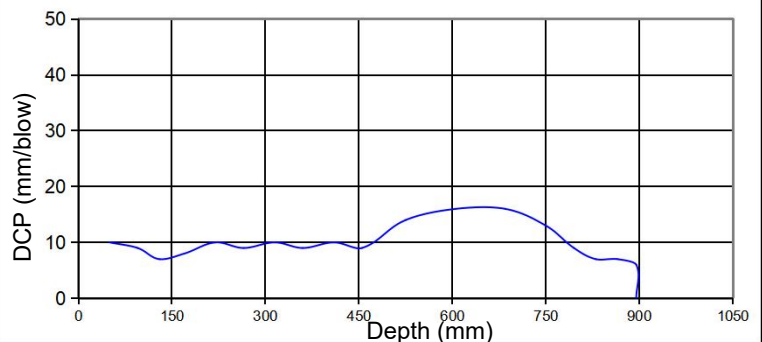
DESCRIPTION: DCP test was done from existing ground level
 DCP No: DCP47 at TP47

DATE TESTED: 22/08/2023

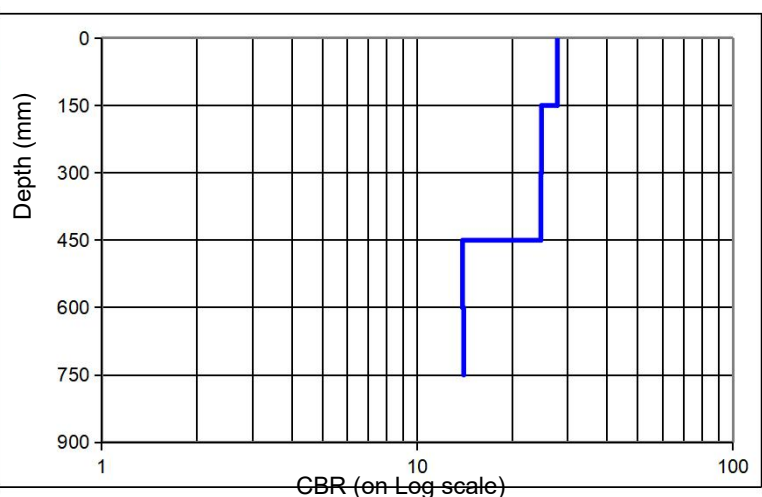
DCP Readings		Blows per reading: 5							
no.	mm	no.	mm	no.	mm	no.	mm	no.	mm
1	50	11	525	21		31		41	
2	95	12	605	22		32		42	
3	130	13	685	23		33		43	
4	170	14	750	24		34		44	
5	220	15	795	25		35		45	
6	265	16	830	26		36		46	
7	315	17	865	27		37		47	
8	360	18	895	28		38		48	
9	410	19		29		39		49	
10	455	20		30		40		50	



DCP number (mm / Blow) DN									
DN	mm	DN	mm	DN	mm	DN	mm	DN	mm
10	50	14	525						
9	95	16	605						
7	130	16	685						
8	170	13	750						
10	220	9	795						
9	265	7	830						
10	315	7	865						
9	360	6	895						
10	410								
9	455								



Depth (mm)				In situ		Blows/mm	
no.	From	-	To	DN	CBR	150mm	300mm
1	1	-	150	8,6	28	18	16,8
2	151	-	300	9,4	25	16	
3	301	-	450	9,4	25	16	13,1
4	451	-	600	14,6	14	10	
5	601	-	750	14,5	14	10	
6	751	-	900				
8	901	-	1050				
9	1051	-	1200				
10	1201	-	1350				
11	1351	-	1500				
12	1501	-	1650				
13	1651	-	1800				
14	1801	-	1950				
15	1951	-	2100				



REMARKS:

Max penetration depth **895 mm**

Where values are presented as continued "cont", refer to the adjacent graphs where DN values vs depth and penetration vs depths can be read.

Signature 

APPENDIX C: LABORATORY TEST RESULTS

(SANS 3001-GR1, GR3, GR10, GR20, GR30, GR40, PR5: 2013 – Civil Engineering test methods – Foundation Indicator Tests – SANS Accredited Soil Laboratory)

Client Name: BAV Consulting (Pty) Ltd
Project Name: Komati Power Station Repurposing
Job Number: PVT-291
Date: 26-Oct-23
Method: SANS 3001 GR1, GR3, GR10, GR12 GR20, GR30, GR31, GR40, GR50, GR53, GR54 & BS 1377 (where applicable)

SUMMARY OF TEST DATA	Sheet Reference: R-STL-038 Rev01
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Grading & Hydrometer Analysis (% Passing)								
Sample	KTP1	KTP2	KTP3	KTP4	KTP5	KTP7	KTP8	KTP12
Depth (m)	0.7 - 2.2	0.8 - 2.4	0.0 - 0.4	0.3 - 2.3	0.4 - 2.5	0.0 - 1.9	0.4 - 2.3	0.0 - 0.6
Lab No	PVT-291-2026	PVT-291-2027	PVT-291-2028	PVT-291-2029	PVT-291-2030	PVT-291-2032	PVT-291-2033	PVT-291-2035
53.0	100	100	100	100	100	100	100	100
37.5	100	100	100	100	100	100	100	100
26.5	100	100	100	100	100	100	100	100
19.0	100	100	100	100	100	100	100	100
13.2	99	100	100	100	99	99	100	100
9.5	96	99	100	99	97	85	100	98
6.7	91	95	100	96	95	67	99	96
4.75	87	93	100	93	91	58	98	93
2.0	83	89	100	89	84	48	96	91
1.0	80	85	99	86	81	45	93	89
0.425	70	77	88	78	73	39	84	75
0.250	57	68	65	66	64	34	75	60
0.150	42	55	41	53	51	26	60	40
0.075	26	39	26	38	36	17	47	23
0.060	21	33	21	34	31	14	40	19
0.050	18	30	19	32	29	13	37	17
0.035	14	25	15	29	25	11	31	14
0.020	12	23	11	26	23	9	28	12
0.006	10	20	7	23	20	7	24	9
0.002	9	17	4	21	19	5	20	6
GM	1.21	0.95	0.86	0.95	1.07	1.96	0.73	1.11

Atterberg Limits								
LL (%)	17	23	-	29	29	20	26	-
PI (%)	3	11	SP	16	15	5	13	SP
LS (%)	2.5	5.0	0.5	8.0	7.5	2.5	6.0	0.5

pH & Conductivity								
pH			6.2			6.1		
EC (S/m)			0.014			0.005		

MDD / OMC								
MDD (kg/m³)	2062	1951		1835	1872			
OMC (%)	9.4	11.9		15.6	13.9			

CBR								
100%	11.6	9.0		5.2	5.5			
98%	10.6	8.0		4.4	4.7			
97%	10.1	7.6		4.0	4.4			
95%	9.0	6.7		3.3	3.6			
93%	8.1	5.9		2.8	3.0			
90%	6.8	4.9		2.1	2.3			
Swell (%)	0.1	0.2		1.0	0.5			

UCS (MPa)								
100%								
97%								
90%								

COLTO Classification								
	G9	*		*	*			

Remarks: *= Not Classifiable

Client Name: BAV Consulting (Pty) Ltd
Project Name: Komati Power Station Repurposing
Job Number: PVT-291
Date: 26-Oct-23
Method: SANS 3001 GR1, GR3, GR10, GR12 GR20, GR30, GR31, GR40, GR50, GR53, GR54 & BS 1377 (where applicable)

SUMMARY OF TEST DATA	Sheet Reference: R-STL-038 Rev01
----------------------	-------------------------------------

Grading & Hydrometer Analysis (% Passing)								
Sample	KTP13	KTP14	KTP16	KTP19	KTP20	KTP21	KTP22	KTP23
Depth (m)	0.3 - 1.8	0.3 - 1.8	0.3 - 2.1	0.6 - 2.3	0.1 - 2.7	0.1 - 2.4	0.2 - 2.3	0.3 - 2.4
Lab No	PVT-291-2036	PVT-291-2037	PVT-291-2038	PVT-291-2040	PVT-291-2041	PVT-291-2042	PVT-291-2043	PVT-291-2044
53.0	100	100	100	100	100	100	100	100
37.5	100	100	100	100	100	100	100	100
26.5	100	100	100	100	100	100	100	100
19.0	100	100	100	100	100	100	100	100
13.2	100	100	100	99	100	100	100	100
9.5	100	100	100	97	100	98	100	98
6.7	98	100	96	88	99	92	99	93
4.75	94	99	89	80	98	84	98	88
2.0	88	97	63	71	96	75	96	81
1.0	85	94	52	68	96	72	94	80
0.425	76	86	42	60	92	65	88	72
0.250	66	78	37	51	83	60	77	64
0.150	51	62	31	41	64	44	52	49
0.075	41	48	24	30	43	31	35	33
0.060	38	44	21	26	38	27	30	28
0.050	36	41	19	24	35	25	27	25
0.035	33	37	17	21	30	23	22	21
0.020	30	32	15	18	26	20	19	17
0.006	26	26	12	13	20	16	14	13
0.002	24	22	9	10	17	13	10	11
GM	0.95	0.69	1.71	1.39	0.69	1.29	0.81	1.14

Atterberg Limits								
LL (%)	29	33	29	26	27	26	20	25
PI (%)	14	17	13	12	12	10	6	10
LS (%)	7.0	8.5	6.5	5.5	6.0	5.0	3.0	5.0

pH & Conductivity								
pH		6.4			6.1		7.0	
EC (S/m)		0.018			0.017		0.007	

MDD / OMC								
MDD (kg/m³)	1883		2071		1843	2095		1991
OMC (%)	13.9		9.5		13.1	10.7		11.7

CBR								
100%	8.2		22		5.8	24		10.5
98%	7.6		21		5.1	22		9.5
97%	7.3		20		4.8	21		9.0
95%	6.7		19		4.1	19		8.2
93%	5.9		16		3.5	16		7.5
90%	4.1		13		2.8	13		6.6
Swell (%)	0.3		0.3		0.6	0.0		0.2

UCS (MPa)								
100%								
97%								
90%								

COLTO Classification								
	*		G7		*	G7		G9

Remarks: *= Not Classifiable

Client Name: BAV Consulting (Pty) Ltd
Project Name: Komati Power Station Repurposing
Job Number: PVT-291
Date: 26-Oct-23
Method: SANS 3001 GR1, GR3, GR10, GR12 GR20, GR30, GR31, GR40, GR50, GR53, GR54 & BS 1377 (where applicable)

SUMMARY OF TEST DATA								Sheet Reference: R-STL-038 Rev01
Grading & Hydrometer Analysis (% Passing)								
Sample	KTP25	KTP26	KTP28	KTP29	KTP30	KTP31	KTP32	KTP33
Depth (m)	0.4 - 2.3	0.6 - 2.1	0.2 - 2.2	0.5 - 2.8	0.2 - 2.4	0.2 - 2.1	0.2 - 2.2	0.2 - 2.4
Lab No	PVT-291-2046	PVT-291-2047	PVT-291-2049	PVT-291-2050	PVT-291-2051	PVT-291-2052	PVT-291-2053	PVT-291-2054
53.0	100	100	100	100	100	100	100	100
37.5	100	100	100	100	100	100	100	100
26.5	100	100	100	100	100	100	100	100
19.0	100	100	100	100	100	100	100	100
13.2	100	99	100	100	100	100	100	100
9.5	99	95	98	100	100	99	99	100
6.7	99	87	95	100	99	95	98	98
4.75	97	79	91	99	97	91	96	94
2.0	91	69	86	96	93	87	91	88
1.0	87	66	84	94	90	86	89	86
0.425	73	60	80	86	83	81	77	80
0.250	62	51	72	72	72	74	65	67
0.150	43	42	59	59	61	67	54	51
0.075	32	32	44	43	44	52	43	38
0.060	29	29	40	39	38	47	40	34
0.050	27	27	38	37	35	44	38	32
0.035	24	23	35	33	31	40	35	28
0.020	22	20	32	29	27	34	31	25
0.006	18	18	29	26	22	27	27	22
0.002	14	17	27	23	19	25	23	21
GM	1.04	1.39	0.90	0.75	0.80	0.80	0.89	0.94
Atterberg Limits								
LL (%)	19	30	38	29	35	36	28	30
PI (%)	5	14	20	12	13	17	11	13
LS (%)	2.5	6.5	9.5	6.0	7.0	8.5	5.5	6.5
pH & Conductivity								
pH			6.1			5.9		
EC (S/m)			0.023			0.015		
MDD / OMC								
MDD (kg/m³)	2000	1968		1872	1675		2022	1965
OMC (%)	12.5	11.6		13.5	18.9		11.1	11.7
CBR								
100%	15	8.2		8.7	10.0		15	13
98%	13	7.2		8.2	9.8		14	12
97%	12	6.8		7.7	9.4		13	11
95%	9	6.0		6.8	8.6		12	10
93%	6	5.3		6.1	7.0		10	9
90%	4	4.4		5.1	6.8		8	7
Swell (%)	0.0	0.1		0.0	0.4		0.1	0.3
UCS (MPa)								
100%								
97%								
90%								
COLTO Classification								
	*	*		*	*		G8	*
Remarks:	*= Not Classifiable							

Client Name: BAV Consulting (Pty) Ltd
Project Name: Komati Power Station Repurposing
Job Number: PVT-291
Date: 26-Oct-23
Method: SANS 3001 GR1, GR3, GR10, GR12 GR20, GR30, GR31, GR40, GR50, GR53, GR54 & BS 1377 (where applicable)

SUMMARY OF TEST DATA	Sheet Reference: R-STL-038 Rev01
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Grading & Hydrometer Analysis (% Passing)								
Sample	KTP35	KTP36	KTP37	KTP39	KTP41	KTP42	KTP43	KTP44
Depth (m)	0.2 - 2.6	0.9 - 1.9	0.3 - 2.4	1.1 - 3.2	1.4 - 2.0	0.7 - 2.5	0.1 - 2.4	0.1 - 2.6
Lab No	PVT-291-2056	PVT-291-2057	PVT-291-2058	PVT-291-2060	PVT-291-2062	PVT-291-2063	PVT-291-2064	PVT-291-2065
53.0	100	100	100	100	100	100	100	100
37.5	100	100	100	100	100	100	100	100
26.5	100	100	100	100	100	100	100	100
19.0	100	100	100	100	100	100	100	100
13.2	100	100	92	98	100	100	100	100
9.5	99	100	85	96	99	100	100	98
6.7	95	100	72	93	99	100	100	96
4.75	92	100	62	91	99	100	100	94
2.0	85	100	48	88	98	98	100	91
1.0	83	96	45	86	96	97	99	89
0.425	75	79	40	76	81	90	89	82
0.250	59	46	34	59	47	81	78	68
0.150	48	29	30	46	31	67	68	52
0.075	35	20	23	33	22	49	54	36
0.060	31	17	20	29	17	41	47	30
0.050	28	15	19	27	15	37	44	27
0.035	24	12	17	23	12	31	40	23
0.020	20	9	15	19	10	28	37	21
0.006	17	5	9	12	7	26	33	19
0.002	15	3	3	4	4	25	30	18
GM	1.05	1.01	1.89	1.03	0.99	0.63	0.57	0.91

Atterberg Limits								
LL (%)	25	-	29	27	-	28	22	25
PI (%)	9	NP	13	13	NP	13	8	10
LS (%)	4.0	0.0	6.5	6.5	0.0	6.5	4.0	5.0

pH & Conductivity								
pH			5.5	4.7		5.4		
EC (S/m)			0.007	0.070		0.014		

MDD / OMC								
MDD (kg/m ³)	2030	1914		1910	1937	1889		1942
OMC (%)	10.3	10.5		12.0	10.0	13.9		11.1

CBR								
100%	16	13		7.6	21	15		17
98%	15	12		6.6	18	14		16
97%	14	11		6.2	16	13		15
95%	12	10		4.9	14	11		13
93%	11	8		3.7	12	9		11
90%	9	5		2.5	9	7		9
Swell (%)	0.1	0.0		0.7	0.1	0.0		0.0

UCS (MPa)								
100%								
97%								
90%								

COLTO Classification								
	G9	G9		*	G8	*		G8

Remarks: *= Not Classifiable

Client Name: BAV Consulting (Pty) Ltd
Project Name: Komati Power Station Repurposing
Job Number: PVT-291
Date: 26-Oct-23
Method: SANS 3001 GR1, GR3, GR10, GR12 GR20, GR30, GR31, GR40, GR50, GR53, GR54 & BS 1377 (where applicable)

SUMMARY OF TEST DATA	Sheet Reference: R-STL-038 Rev01
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Grading & Hydrometer Analysis (% Passing)								
Sample	KTP45	KTP46	KTP47					
Depth (m)	0.1 - 2.2	1.0 - 2.3	0.4 - 2.2					
Lab No	PVT-291-2066	PVT-291-2067	PVT-291-2068					
53.0	100	100	100					
37.5	100	100	100					
26.5	100	100	100					
19.0	100	100	100					
13.2	100	100	100					
9.5	100	100	99					
6.7	100	100	98					
4.75	100	100	96					
2.0	99	99	94					
1.0	98	97	92					
0.425	92	90	85					
0.250	75	79	73					
0.150	60	70	63					
0.075	46	57	52					
0.060	40	51	48					
0.050	37	48	46					
0.035	31	42	43					
0.020	28	39	39					
0.006	25	35	36					
0.002	23	33	35					
GM	0.63	0.54	0.69					

Atterberg Limits								
LL (%)	29	32	40					
PI (%)	11	17	23					
LS (%)	6.0	8.5	12.0					

pH & Conductivity								
pH			6.4					
EC (S/m)			0.027					

MDD / OMC								
MDD (kg/m ³)		1911						
OMC (%)		12.9						

CBR								
100%		11						
98%		10						
97%		9						
95%		8						
93%		6						
90%		4						
Swell (%)		0.1						

UCS (MPa)								
100%								
97%								
90%								

COLTO Classification								
		*						

Remarks: *= Not Classifiable



**Specialised
Testing
Laboratory** (Pty) Ltd
Asphalt | Aggregate | Bitumen | Geotechnical

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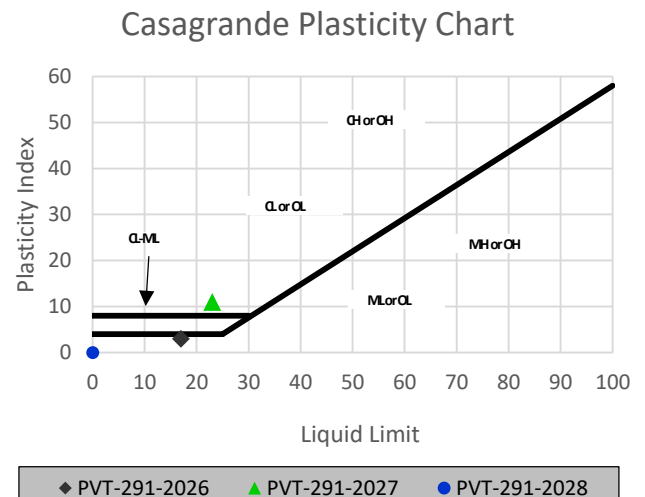
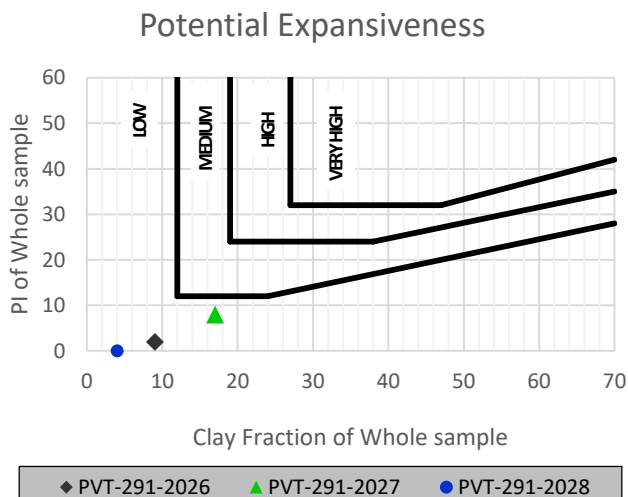
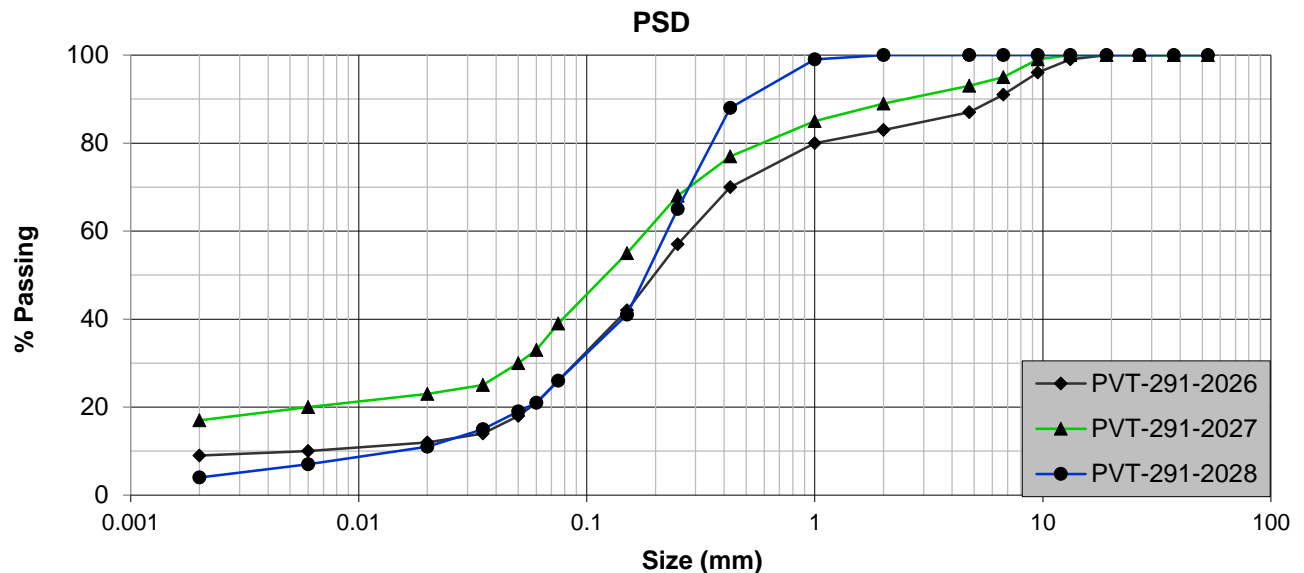
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Quality | Excellence | On Time

Client Name: BAV Consulting (Pty) Ltd
Project Name: Komati Power Station Repurposing
Job Number: PVT-291
Date: 2023-10-26
Method: SANS 3001 GR1, GR3, GR10 GR12 & BS 1377 (where applicable)

FOUNDATION INDICATOR

Sheet Reference:
R-STL-011 Rev02



Although everything possible is done to ensure testing is performed accurately, neither Specialised Testing Laboratory (Pty) Ltd nor any of its directors, managers, employees or contractors can be held liable for any damages whatsoever arising from any error made in performing any tests, nor from any conclusions drawn therefrom. Test results are to be published in full. Samples will be kept for 1 month after the submission of test results due to limited storage space, unless other arrangements are in place. Confidentiality statement: Unless the release of information is required by law or covered by confidentiality agreements all information obtained or created during the performance of laboratory activities will be kept confidential.



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Laboratory** (Pty) Ltd
Asphalt | Aggregate | Bitumen | Geotechnical

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Quality | Excellence | On Time

Client Name: BAV Consulting (Pty) Ltd
Project Name: Komati Power Station Repurposing
Job Number: PVT-291
Date: 2023-10-26
Method: SANS 3001 GR1, GR3, GR10 GR12 & BS 1377 (where applicable)

FOUNDATION INDICATOR

Sheet Reference:
R-STL-011 Rev02

Grading & Hydrometer Analysis (Particle Size (mm) & % Passing)

Atterberg Limits & Classification

Sample	KTP4	KTP5	KTP7	Sample	KTP4	KTP5	KTP7
Depth (m)	0.3 - 2.3	0.4 - 2.5	0.0 - 1.9	Depth (m)	0.3 - 2.3	0.4 - 2.5	0.0 - 1.9
Lab No	PVT-291-2029	PVT-291-2030	PVT-291-2032	Lab No	PVT-291-2029	PVT-291-2030	PVT-291-2032
53.0	100	100	100	Liquid Limit (%)	29	29	20
37.5	100	100	100	Plastic Limit (%)	13	14	15
26.5	100	100	100	Plasticity Index (%)	16	15	5
19.0	100	100	100	Linear Shrinkage (%)	8.0	7.5	2.5
13.2	100	99	99	PI of whole sample	12	11	2
9.5	99	97	85				
6.7	96	95	67	% Gravel	11	16	52
4.75	93	91	58	% Sand	55	53	34
2.00	89	84	48	% Silt	13	12	9
1.00	86	81	45	% Clay	21	19	5
0.425	78	73	39	Activity	0.8	0.8	1.0
0.250	66	64	34				
0.150	53	51	26	% Soil Mortar	89	84	48
0.075	38	36	17				
0.060	34	31	14	Grading Modulus	0.95	1.07	1.96
0.050	32	29	13	Moisture Content (%)	N / T	N / T	N / T
0.035	29	25	11	Relative Density (SG)*	2.65	2.65	2.65
0.020	26	23	9				
0.006	23	20	7	Unified (ASTM D2487)	SC	SC	GC-GM
0.002	21	19	5	AASHTO (M145-91)	A - 6	A - 6	A - 1 - b

Remarks: *: Assumed

N / T: Not Tested

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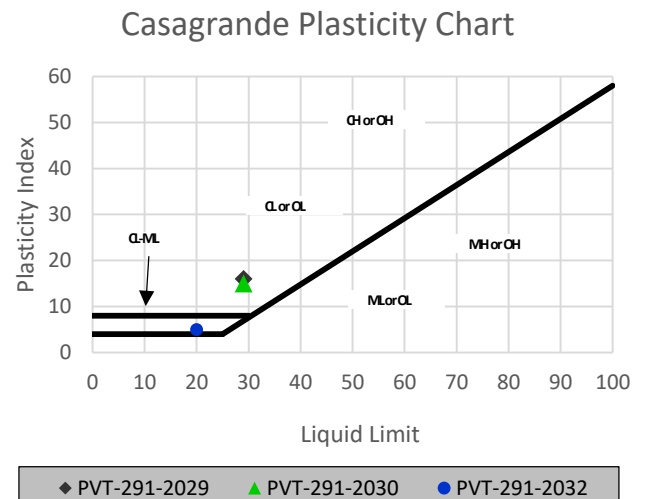
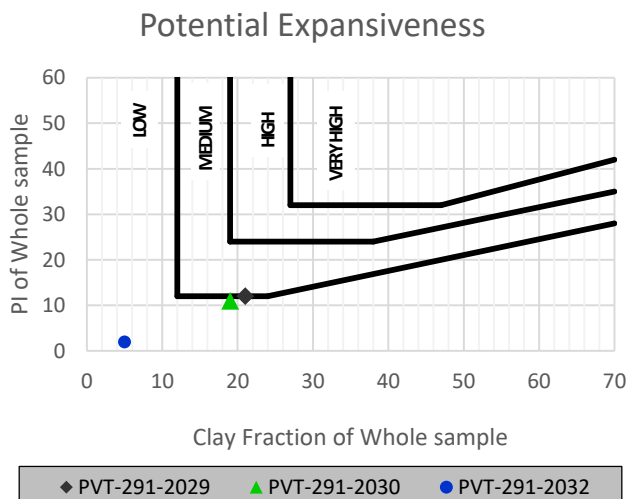
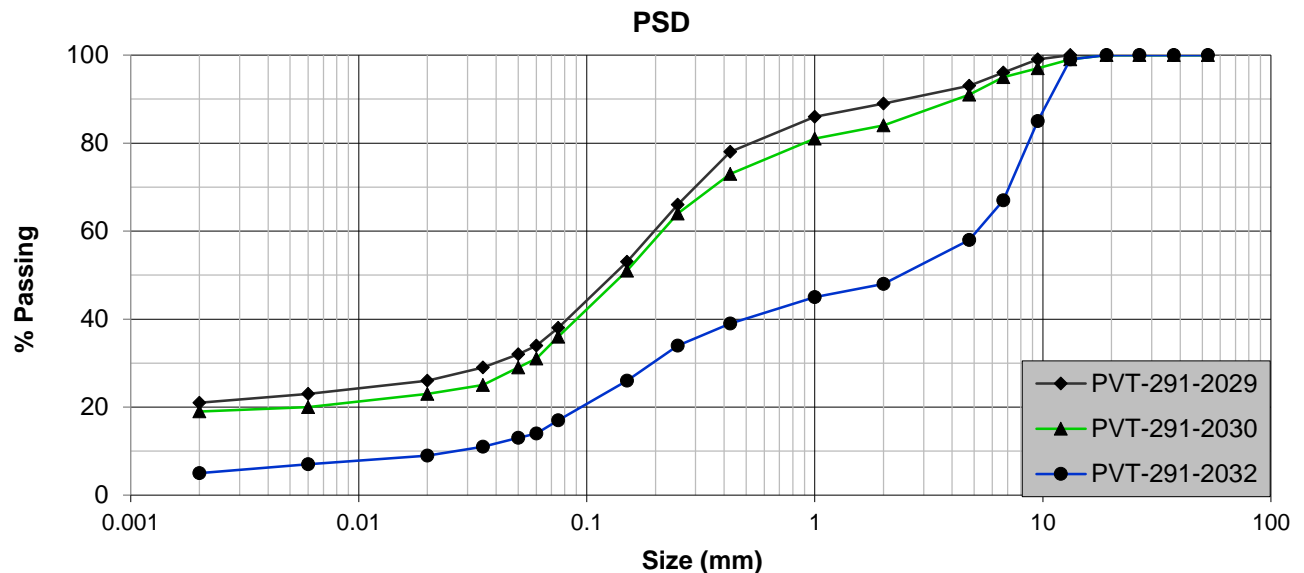
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Client Name: BAV Consulting (Pty) Ltd
Project Name: Komati Power Station Repurposing
Job Number: PVT-291
Date: 2023-10-26
Method: SANS 3001 GR1, GR3, GR10 GR12 & BS 1377 (where applicable)

FOUNDATION INDICATOR

Sheet Reference:
R-STL-011 Rev02



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FOUNDATION INDICATOR						Sheet Reference: R-STL-011 Rev02	
Grading & Hydrometer Analysis (Particle Size (mm) & % Passing)				Atterberg Limits & Classification			
Sample	KTP8	KTP12	KTP13	Sample	KTP8	KTP12	KTP13
Depth (m)	0.4 - 2.3	0.0 - 0.6	0.3 - 1.8	Depth (m)	0.4 - 2.3	0.0 - 0.6	0.3 - 1.8
Lab No	PVT-291-2033	PVT-291-2035	PVT-291-2036	Lab No	PVT-291-2033	PVT-291-2035	PVT-291-2036
53.0	100	100	100	Liquid Limit (%)	26	-	29
37.5	100	100	100	Plastic Limit (%)	13	-	15
26.5	100	100	100	Plasticity Index (%)	13	SP	14
19.0	100	100	100	Linear Shrinkage (%)	6.0	0.5	7.0
13.2	100	100	100	PI of whole sample	11	-	11
9.5	100	98	100				
6.7	99	96	98	% Gravel	4	9	12
4.75	98	93	94	% Sand	56	72	50
2.00	96	91	88	% Silt	20	13	14
1.00	93	89	85	% Clay	20	6	24
0.425	84	75	76	Activity	0.7	0.0	0.6
0.250	75	60	66				
0.150	60	40	51	% Soil Mortar	96	91	88
0.075	47	23	41				
0.060	40	19	38	Grading Modulus	0.73	1.11	0.95
0.050	37	17	36	Moisture Content (%)	N / T	N / T	N / T
0.035	31	14	33	Relative Density (SG)*	2.65	2.65	2.65
0.020	28	12	30				
0.006	24	9	26	Unified (ASTM D2487)	SC	SM	SC
0.002	20	6	24	AASHTO (M145-91)	A - 6	A - 2 - 4	A - 6
Remarks: *: Assumed							
N / T: Not Tested							
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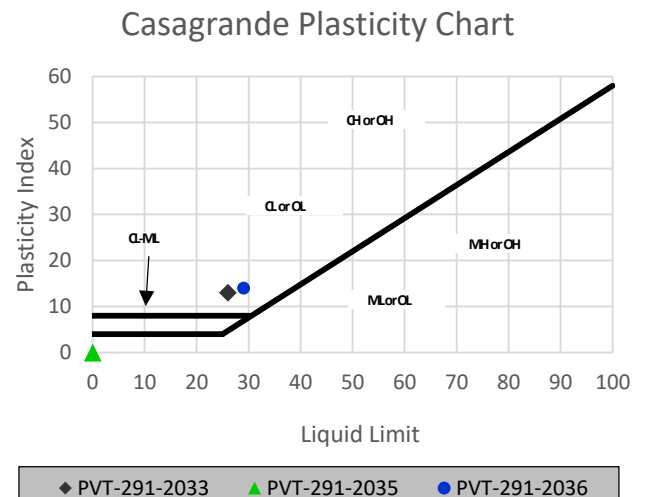
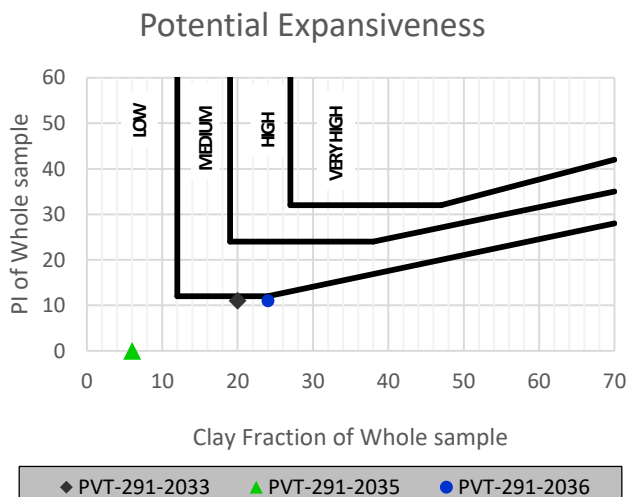
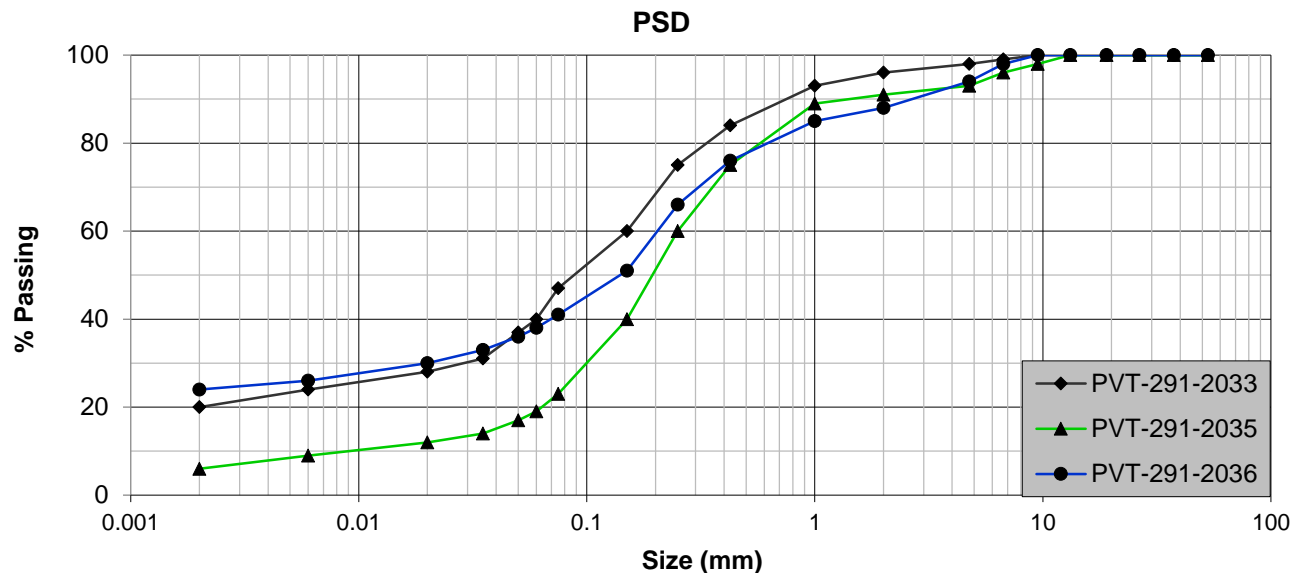
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Client Name: BAV Consulting (Pty) Ltd
Project Name: Komati Power Station Repurposing
Job Number: PVT-291
Date: 2023-10-26
Method: SANS 3001 GR1, GR3, GR10 GR12 & BS 1377 (where applicable)

FOUNDATION INDICATOR

Sheet Reference:
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Date: 2023-10-26
Method: SANS 3001 GR1, GR3, GR10 GR12 & BS 1377 (where applicable)

FOUNDATION INDICATOR

Sheet Reference:
R-STL-011 Rev02

Grading & Hydrometer Analysis (Particle Size (mm) & % Passing)

Atterberg Limits & Classification

Sample	KTP14	KTP16	KTP19	Sample	KTP14	KTP16	KTP19
Depth (m)	0.3 - 1.8	0.3 - 2.1	0.6 - 2.3	Depth (m)	0.3 - 1.8	0.3 - 2.1	0.6 - 2.3
Lab No	PVT-291-2037	PVT-291-2038	PVT-291-2040	Lab No	PVT-291-2037	PVT-291-2038	PVT-291-2040
53.0	100	100	100	Liquid Limit (%)	33	29	26
37.5	100	100	100	Plastic Limit (%)	16	16	14
26.5	100	100	100	Plasticity Index (%)	17	13	12
19.0	100	100	100	Linear Shrinkage (%)	8.5	6.5	5.5
13.2	100	100	99	PI of whole sample	15	5	7
9.5	100	100	97				
6.7	100	96	88	% Gravel	3	37	29
4.75	99	89	80	% Sand	53	42	45
2.00	97	63	71	% Silt	22	12	16
1.00	94	52	68	% Clay	22	9	10
0.425	86	42	60	Activity	0.8	1.4	1.2
0.250	78	37	51				
0.150	62	31	41	% Soil Mortar	97	63	71
0.075	48	24	30				
0.060	44	21	26	Grading Modulus	0.69	1.71	1.39
0.050	41	19	24	Moisture Content (%)	N / T	N / T	N / T
0.035	37	17	21	Relative Density (SG)*	2.65	2.65	2.65
0.020	32	15	18				
0.006	26	12	13	Unified (ASTM D2487)	SC	SC	SC
0.002	22	9	10	AASHTO (M145-91)	A - 6	A - 2 - 6	A - 2 - 6

Remarks: *: Assumed

N / T: Not Tested

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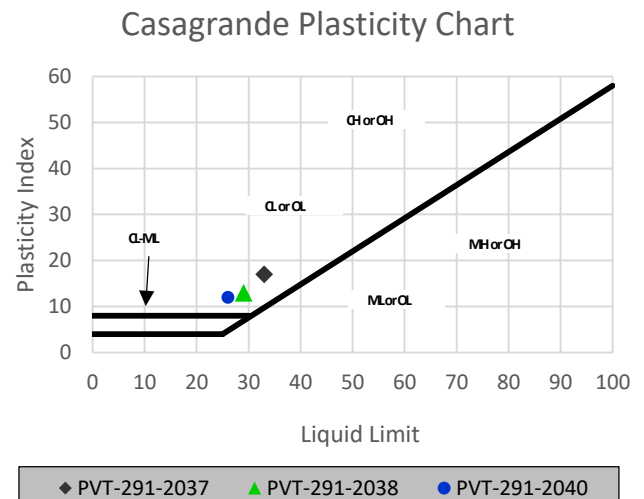
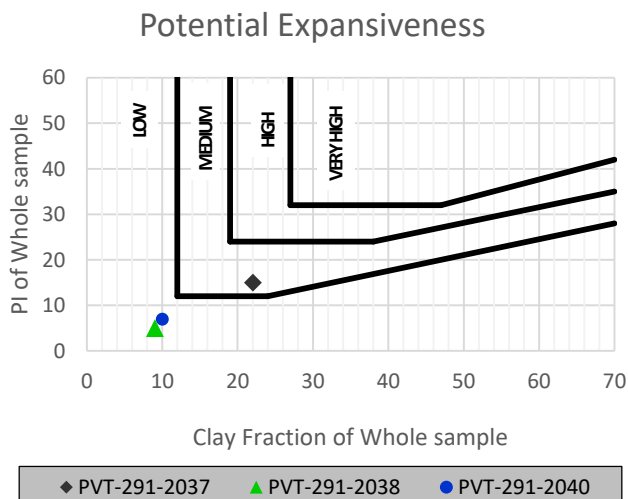
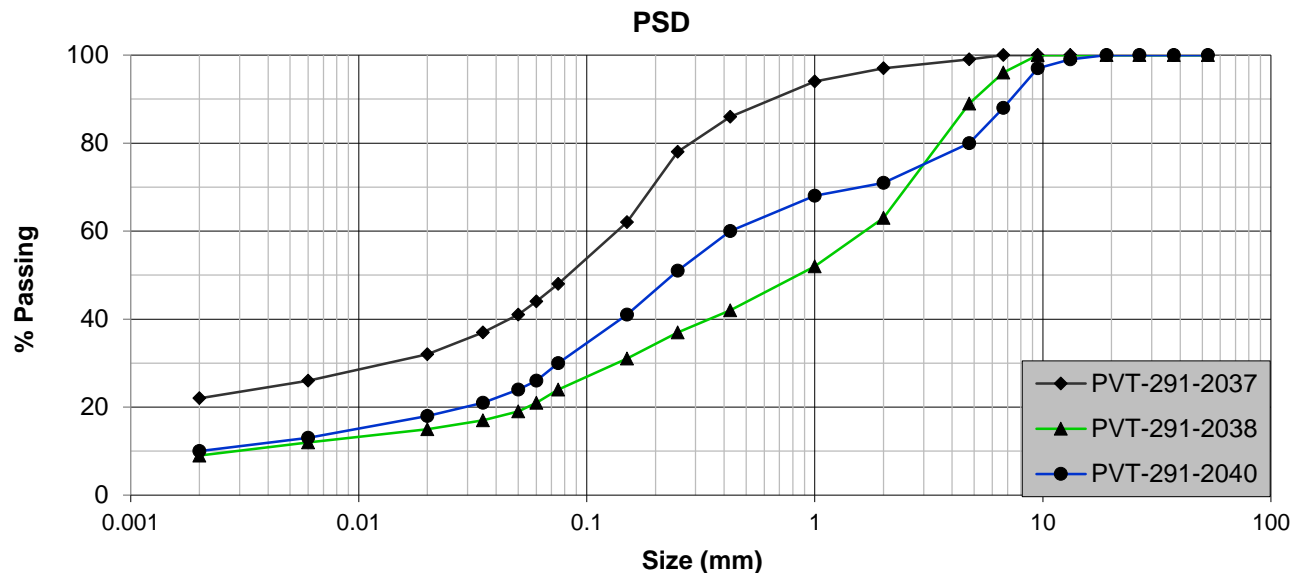
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Client Name: BAV Consulting (Pty) Ltd
Project Name: Komati Power Station Repurposing
Job Number: PVT-291
Date: 2023-10-26
Method: SANS 3001 GR1, GR3, GR10 GR12 & BS 1377 (where applicable)

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FOUNDATION INDICATOR

Sheet Reference:
R-STL-011 Rev02

Grading & Hydrometer Analysis (Particle Size (mm) & % Passing)

Atterberg Limits & Classification

Sample	KTP20	KTP21	KTP22	Sample	KTP20	KTP21	KTP22
Depth (m)	0.1 - 2.7	0.1 - 2.4	0.2 - 2.3	Depth (m)	0.1 - 2.7	0.1 - 2.4	0.2 - 2.3
Lab No	PVT-291-2041	PVT-291-2042	PVT-291-2043	Lab No	PVT-291-2041	PVT-291-2042	PVT-291-2043
53.0	100	100	100	Liquid Limit (%)	27	26	20
37.5	100	100	100	Plastic Limit (%)	15	16	14
26.5	100	100	100	Plasticity Index (%)	12	10	6
19.0	100	100	100	Linear Shrinkage (%)	6.0	5.0	3.0
13.2	100	100	100	PI of whole sample	11	7	5
9.5	100	98	100				
6.7	99	92	99	% Gravel	4	25	4
4.75	98	84	98	% Sand	58	48	66
2.00	96	75	96	% Silt	21	14	20
1.00	96	72	94	% Clay	17	13	10
0.425	92	65	88	Activity	0.7	0.8	0.6
0.250	83	60	77				
0.150	64	44	52	% Soil Mortar	96	75	96
0.075	43	31	35				
0.060	38	27	30	Grading Modulus	0.69	1.29	0.81
0.050	35	25	27	Moisture Content (%)	N / T	N / T	N / T
0.035	30	23	22	Relative Density (SG)*	2.65	2.65	2.65
0.020	26	20	19				
0.006	20	16	14	Unified (ASTM D2487)	SC	SC	SC-SM
0.002	17	13	10	AASHTO (M145-91)	A - 6	A - 2 - 4	A - 2 - 4

Remarks: *: Assumed

N / T: Not Tested

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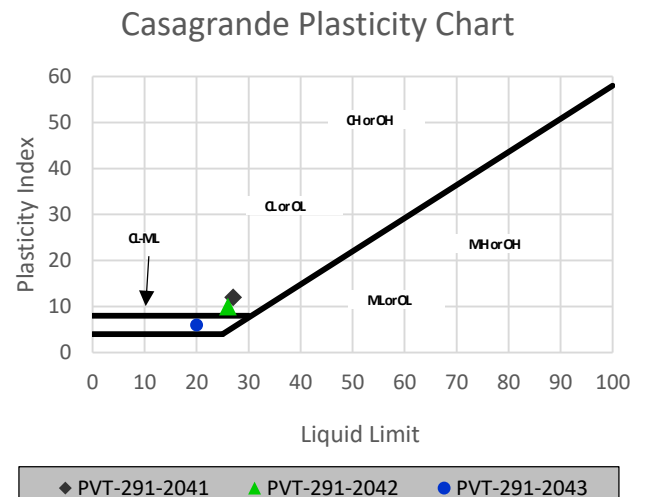
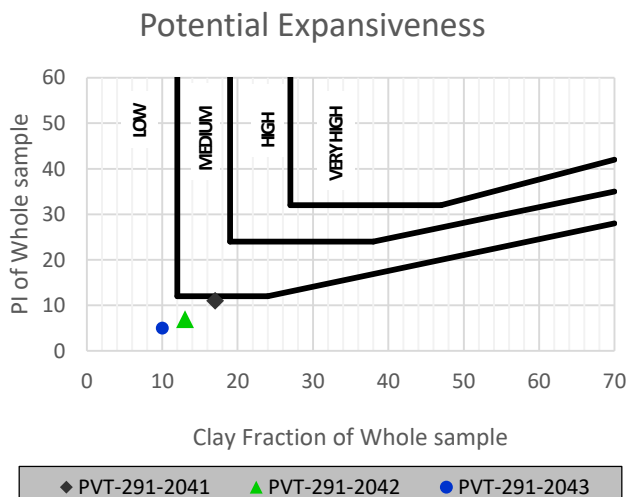
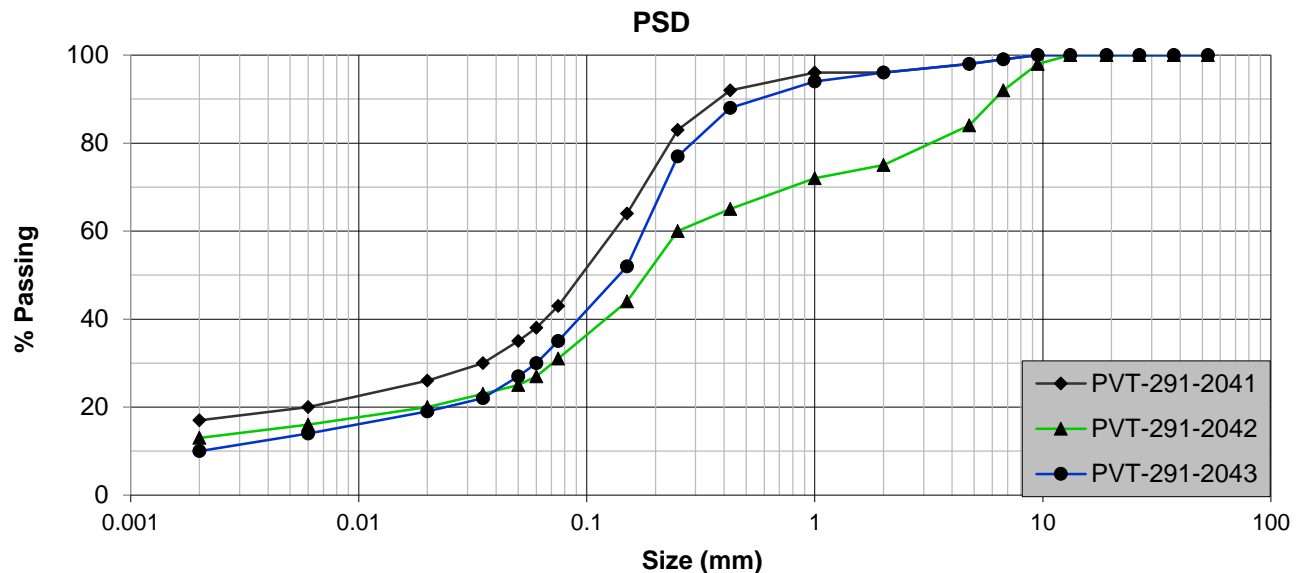
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Client Name: BAV Consulting (Pty) Ltd
Project Name: Komati Power Station Repurposing
Job Number: PVT-291
Date: 2023-10-26
Method: SANS 3001 GR1, GR3, GR10 GR12 & BS 1377 (where applicable)

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FOUNDATION INDICATOR

Sheet Reference:
R-STL-011 Rev02

Grading & Hydrometer Analysis (Particle Size (mm) & % Passing)

Atterberg Limits & Classification

Sample	KTP23	KTP25	KTP26	Sample	KTP23	KTP25	KTP26
Depth (m)	0.3 - 2.4	0.4 - 2.3	0.6 - 2.1	Depth (m)	0.3 - 2.4	0.4 - 2.3	0.6 - 2.1
Lab No	PVT-291-2044	PVT-291-2046	PVT-291-2047	Lab No	PVT-291-2044	PVT-291-2046	PVT-291-2047
53.0	100	100	100	Liquid Limit (%)	25	19	30
37.5	100	100	100	Plastic Limit (%)	15	14	16
26.5	100	100	100	Plasticity Index (%)	10	5	14
19.0	100	100	100	Linear Shrinkage (%)	5.0	2.5	6.5
13.2	100	100	99	PI of whole sample	7	4	8
9.5	98	99	95				
6.7	93	99	87	% Gravel	19	9	31
4.75	88	97	79	% Sand	53	62	40
2.00	81	91	69	% Silt	17	15	12
1.00	80	87	66	% Clay	11	14	17
0.425	72	73	60	Activity	0.9	0.4	0.8
0.250	64	62	51				
0.150	49	43	42	% Soil Mortar	81	91	69
0.075	33	32	32				
0.060	28	29	29	Grading Modulus	1.14	1.04	1.39
0.050	25	27	27	Moisture Content (%)	N / T	N / T	N / T
0.035	21	24	23	Relative Density (SG)*	2.65	2.65	2.65
0.020	17	22	20				
0.006	13	18	18	Unified (ASTM D2487)	SC	SC-SM	SC
0.002	11	14	17	AASHTO (M145-91)	A - 2 - 4	A - 2 - 4	A - 2 - 6

Remarks: *: Assumed

N / T: Not Tested

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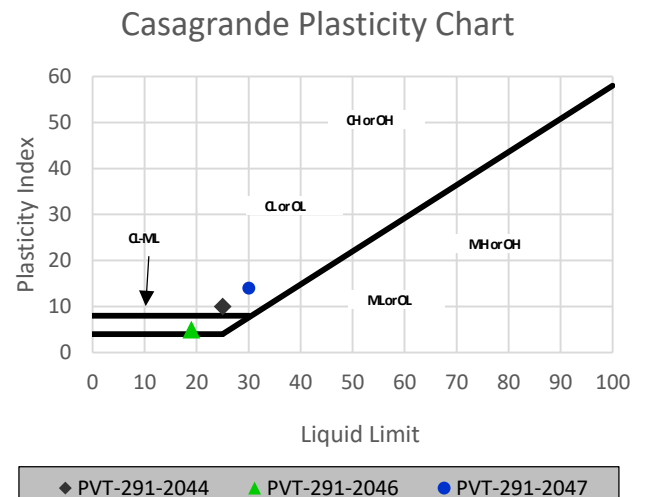
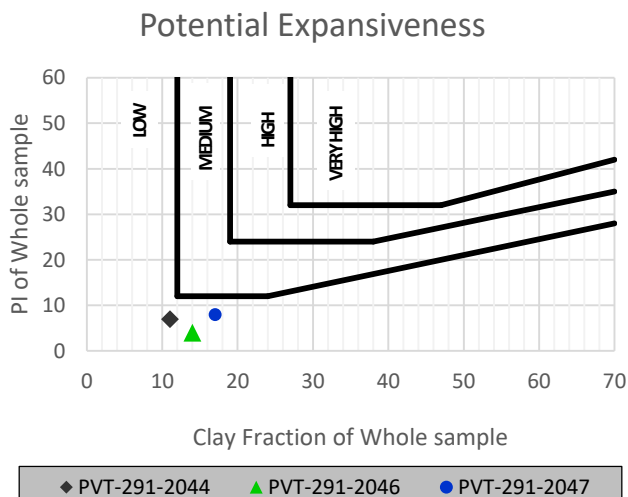
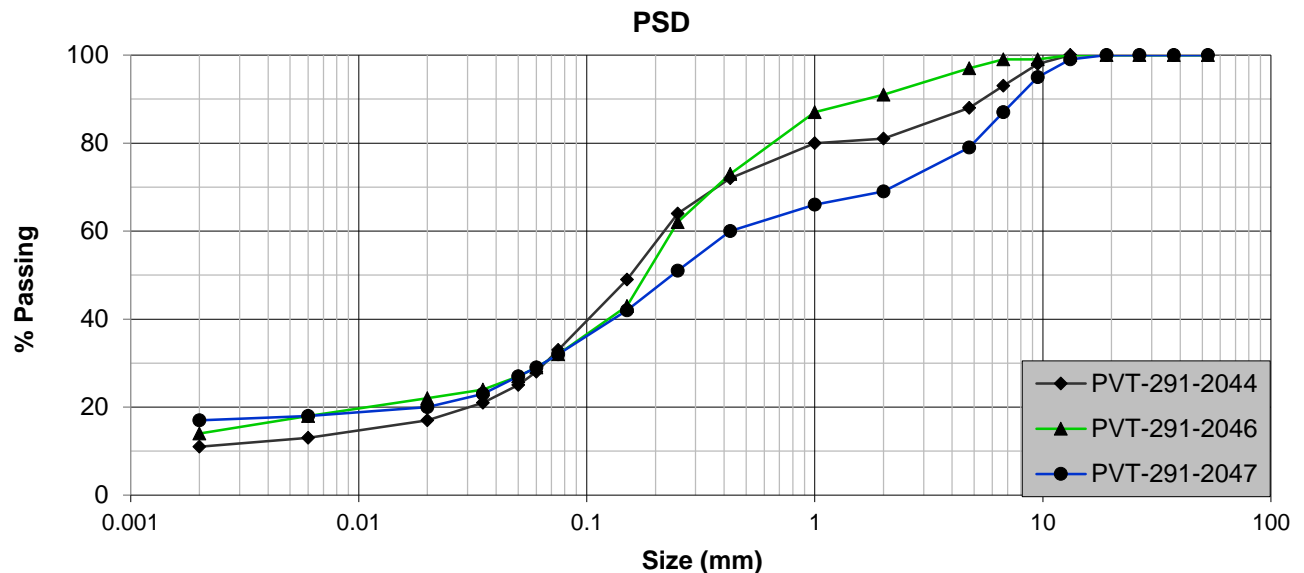
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Client Name: BAV Consulting (Pty) Ltd
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Job Number: PVT-291
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FOUNDATION INDICATOR

Sheet Reference:
R-STL-011 Rev02

Grading & Hydrometer Analysis (Particle Size (mm) & % Passing)

Atterberg Limits & Classification

Sample	KTP28	KTP29	KTP30	Sample	KTP28	KTP29	KTP30
Depth (m)	0.2 - 2.2	0.5 - 2.8	0.2 - 2.4	Depth (m)	0.2 - 2.2	0.5 - 2.8	0.2 - 2.4
Lab No	PVT-291-2049	PVT-291-2050	PVT-291-2051	Lab No	PVT-291-2049	PVT-291-2050	PVT-291-2051
53.0	100	100	100	Liquid Limit (%)	38	29	35
37.5	100	100	100	Plastic Limit (%)	18	17	22
26.5	100	100	100	Plasticity Index (%)	20	12	13
19.0	100	100	100	Linear Shrinkage (%)	9.5	6.0	7.0
13.2	100	100	100	PI of whole sample	16	10	11
9.5	98	100	100				
6.7	95	100	99	% Gravel	14	4	7
4.75	91	99	97	% Sand	46	57	55
2.00	86	96	93	% Silt	13	16	19
1.00	84	94	90	% Clay	27	23	19
0.425	80	86	83	Activity	0.7	0.5	0.7
0.250	72	72	72				
0.150	59	59	61	% Soil Mortar	86	96	93
0.075	44	43	44				
0.060	40	39	38	Grading Modulus	0.90	0.75	0.80
0.050	38	37	35	Moisture Content (%)	N / T	N / T	N / T
0.035	35	33	31	Relative Density (SG)*	2.65	2.65	2.65
0.020	32	29	27				
0.006	29	26	22	Unified (ASTM D2487)	SC	SC	SC
0.002	27	23	19	AASHTO (M145-91)	A - 6	A - 6	A - 6

Remarks: *: Assumed

N / T: Not Tested

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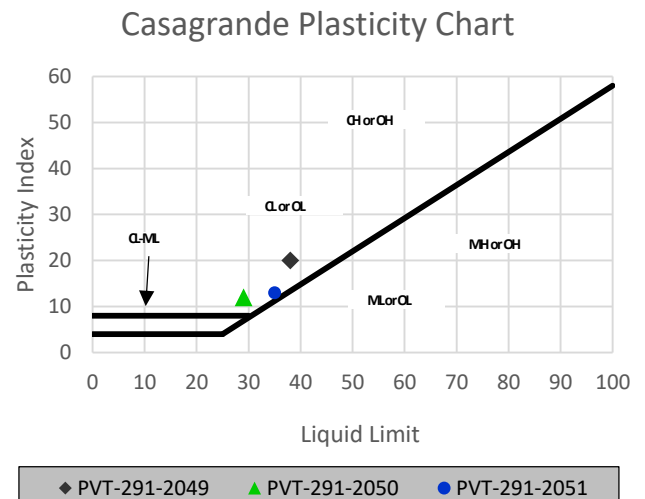
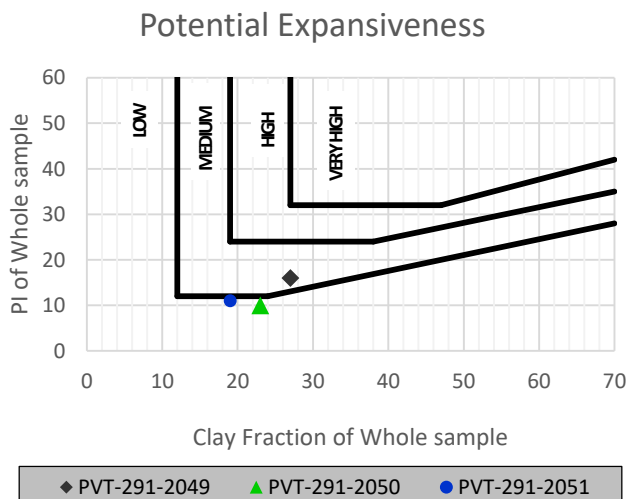
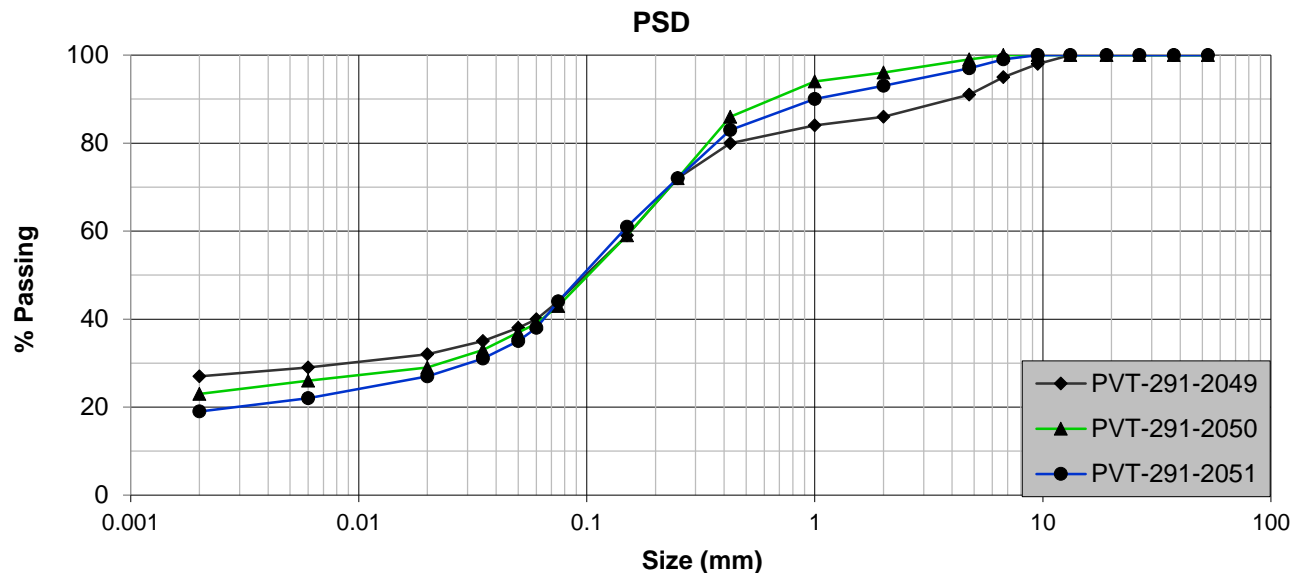
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Client Name: BAV Consulting (Pty) Ltd
Project Name: Komati Power Station Repurposing
Job Number: PVT-291
Date: 2023-10-26
Method: SANS 3001 GR1, GR3, GR10 GR12 & BS 1377 (where applicable)

FOUNDATION INDICATOR

Sheet Reference:
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Sheet Reference:
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Grading & Hydrometer Analysis (Particle Size (mm) & % Passing)				Atterberg Limits & Classification			
Sample	KTP31	KTP32	KTP33	Sample	KTP31	KTP32	KTP33
Depth (m)	0.2 - 2.1	0.2 - 2.2	0.2 - 2.4	Depth (m)	0.2 - 2.1	0.2 - 2.2	0.2 - 2.4
Lab No	PVT-291-2052	PVT-291-2053	PVT-291-2054	Lab No	PVT-291-2052	PVT-291-2053	PVT-291-2054
53.0	100	100	100	Liquid Limit (%)	36	28	30
37.5	100	100	100	Plastic Limit (%)	19	17	17
26.5	100	100	100	Plasticity Index (%)	17	11	13
19.0	100	100	100	Linear Shrinkage (%)	8.5	5.5	6.5
13.2	100	100	100	PI of whole sample	14	8	10
9.5	99	99	100				
6.7	95	98	98	% Gravel	13	9	12
4.75	91	96	94	% Sand	40	51	54
2.00	87	91	88	% Silt	22	17	13
1.00	86	89	86	% Clay	25	23	21
0.425	81	77	80	Activity	0.7	0.5	0.6
0.250	74	65	67				
0.150	67	54	51	% Soil Mortar	87	91	88
0.075	52	43	38				
0.060	47	40	34	Grading Modulus	0.80	0.89	0.94
0.050	44	38	32	Moisture Content (%)	N / T	N / T	N / T
0.035	40	35	28	Relative Density (SG)*	2.65	2.65	2.65
0.020	34	31	25				
0.006	27	27	22	Unified (ASTM D2487)	CL	SC	SC
0.002	25	23	21	AASHTO (M145-91)	A - 6	A - 6	A - 6

Remarks: *: Assumed

N / T: Not Tested

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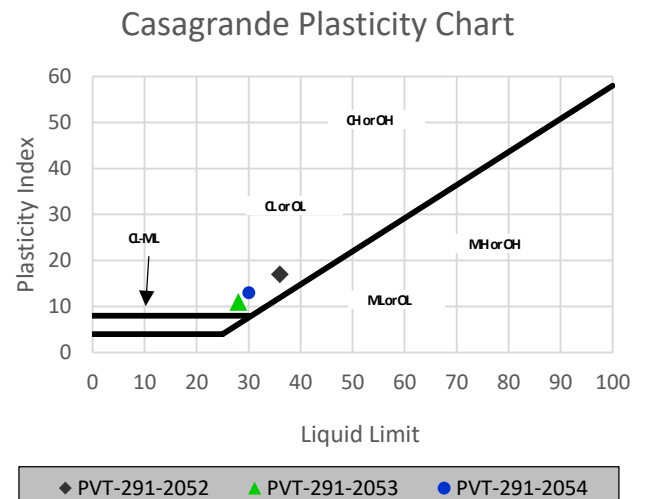
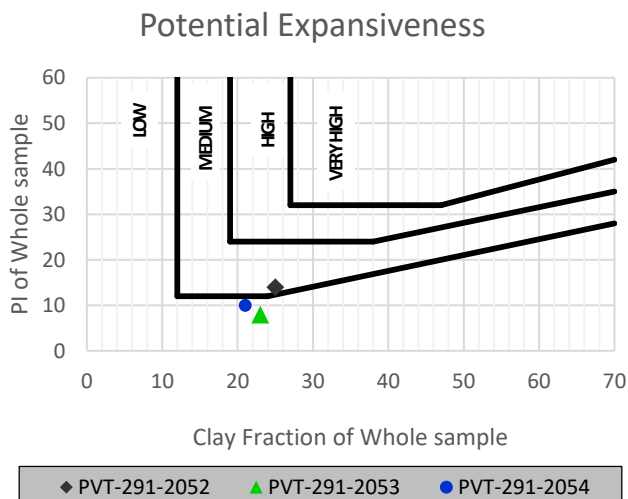
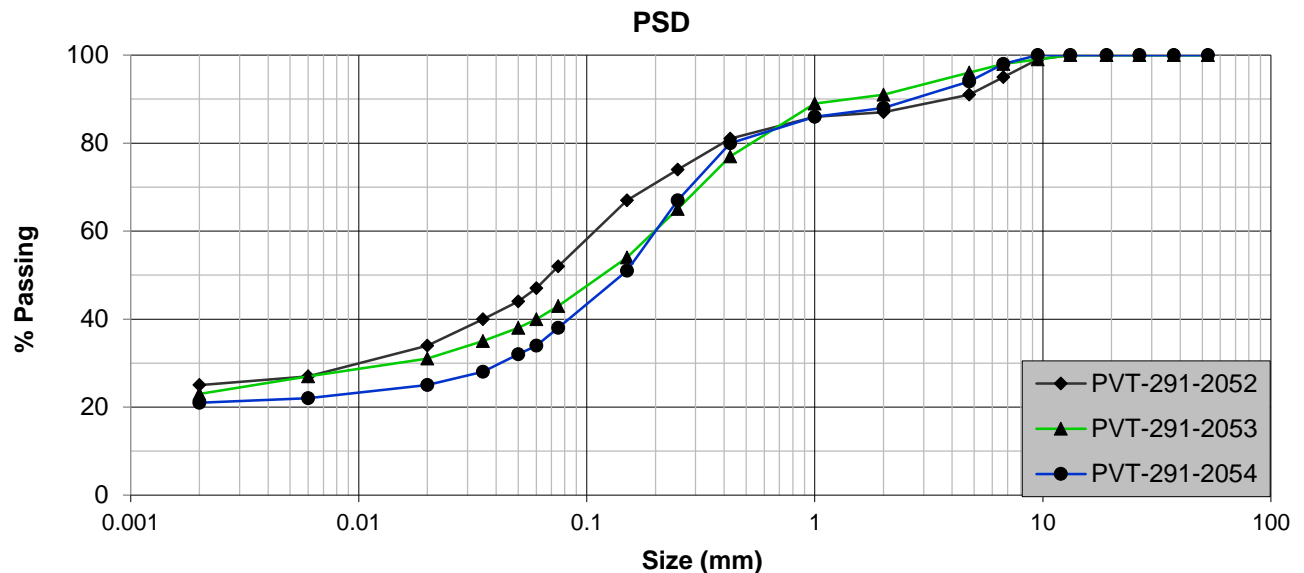
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Job Number: PVT-291
Date: 2023-10-26
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Method: SANS 3001 GR1, GR3, GR10 GR12 & BS 1377 (where applicable)

FOUNDATION INDICATOR

Sheet Reference:
R-STL-011 Rev02

Grading & Hydrometer Analysis (Particle Size (mm) & % Passing)

Atterberg Limits & Classification

Sample	KTP35	KTP36	KTP37	Sample	KTP35	KTP36	KTP37
Depth (m)	0.2 - 2.6	0.9 - 1.9	0.3 - 2.4	Depth (m)	0.2 - 2.6	0.9 - 1.9	0.3 - 2.4
Lab No	PVT-291-2056	PVT-291-2057	PVT-291-2058	Lab No	PVT-291-2056	PVT-291-2057	PVT-291-2058
53.0	100	100	100	Liquid Limit (%)	25	-	29
37.5	100	100	100	Plastic Limit (%)	16	-	16
26.5	100	100	100	Plasticity Index (%)	9	NP	13
19.0	100	100	100	Linear Shrinkage (%)	4.0	0.0	6.5
13.2	100	100	92	PI of whole sample	7	-	5
9.5	99	100	85				
6.7	95	100	72	% Gravel	15	0	52
4.75	92	100	62	% Sand	54	83	28
2.00	85	100	48	% Silt	16	14	17
1.00	83	96	45	% Clay	15	3	3
0.425	75	79	40	Activity	0.6	0.0	4.3
0.250	59	46	34				
0.150	48	29	30	% Soil Mortar	85	100	48
0.075	35	20	23				
0.060	31	17	20	Grading Modulus	1.05	1.01	1.89
0.050	28	15	19	Moisture Content (%)	N / T	N / T	N / T
0.035	24	12	17	Relative Density (SG)*	2.65	2.65	2.65
0.020	20	9	15				
0.006	17	5	9	Unified (ASTM D2487)	SC	SM	SC
0.002	15	3	3	AASHTO (M145-91)	A - 4	A - 2 - 4	A - 2 - 6

Remarks: *: Assumed

N / T: Not Tested

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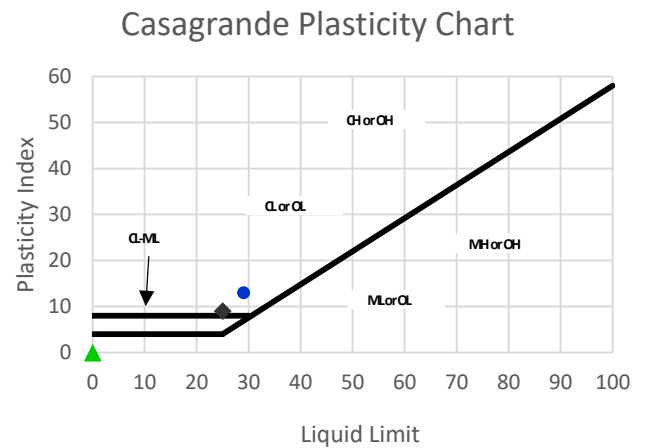
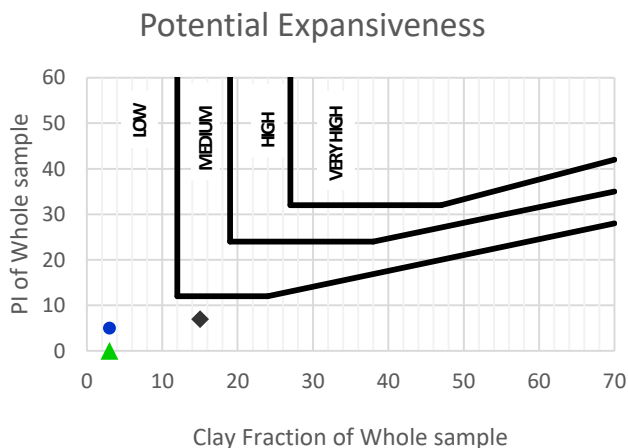
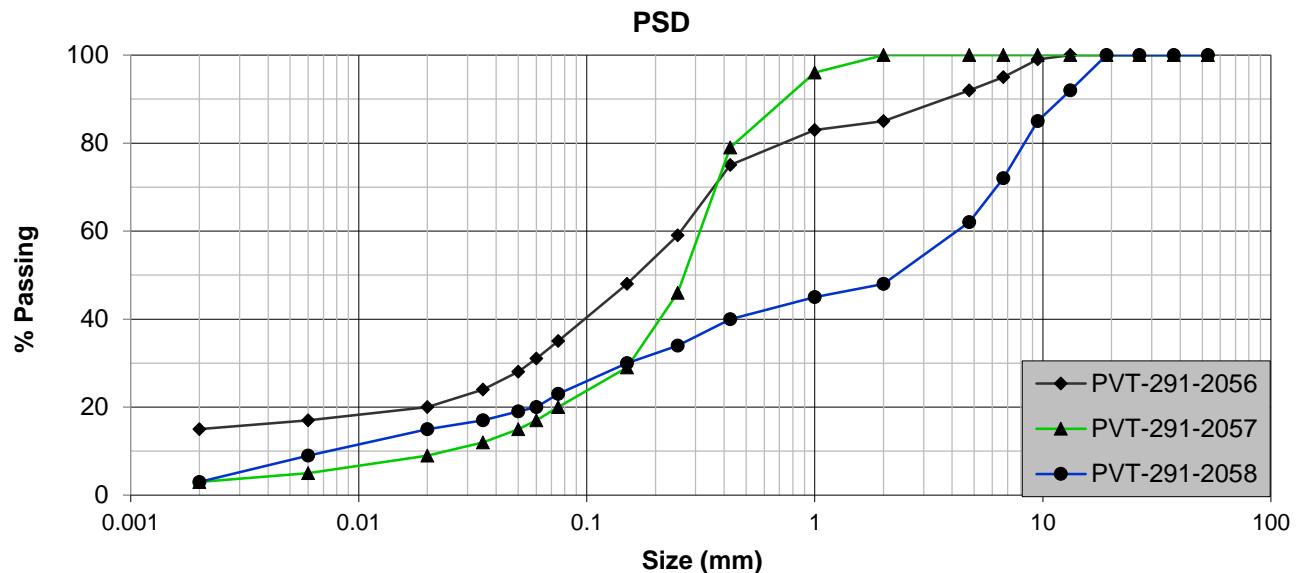
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Client Name: BAV Consulting (Pty) Ltd
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FOUNDATION INDICATOR

Sheet Reference:
R-STL-011 Rev02

Grading & Hydrometer Analysis (Particle Size (mm) & % Passing)

Atterberg Limits & Classification

Sample	KTP39	KTP41	KTP42	Sample	KTP39	KTP41	KTP42
Depth (m)	1.1 - 3.2	1.4 - 2.0	0.7 - 2.5	Depth (m)	1.1 - 3.2	1.4 - 2.0	0.7 - 2.5
Lab No	PVT-291-2060	PVT-291-2062	PVT-291-2063	Lab No	PVT-291-2060	PVT-291-2062	PVT-291-2063
53.0	100	100	100	Liquid Limit (%)	27	-	28
37.5	100	100	100	Plastic Limit (%)	14	-	15
26.5	100	100	100	Plasticity Index (%)	13	NP	13
19.0	100	100	100	Linear Shrinkage (%)	6.5	0.0	6.5
13.2	98	100	100	PI of whole sample	10	-	12
9.5	96	99	100				
6.7	93	99	100	% Gravel	12	2	2
4.75	91	99	100	% Sand	59	81	57
2.00	88	98	98	% Silt	25	13	16
1.00	86	96	97	% Clay	4	4	25
0.425	76	81	90	Activity	3.3	0.0	0.5
0.250	59	47	81				
0.150	46	31	67	% Soil Mortar	88	98	98
0.075	33	22	49				
0.060	29	17	41	Grading Modulus	1.03	0.99	0.63
0.050	27	15	37	Moisture Content (%)	N / T	N / T	N / T
0.035	23	12	31	Relative Density (SG)*	2.65	2.65	2.65
0.020	19	10	28				
0.006	12	7	26	Unified (ASTM D2487)	SC	SM	SC
0.002	4	4	25	AASHTO (M145-91)	A - 2 - 6	A - 2 - 4	A - 6

Remarks: *: Assumed

N / T: Not Tested

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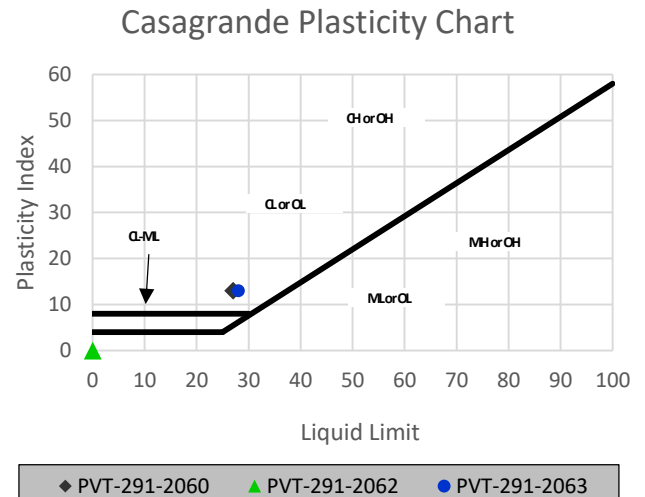
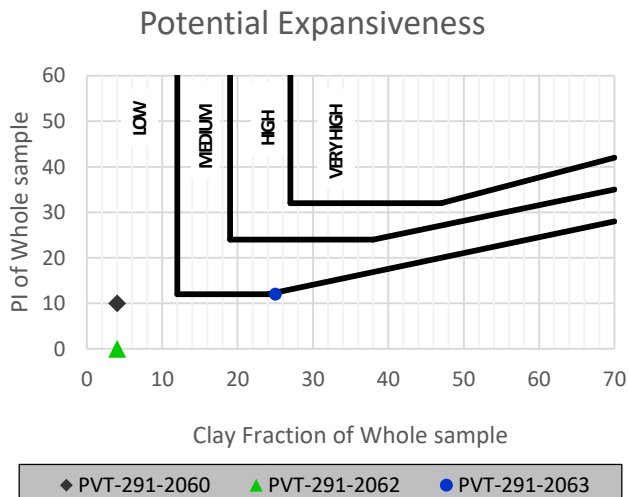
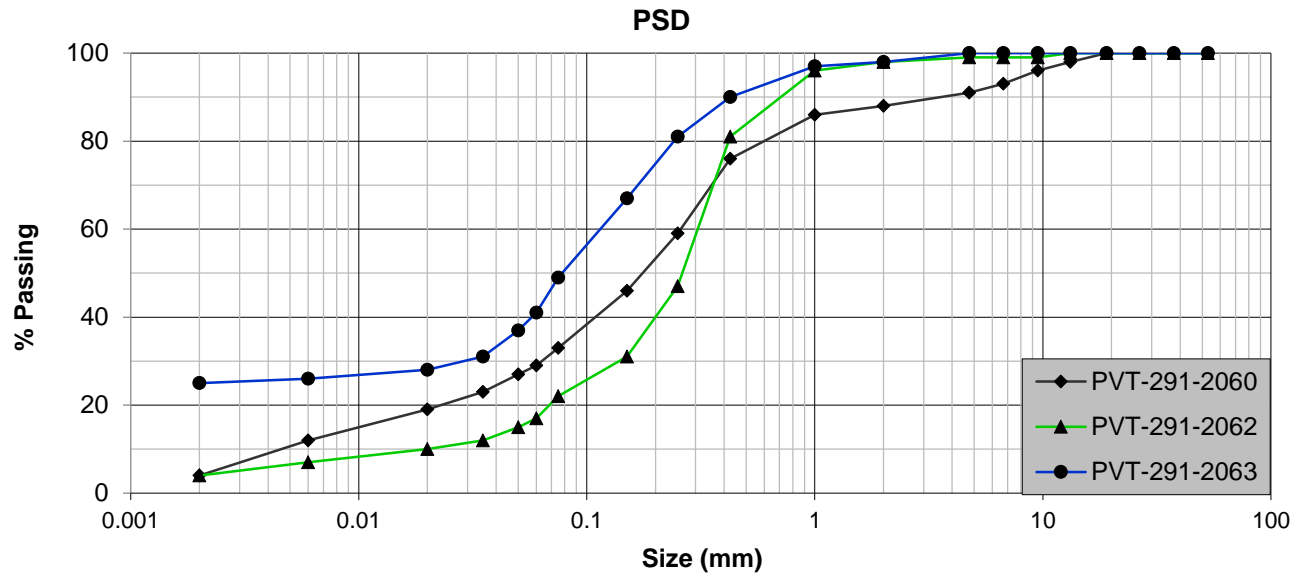
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FOUNDATION INDICATOR

Sheet Reference:
R-STL-011 Rev02

Grading & Hydrometer Analysis (Particle Size (mm) & % Passing)

Atterberg Limits & Classification

Sample	KTP43	KTP44	KTP45	Sample	KTP43	KTP44	KTP45
Depth (m)	0.1 - 2.4	0.1 - 2.6	0.1 - 2.2	Depth (m)	0.1 - 2.4	0.1 - 2.6	0.1 - 2.2
Lab No	PVT-291-2064	PVT-291-2065	PVT-291-2066	Lab No	PVT-291-2064	PVT-291-2065	PVT-291-2066
53.0	100	100	100	Liquid Limit (%)	22	25	29
37.5	100	100	100	Plastic Limit (%)	14	15	18
26.5	100	100	100	Plasticity Index (%)	8	10	11
19.0	100	100	100	Linear Shrinkage (%)	4.0	5.0	6.0
13.2	100	100	100	PI of whole sample	7	8	10
9.5	100	98	100				
6.7	100	96	100	% Gravel	0	9	1
4.75	100	94	100	% Sand	53	61	59
2.00	100	91	99	% Silt	17	12	17
1.00	99	89	98	% Clay	30	18	23
0.425	89	82	92	Activity	0.3	0.6	0.5
0.250	78	68	75				
0.150	68	52	60	% Soil Mortar	100	91	99
0.075	54	36	46				
0.060	47	30	40	Grading Modulus	0.57	0.91	0.63
0.050	44	27	37	Moisture Content (%)	N / T	N / T	N / T
0.035	40	23	31	Relative Density (SG)*	2.65	2.65	2.65
0.020	37	21	28				
0.006	33	19	25	Unified (ASTM D2487)	CL	SC	SC
0.002	30	18	23	AASHTO (M145-91)	A - 4	A - 4	A - 6

Remarks: *: Assumed

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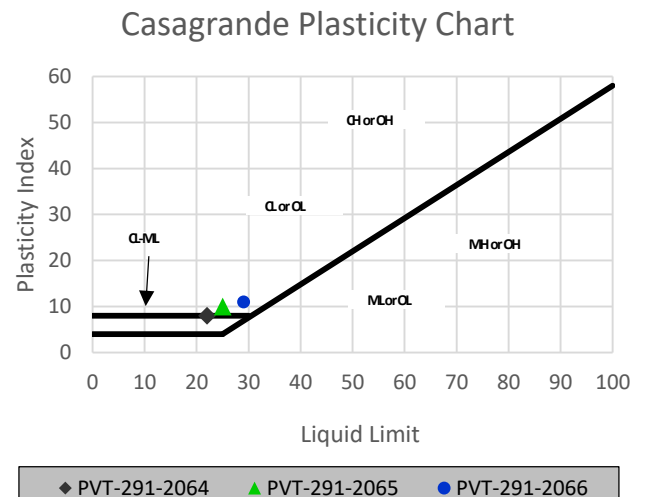
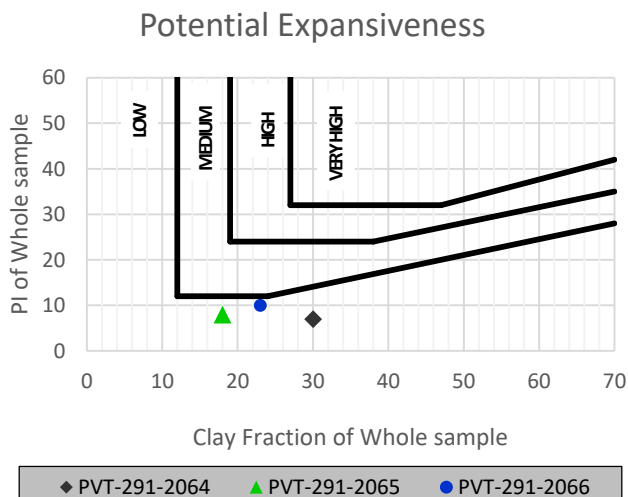
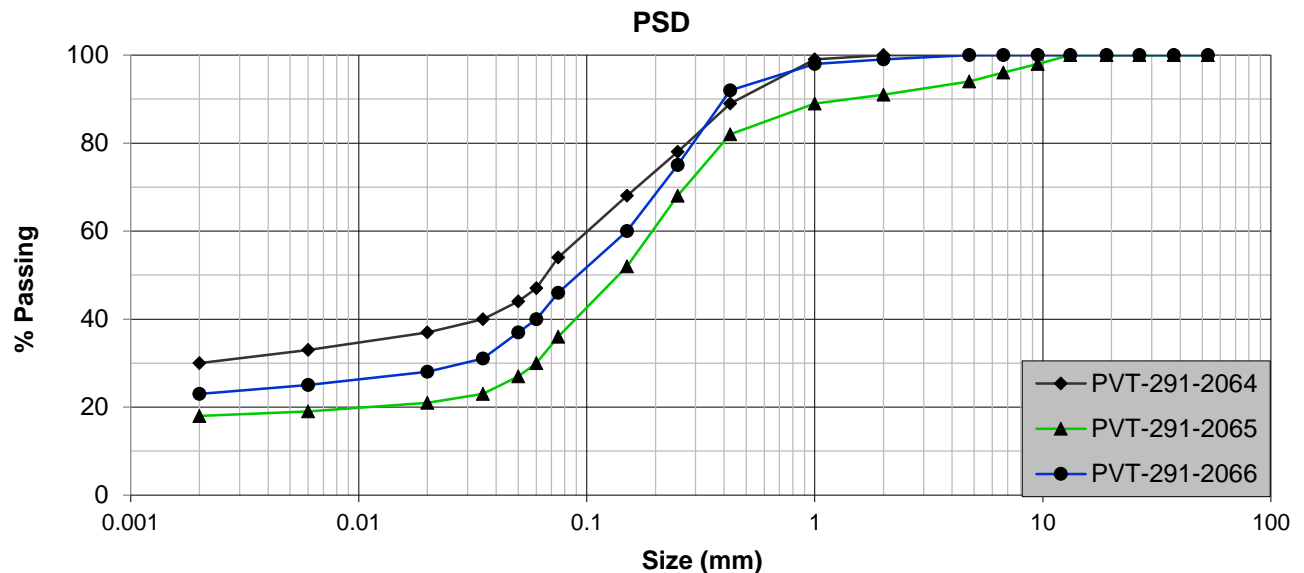
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Job Number: PVT-291
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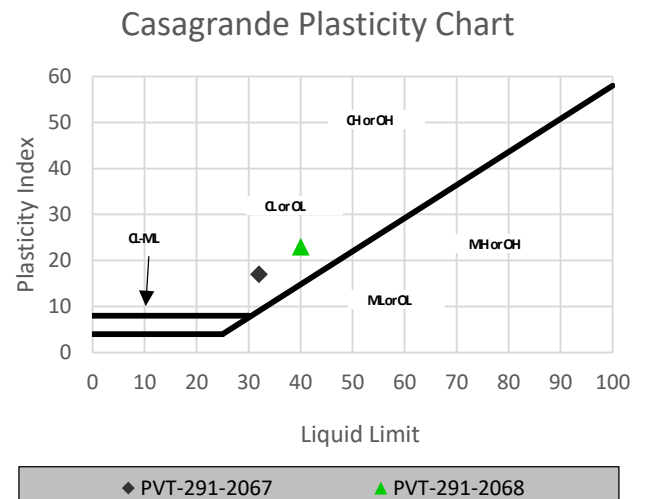
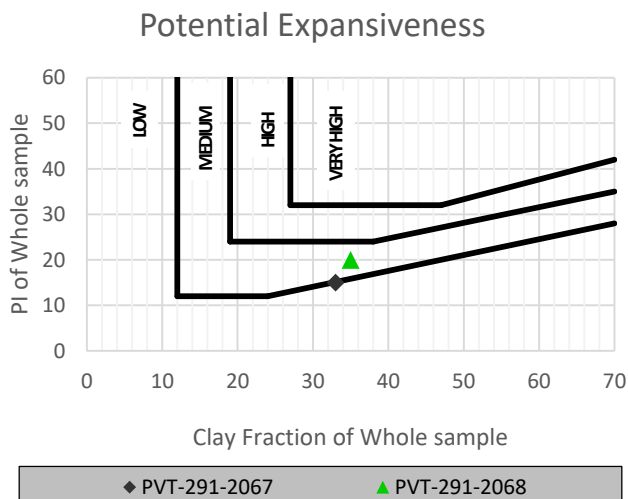
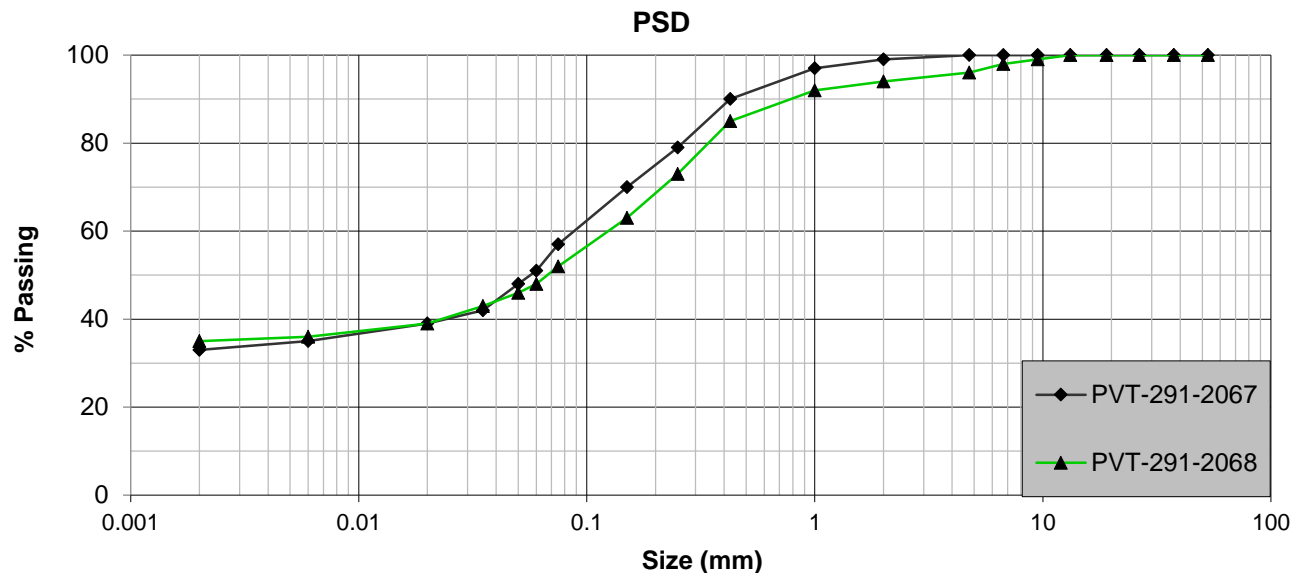
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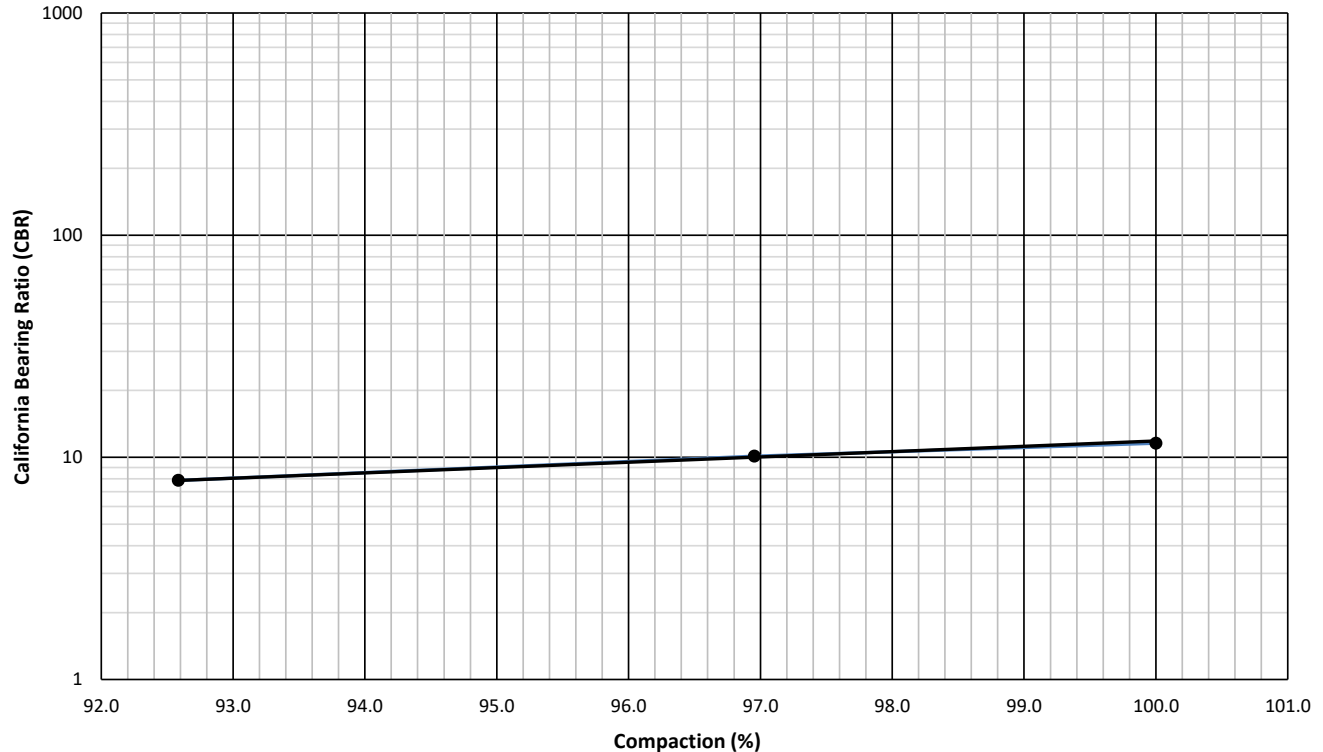
Client Name: BAV Consulting (Pty) Ltd
Project Name: Komati Power Station Repurposing
Sample: KTP1
Depth: (m) 0.7 - 2.2

Job Number: PVT-291
Lab Number: PVT-291-2026
Method: SANS 3001 GR40
Date: 26-Oct-23

CALIFORNIA BEARING RATIO

Sheet Reference:
R-STL-014 Rev01

Mod. AASHTO Values		Compaction Data: CBR			Swell	CBR at (mm)			CBR Values	
MDD	OMC	Dry Dens.	MC	Comp.		2.5	5.0	7.5	Compaction (%)	CBR
(kg/m ³)	(%)	(kg/m ³)	(%)	(%)	(%)					
2062	9.4	2036	9.6	100.0	0.1	12	12	12	100	11.6
									98	10.6
									97	10.1
2062	9.4	1974	9.6	97.0	0.2	10	10	11	95	9.0
									93	8.1
2062	9.4	1885	9.6	92.6	0.2	8	7	6	90	6.8



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Gerrie | 082 309 4448 | gerrie@stlab.co.za

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Client Name: BAV Consulting (Pty) Ltd
Project Name: Komati Power Station Repurposing
Sample: KTP1
Depth: (m) 0.7 - 2.2

Job Number: PVT-291
Lab Number: PVT-291-2026
Method: SANS 3001 GR30
Date: 26-Oct-23

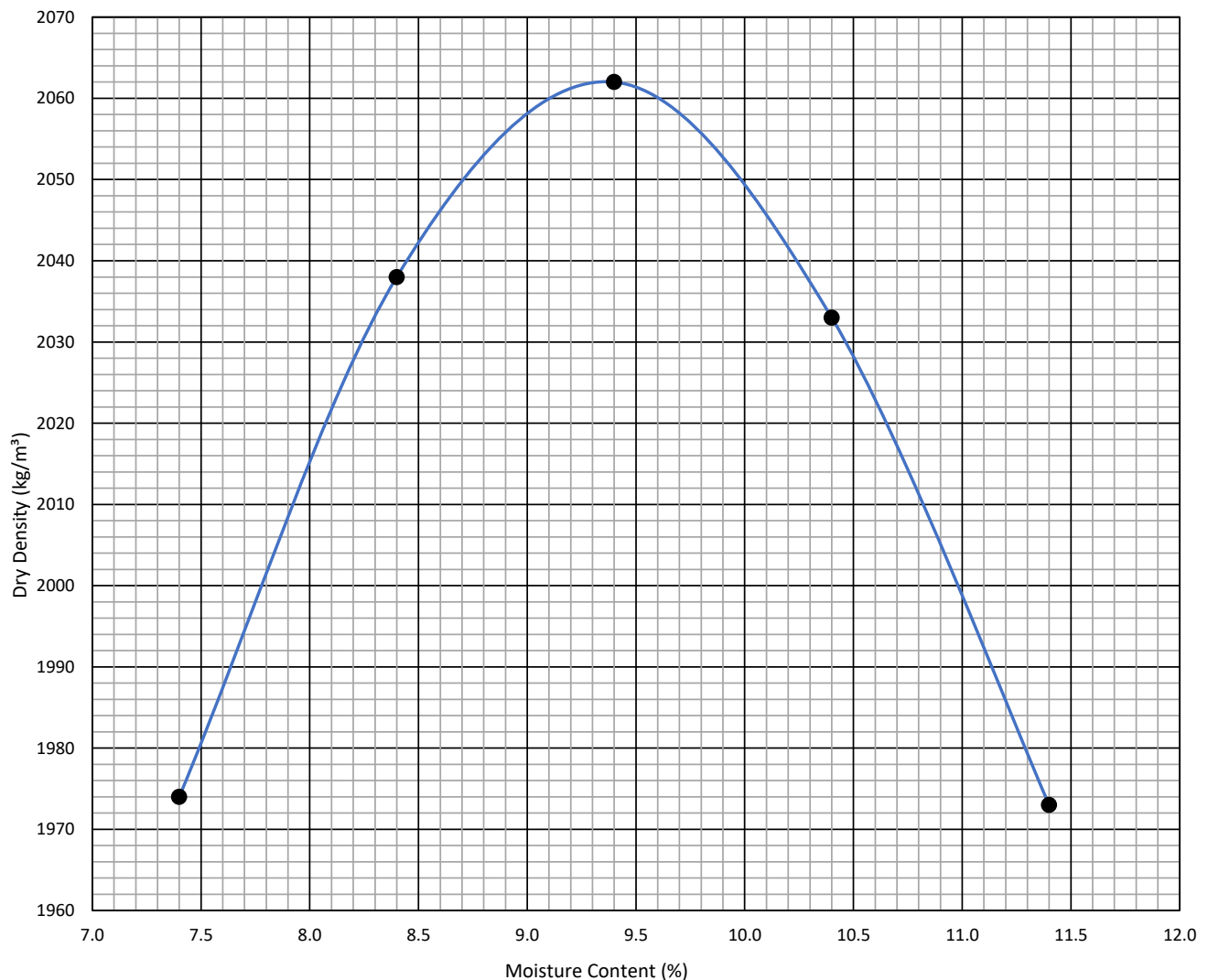
MDD & OMC DETERMINATION (Mod. AASHTO)

Sheet Reference:
R-STL-013 Rev01

Maximum Dry Density: **2062** kg/m³

Optimum Moisture Content: **9.4** %

Moisture Content (%)	7.4	8.4	9.4	10.4	11.4			
Dry Density (kg/m ³)	1974	2038	2062	2033	1973			



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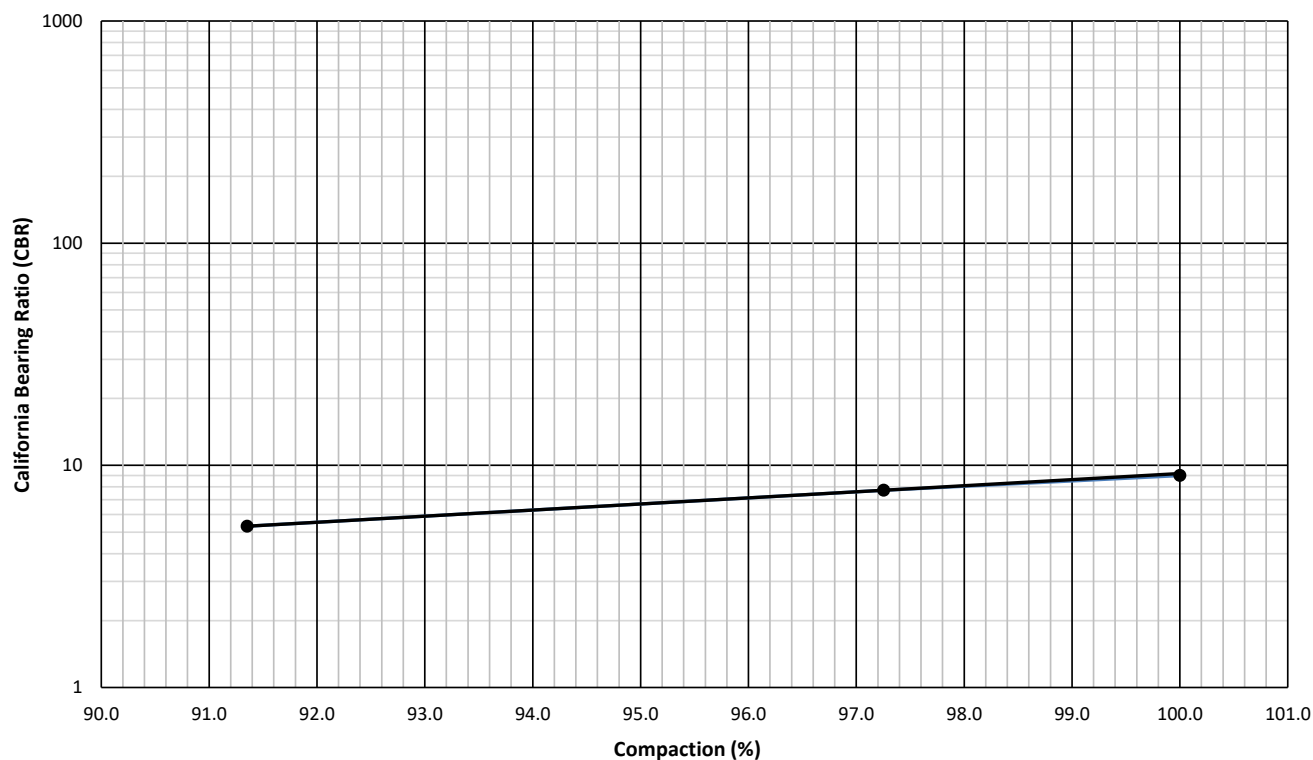
Client Name: BAV Consulting (Pty) Ltd
Project Name: Komati Power Station Repurposing
Sample: KTP2
Depth: (m) 0.8 - 2.4

Job Number: PVT-291
Lab Number: PVT-291-2027
Method: SANS 3001 GR40
Date: 26-Oct-23

CALIFORNIA BEARING RATIO

Sheet Reference:
R-STL-014 Rev01

Mod. AASHTO Values		Compaction Data: CBR			Swell	CBR at (mm)			CBR Values	
MDD	OMC	Dry Dens.	MC	Comp.		2.5	5.0	7.5	Compaction (%)	CBR
(kg/m ³)	(%)	(kg/m ³)	(%)	(%)	(%)					
1951	11.9	1966	11.9	100.0	0.2	9	7	7	100	9.0
									98	8.0
									97	7.6
1951	11.9	1912	11.9	97.3	0.3	8	7	6	95	6.7
									93	5.9
1951	11.9	1796	11.9	91.4	0.3	5	5	4	90	4.9



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Client Name: BAV Consulting (Pty) Ltd
Project Name: Komati Power Station Repurposing
Sample: KTP2
Depth: (m) 0.8 - 2.4

Job Number: PVT-291
Lab Number: PVT-291-2027
Method: SANS 3001 GR30
Date: 26-Oct-23

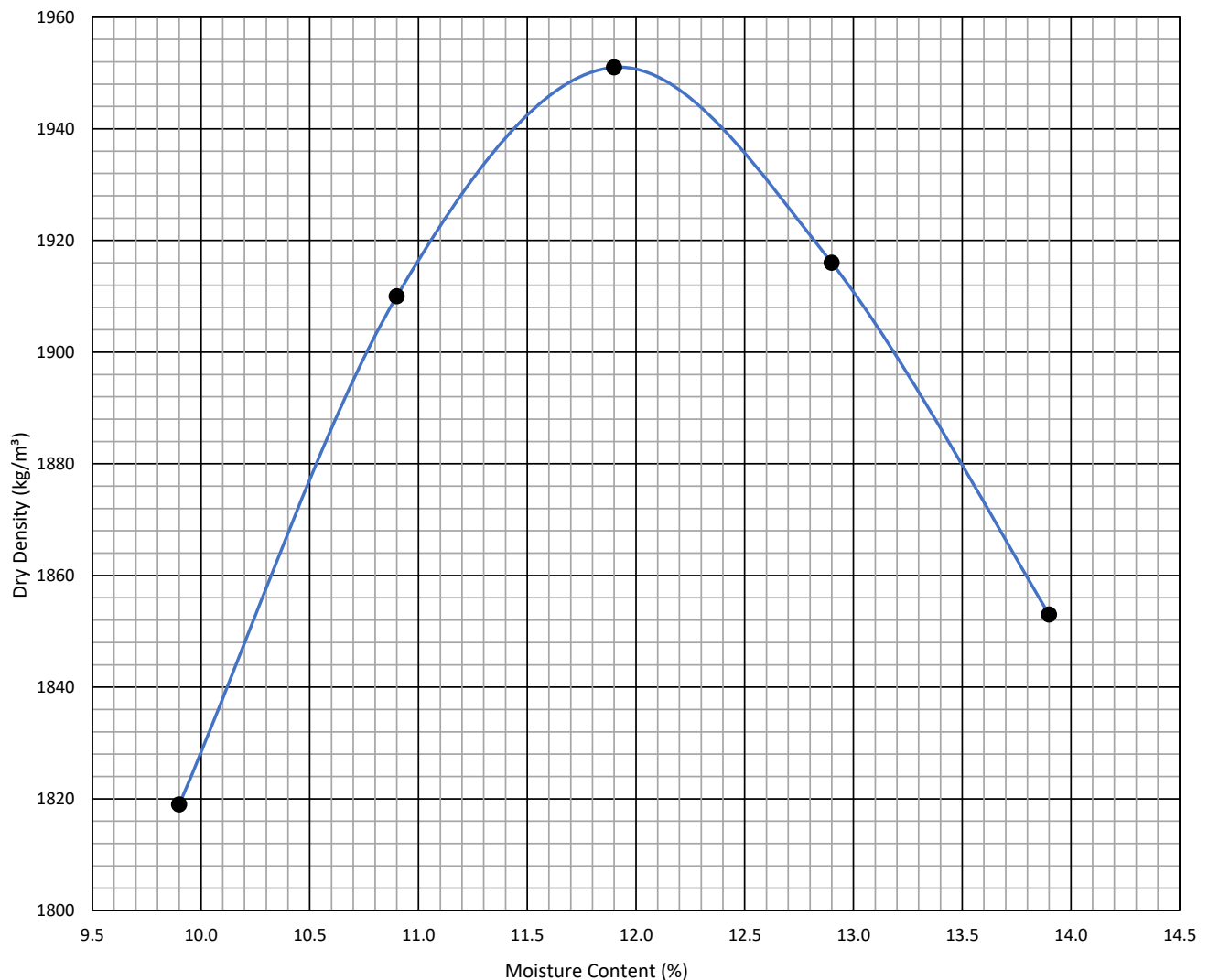
MDD & OMC DETERMINATION (Mod. AASHTO)

Sheet Reference:
R-STL-013 Rev01

Maximum Dry Density: **1951** kg/m³

Optimum Moisture Content: **11.9** %

Moisture Content (%)	9.9	10.9	11.9	12.9	13.9			
Dry Density (kg/m ³)	1819	1910	1951	1916	1853			



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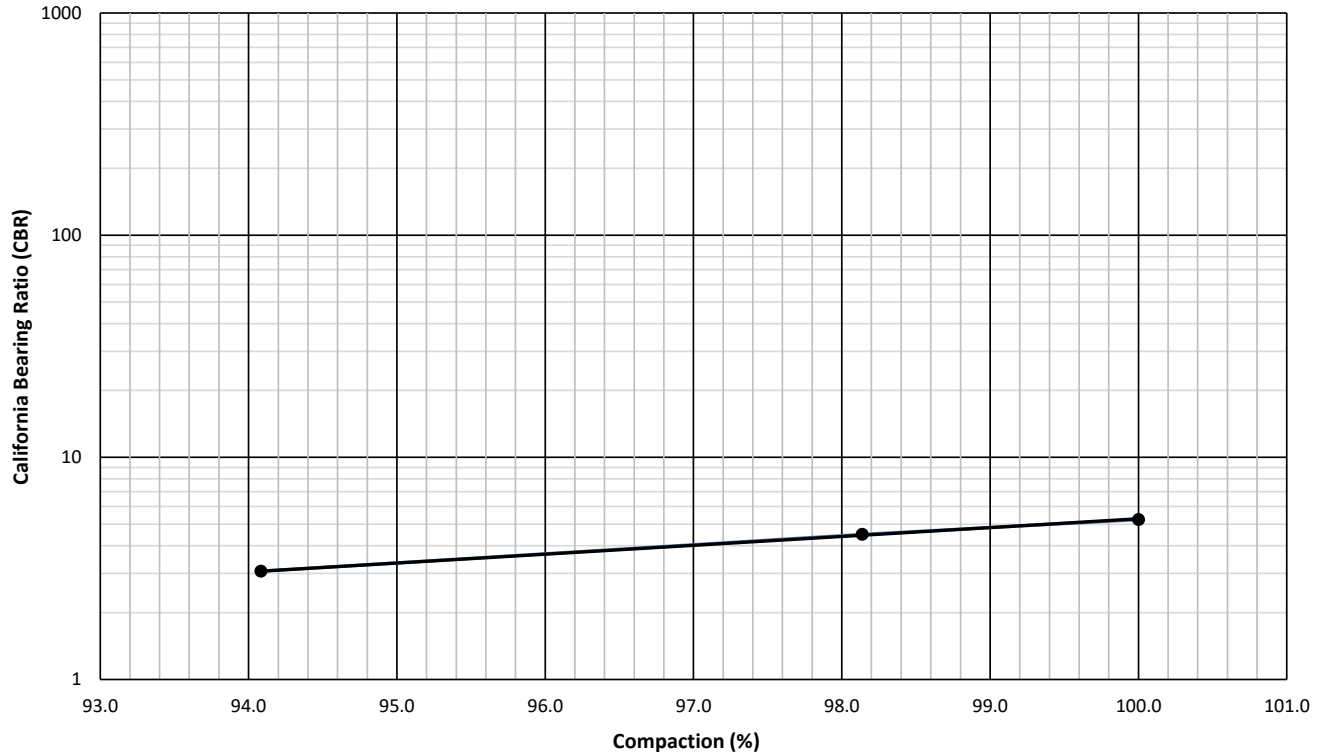
Client Name: BAV Consulting (Pty) Ltd
Project Name: Komati Power Station Repurposing
Sample: KTP4
Depth: (m) 0.3 - 2.3

Job Number: PVT-291
Lab Number: PVT-291-2029
Method: SANS 3001 GR40
Date: 26-Oct-23

CALIFORNIA BEARING RATIO

Sheet Reference:
R-STL-014 Rev01

Mod. AASHTO Values		Compaction Data: CBR			Swell	CBR at (mm)			CBR Values	
MDD	OMC	Dry Dens.	MC	Comp.		2.5	5.0	7.5	Compaction (%)	CBR
(kg/m ³)	(%)	(kg/m ³)	(%)	(%)	(%)					
1835	15.6	1825	14.7	100.0	1.0	5	4	3	100	5.2
									98	4.4
									97	4.0
1835	15.6	1791	14.7	98.1	1.1	4	3	3	95	3.3
									93	2.8
1835	15.6	1717	14.7	94.1	1.5	3	3	2	90	2.1



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Client Name: BAV Consulting (Pty) Ltd
Project Name: Komati Power Station Repurposing
Sample: KTP4
Depth: (m) 0.3 - 2.3

Job Number: PVT-291
Lab Number: PVT-291-2029
Method: SANS 3001 GR30
Date: 26-Oct-23

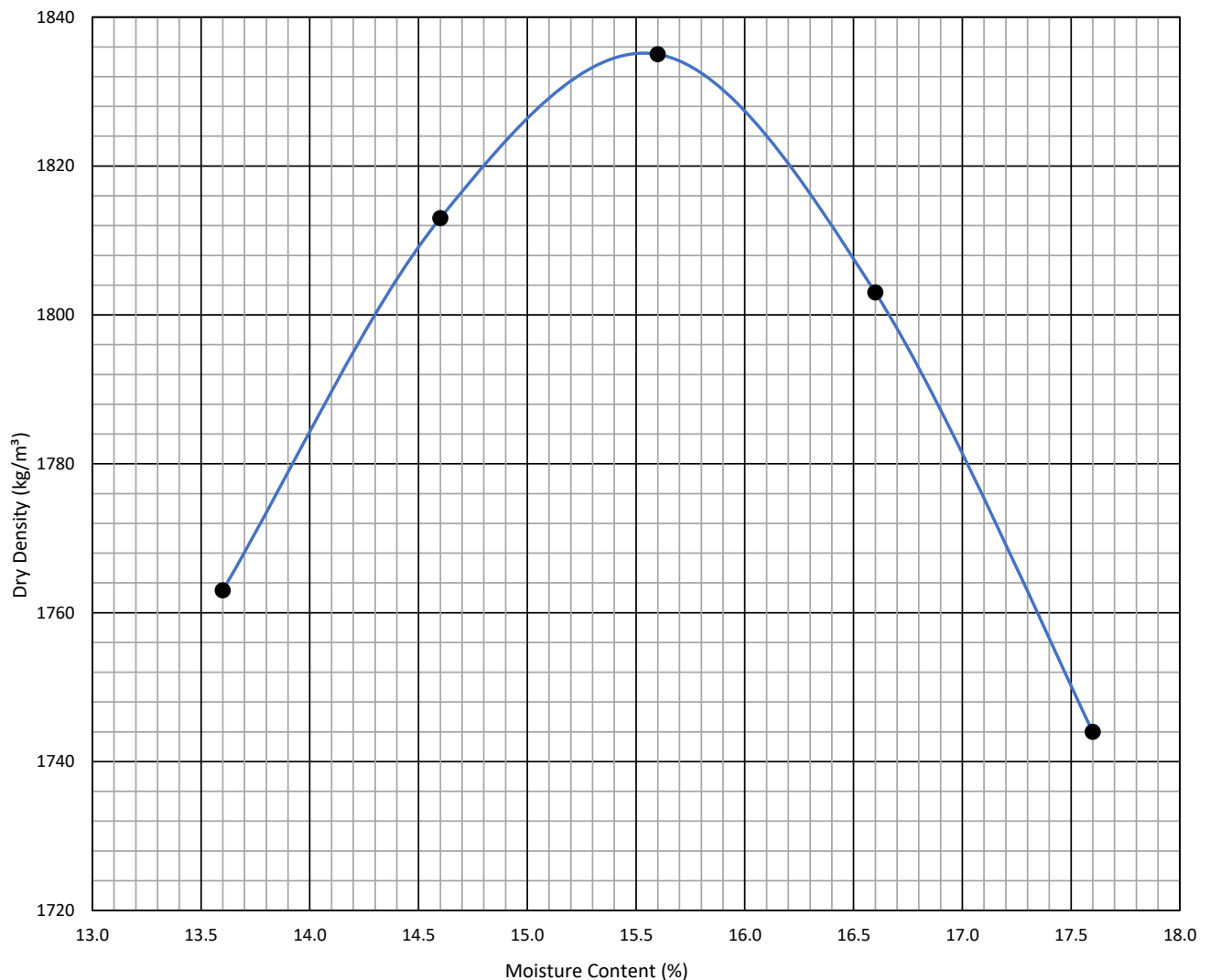
MDD & OMC DETERMINATION (Mod. AASHTO)

Sheet Reference:
R-STL-013 Rev01

Maximum Dry Density: **1835** kg/m³

Optimum Moisture Content: **15.6** %

Moisture Content (%)	13.6	14.6	15.6	16.6	17.6			
Dry Density (kg/m ³)	1763	1813	1835	1803	1744			



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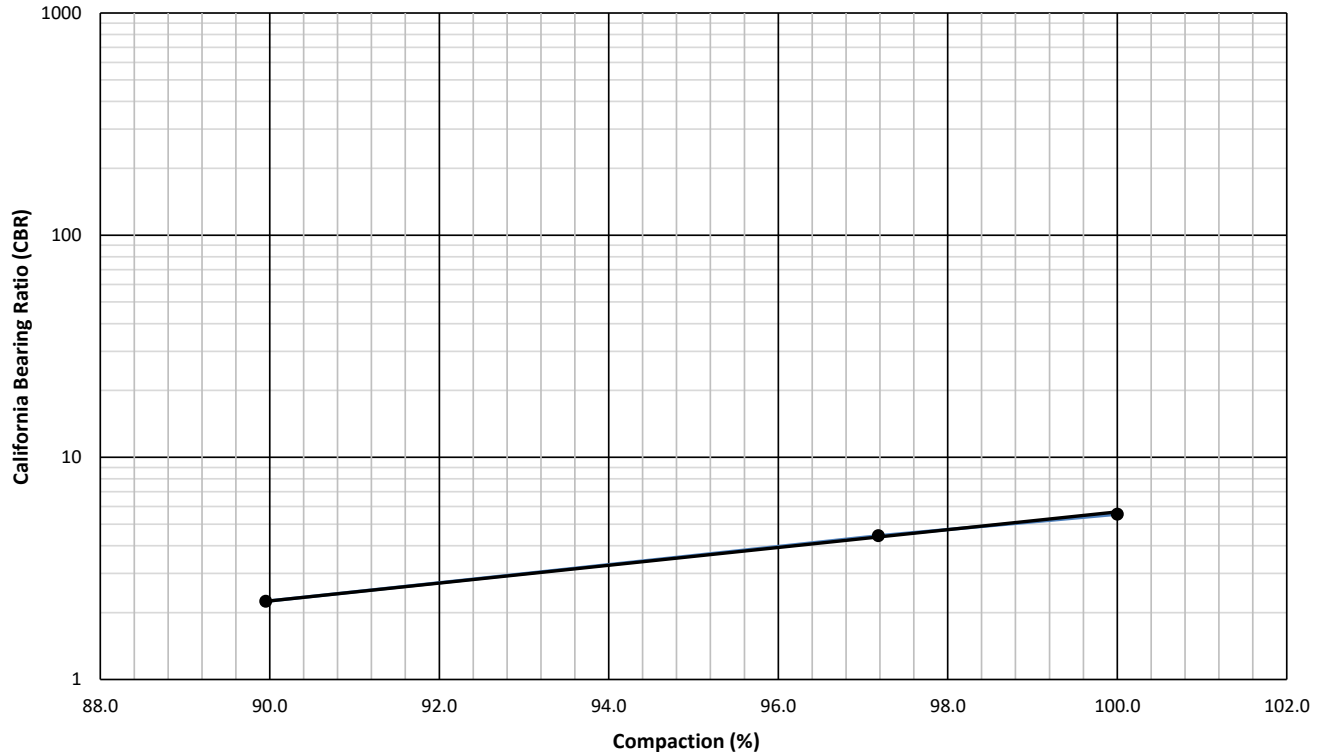
Client Name: BAV Consulting (Pty) Ltd
Project Name: Komati Power Station Repurposing
Sample: KTP5
Depth: (m) 0.4 - 2.5

Job Number: PVT-291
Lab Number: PVT-291-2030
Method: SANS 3001 GR40
Date: 26-Oct-23

CALIFORNIA BEARING RATIO

Sheet Reference:
R-STL-014 Rev01

Mod. AASHTO Values		Compaction Data: CBR			Swell	CBR at (mm)			CBR Values	
MDD	OMC	Dry Dens.	MC	Comp.		2.5	5.0	7.5	Compaction (%)	CBR
(kg/m ³)	(%)	(kg/m ³)	(%)	(%)	(%)					
1872	13.9	1881	14.1	100.0	0.5	6	5	4	100	5.5
									98	4.7
1872	13.9	1828	14.1	97.2	0.6	4	4	3	97	4.4
									95	3.6
1872	13.9	1692	14.1	90.0	1.0	2	2	2	93	3.0
									90	2.3



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Client Name: BAV Consulting (Pty) Ltd
Project Name: Komati Power Station Repurposing
Sample: KTP5
Depth: (m) 0.4 - 2.5

Job Number: PVT-291
Lab Number: PVT-291-2030
Method: SANS 3001 GR30
Date: 26-Oct-23

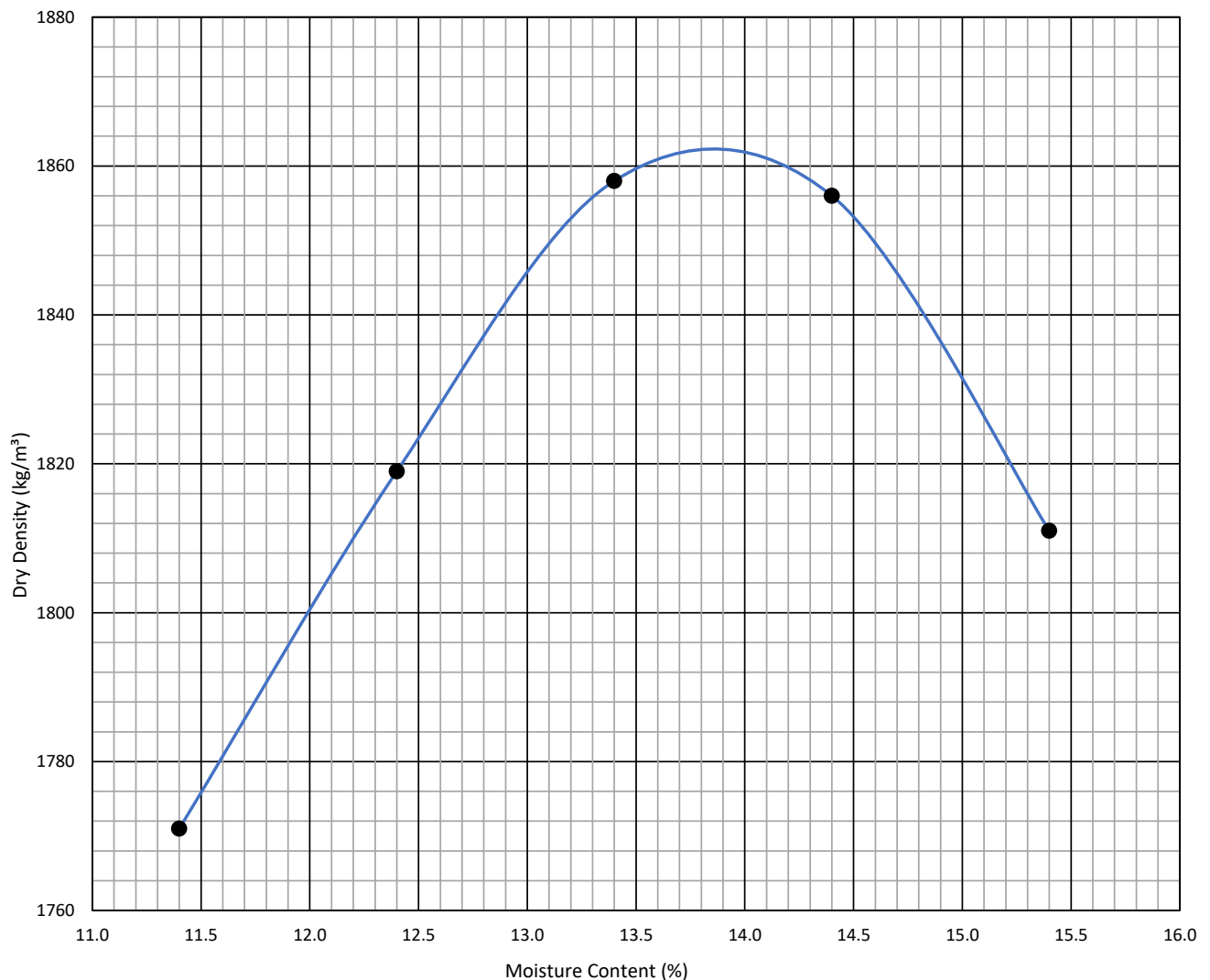
MDD & OMC DETERMINATION (Mod. AASHTO)

Sheet Reference:
R-STL-013 Rev01

Maximum Dry Density: **1872** kg/m³

Optimum Moisture Content: **13.9** %

Moisture Content (%)	11.4	12.4	13.4	14.4	15.4			
Dry Density (kg/m ³)	1771	1819	1858	1856	1811			



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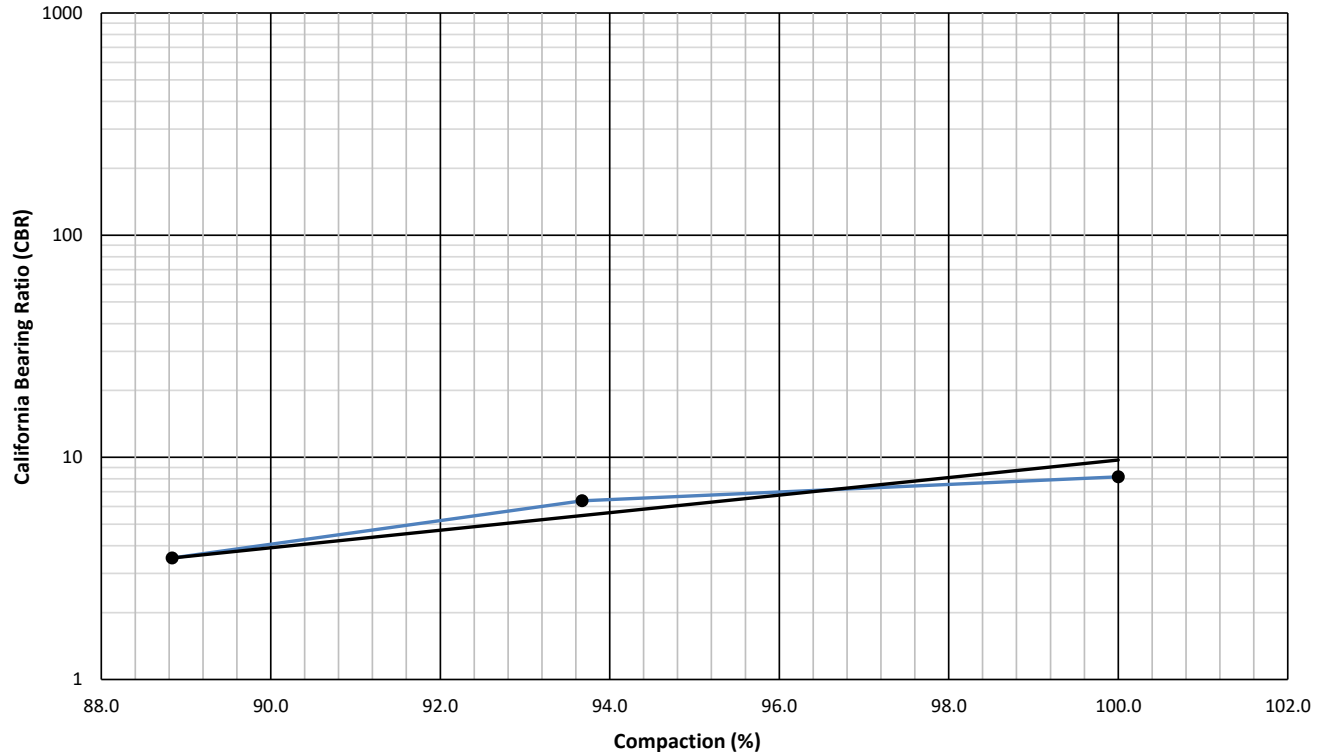
Client Name: BAV Consulting (Pty) Ltd
Project Name: Komati Power Station Repurposing
Sample: KTP13
Depth: (m) 0.3 - 1.8

Job Number: PVT-291
Lab Number: PVT-291-2036
Method: SANS 3001 GR40
Date: 26-Oct-23

CALIFORNIA BEARING RATIO

Sheet Reference:
R-STL-014 Rev01

Mod. AASHTO Values		Compaction Data: CBR			Swell	CBR at (mm)			CBR Values	
MDD	OMC	Dry Dens.	MC	Comp.		2.5	5.0	7.5	Compaction (%)	CBR
(kg/m ³)	(%)	(kg/m ³)	(%)	(%)	(%)					
1883	13.9	1881	14.6	100.0	0.3	8	7	6	100	8.2
									98	7.6
1883	13.9	1762	14.6	93.7	0.4	6	5	5	97	7.3
									95	6.7
1883	13.9	1671	14.6	88.8	0.5	4	3	3	93	5.9
									90	4.1



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Client Name: BAV Consulting (Pty) Ltd
Project Name: Komati Power Station Repurposing
Sample: KTP13
Depth: (m) 0.3 - 1.8

Job Number: PVT-291
Lab Number: PVT-291-2036
Method: SANS 3001 GR30
Date: 26-Oct-23

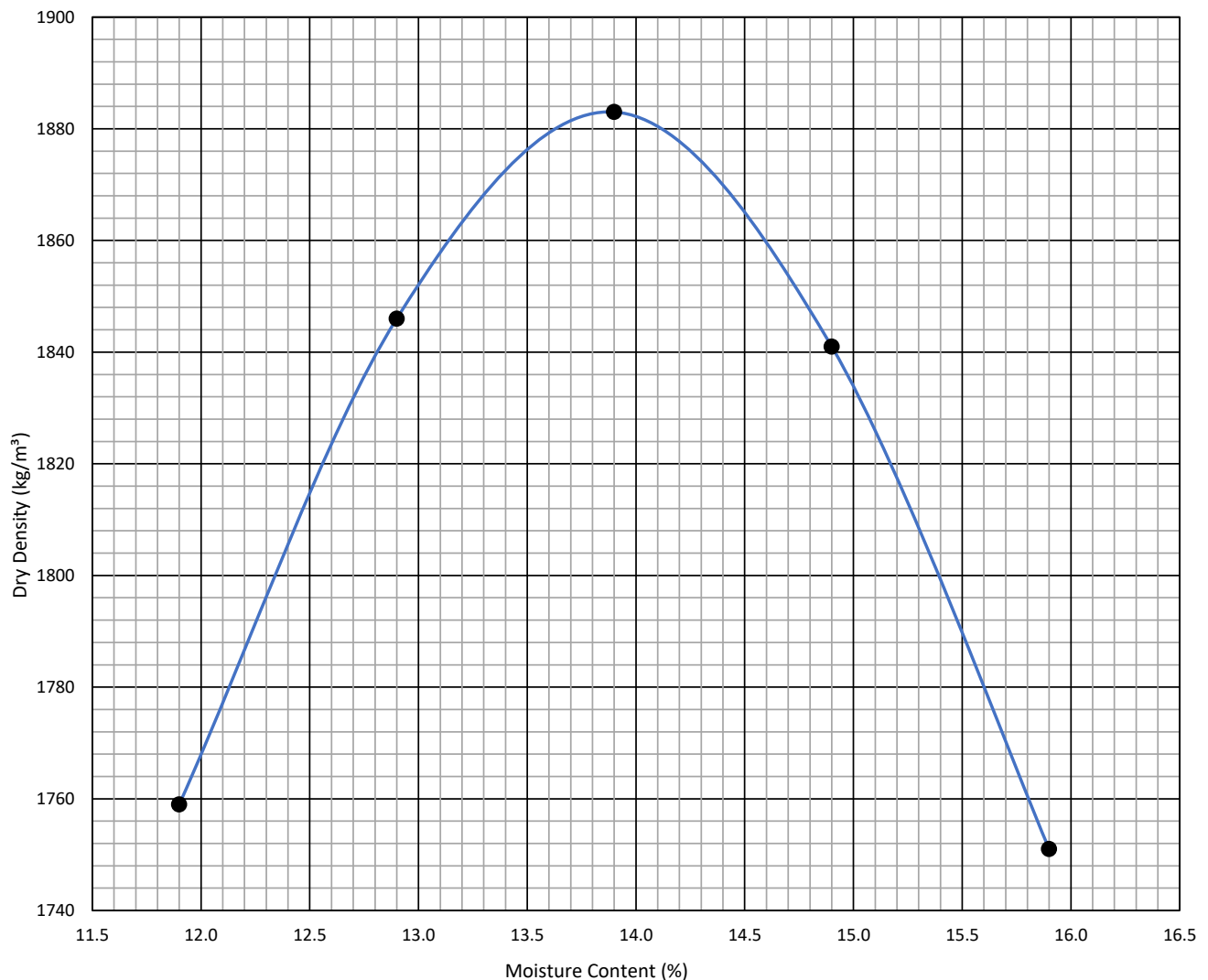
MDD & OMC DETERMINATION (Mod. AASHTO)

Sheet Reference:
R-STL-013 Rev01

Maximum Dry Density: **1883** kg/m³

Optimum Moisture Content: **13.9** %

Moisture Content (%)	11.9	12.9	13.9	14.9	15.9			
Dry Density (kg/m ³)	1759	1846	1883	1841	1751			



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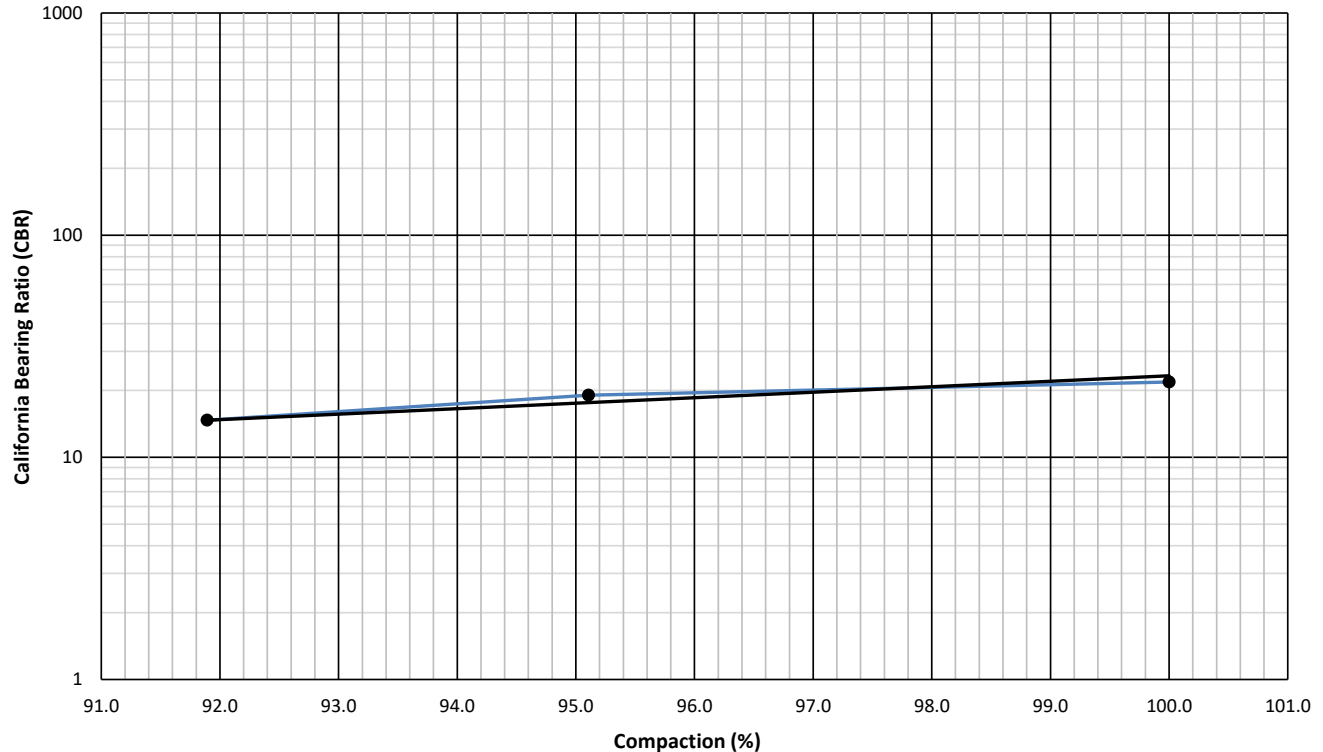
Client Name: BAV Consulting (Pty) Ltd
Project Name: Komati Power Station Repurposing
Sample: KTP16
Depth: (m) 0.3 - 2.1

Job Number: PVT-291
Lab Number: PVT-291-2038
Method: SANS 3001 GR40
Date: 26-Oct-23

CALIFORNIA BEARING RATIO

Sheet Reference:
R-STL-014 Rev01

Mod. AASHTO Values		Compaction Data: CBR			Swell	CBR at (mm)			CBR Values	
MDD	OMC	Dry Dens.	MC	Comp.		2.5	5.0	7.5	Compaction (%)	CBR
(kg/m ³)	(%)	(kg/m ³)	(%)	(%)	(%)					
2071	9.5	2084	9.9	100.0	0.3	22	27	30	100	22
									98	21
									97	20
2071	9.5	1982	9.9	95.1	0.4	19	21	20	95	19
									93	16
2071	9.5	1915	9.9	91.9	0.5	15	13	12	90	13



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Client Name: BAV Consulting (Pty) Ltd
Project Name: Komati Power Station Repurposing
Sample: KTP16
Depth: (m) 0.3 - 2.1

Job Number: PVT-291
Lab Number: PVT-291-2038
Method: SANS 3001 GR30
Date: 26-Oct-23

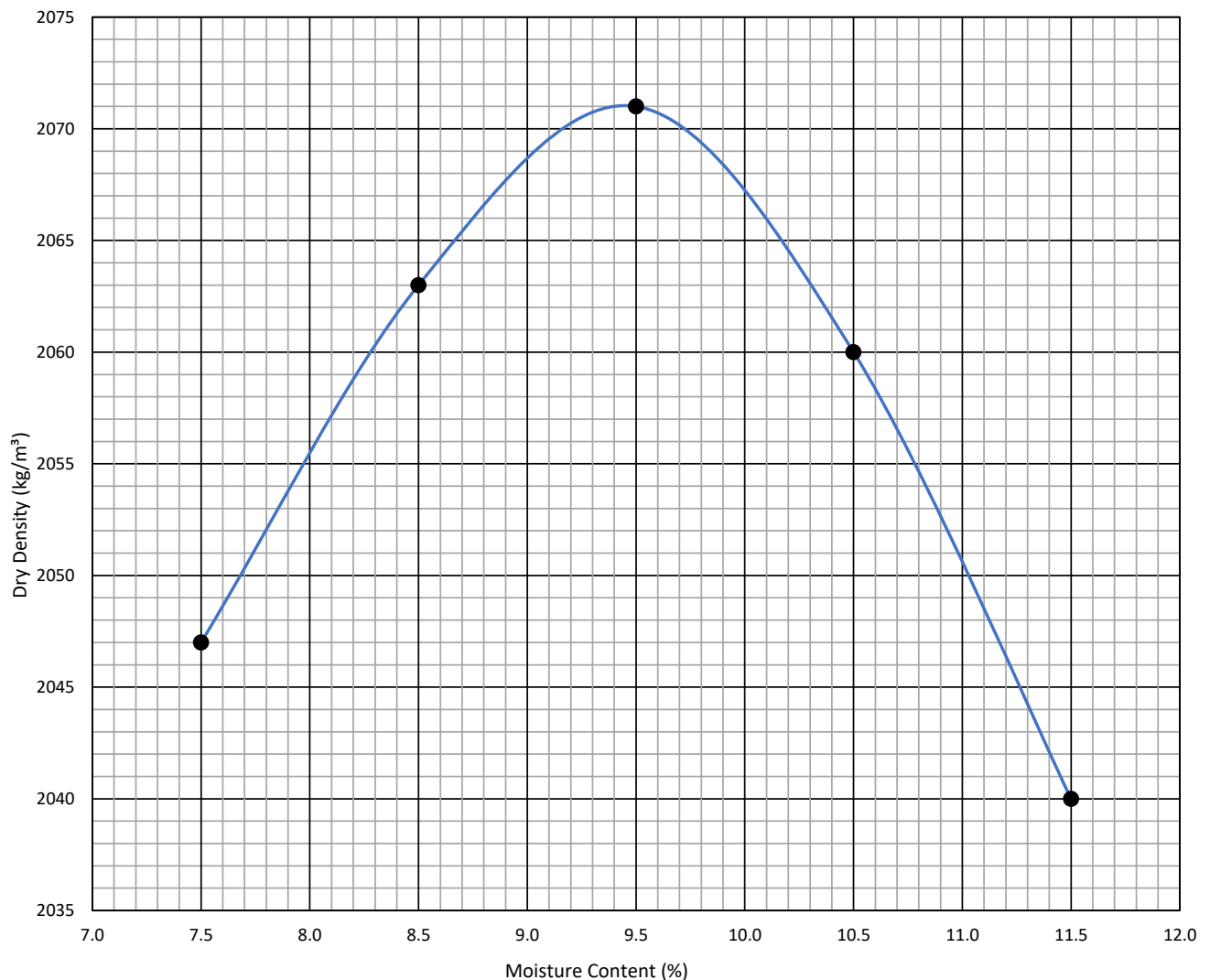
MDD & OMC DETERMINATION (Mod. AASHTO)

Sheet Reference:
R-STL-013 Rev01

Maximum Dry Density: **2071** kg/m³

Optimum Moisture Content: **9.5** %

Moisture Content (%)	7.5	8.5	9.5	10.5	11.5			
Dry Density (kg/m ³)	2047	2063	2071	2060	2040			



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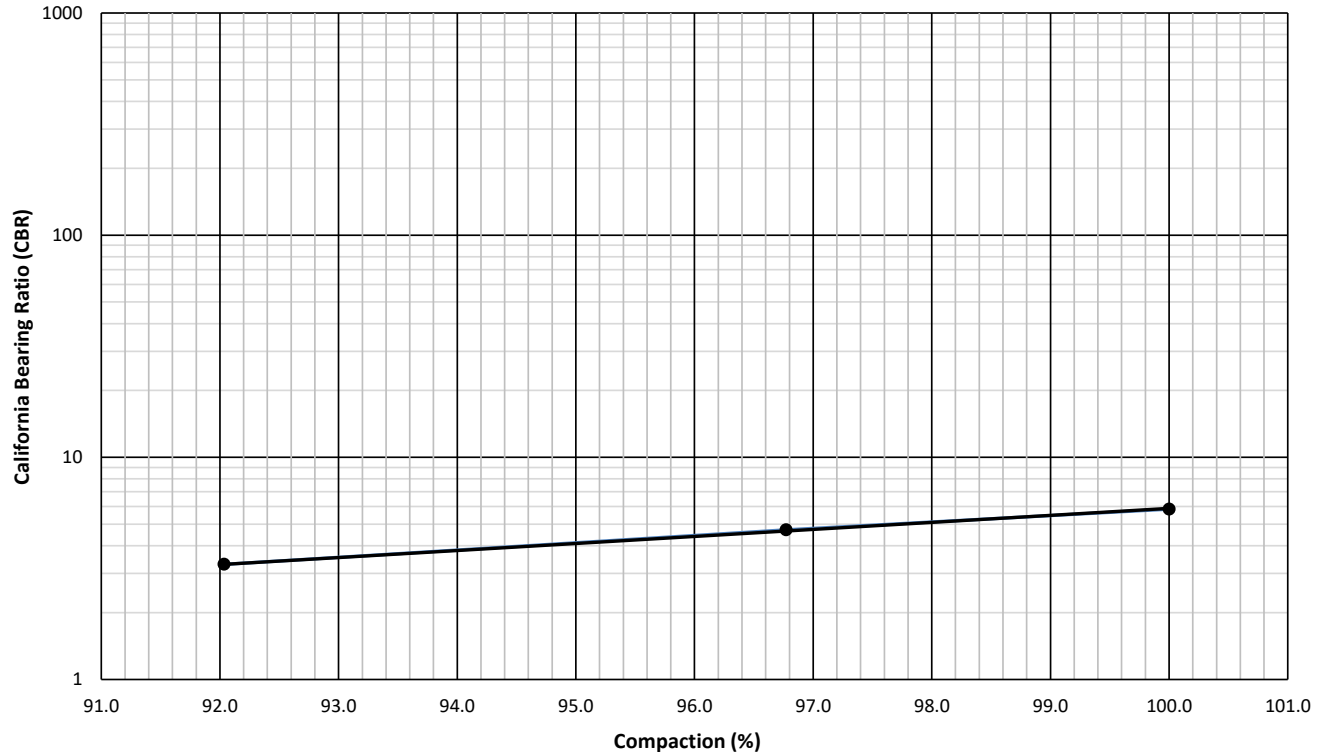
Client Name: BAV Consulting (Pty) Ltd
Project Name: Komati Power Station Repurposing
Sample: KTP20
Depth: (m) 0.1 - 2.7

Job Number: PVT-291
Lab Number: PVT-291-2041
Method: SANS 3001 GR40
Date: 26-Oct-23

CALIFORNIA BEARING RATIO

Sheet Reference:
R-STL-014 Rev01

Mod. AASHTO Values		Compaction Data: CBR			Swell	CBR at (mm)			CBR Values	
MDD	OMC	Dry Dens.	MC	Comp.		2.5	5.0	7.5	Compaction (%)	CBR
(kg/m ³)	(%)	(kg/m ³)	(%)	(%)	(%)					
1843	13.1	1858	13.6	100.0	0.6	6	6	6	100	5.8
									98	5.1
1843	13.1	1798	13.6	96.8	0.9	5	5	5	97	4.8
									95	4.1
1843	13.1	1710	13.6	92.0	1.1	3	3	3	93	3.5
									90	2.8



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Client Name: BAV Consulting (Pty) Ltd
Project Name: Komati Power Station Repurposing
Sample: KTP20
Depth: (m) 0.1 - 2.7

Job Number: PVT-291
Lab Number: PVT-291-2041
Method: SANS 3001 GR30
Date: 26-Oct-23

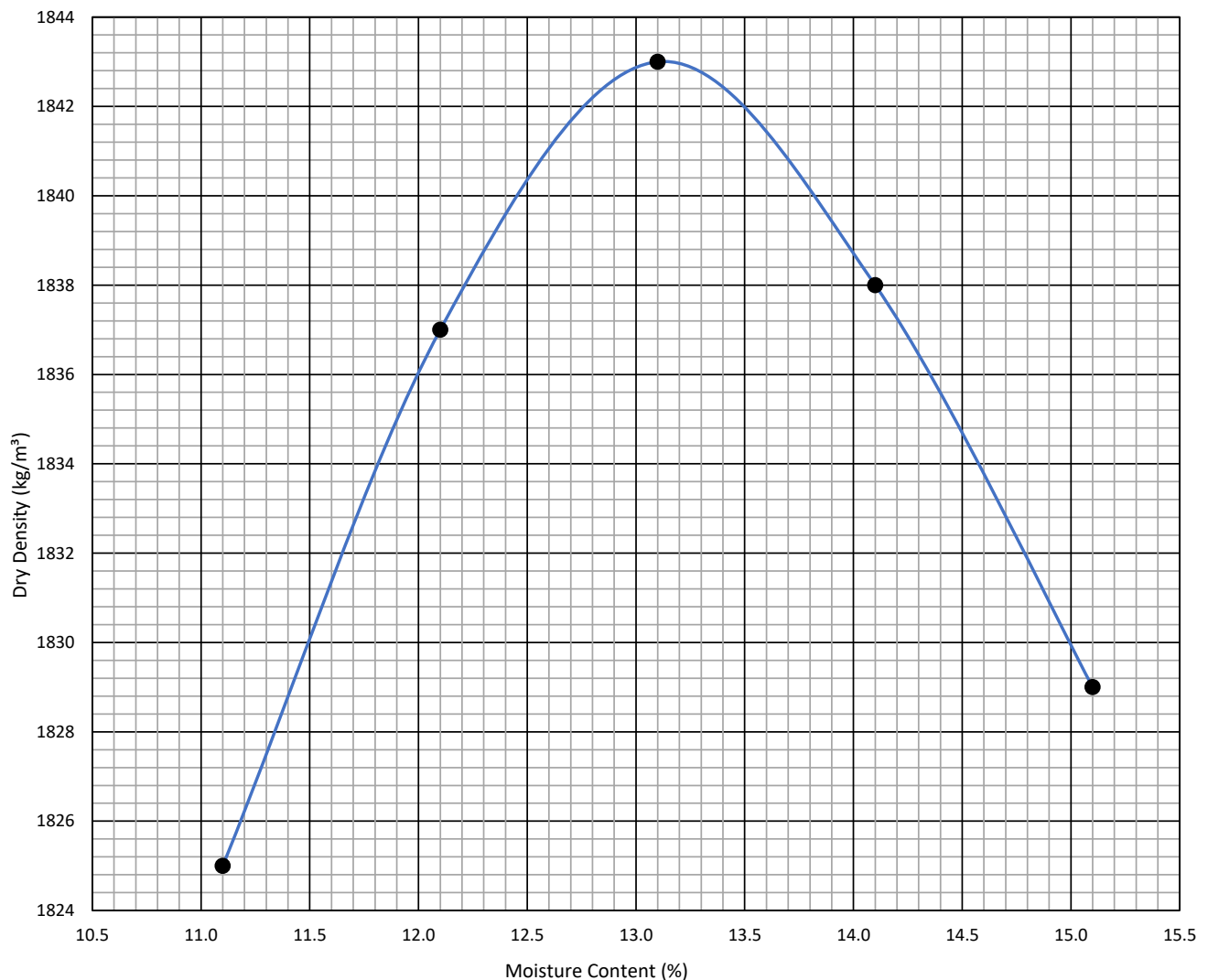
MDD & OMC DETERMINATION (Mod. AASHTO)

Sheet Reference:
R-STL-013 Rev01

Maximum Dry Density: **1843** kg/m³

Optimum Moisture Content: **13.1** %

Moisture Content (%)	11.1	12.1	13.1	14.1	15.1			
Dry Density (kg/m ³)	1825	1837	1843	1838	1829			



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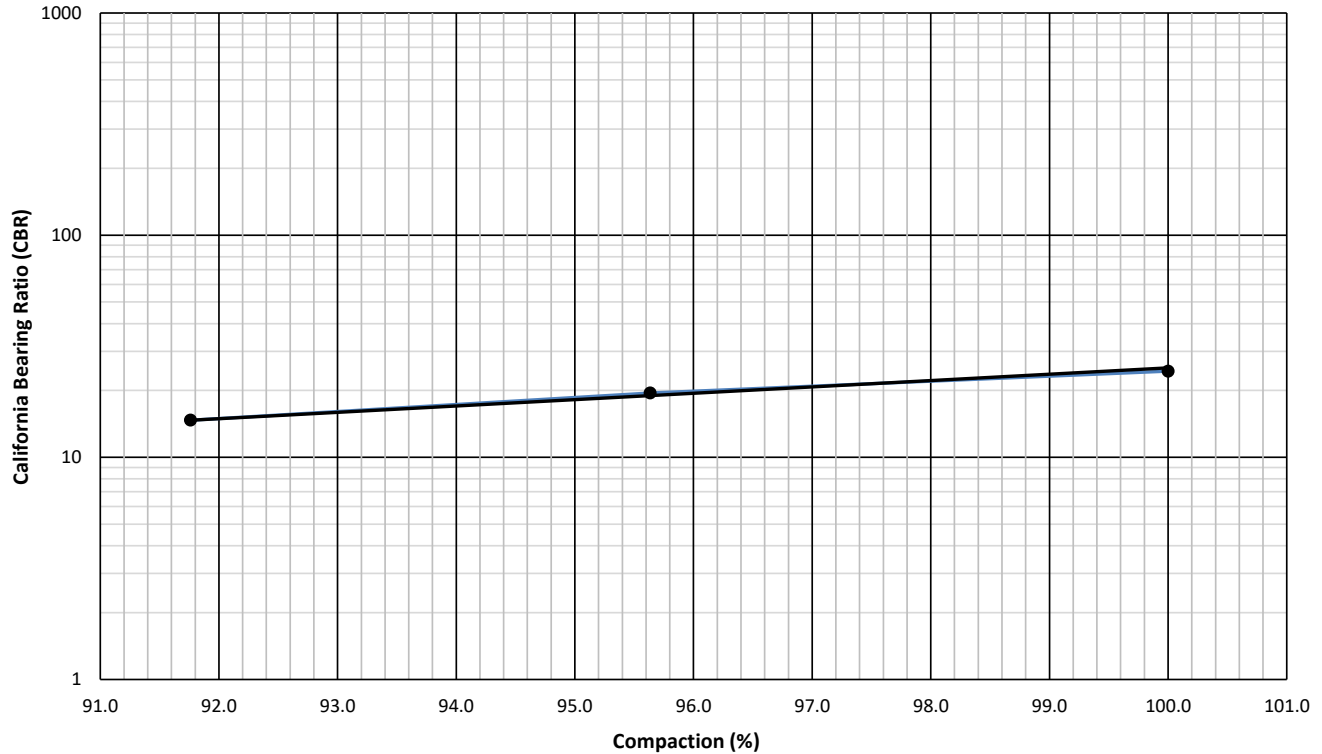
Client Name: BAV Consulting (Pty) Ltd
Project Name: Komati Power Station Repurposing
Sample: KTP21
Depth: (m) 0.1 - 2.4

Job Number: PVT-291
Lab Number: PVT-291-2042
Method: SANS 3001 GR40
Date: 26-Oct-23

CALIFORNIA BEARING RATIO

Sheet Reference:
R-STL-014 Rev01

Mod. AASHTO Values		Compaction Data: CBR			Swell (%)	CBR at (mm)			CBR Values	
MDD (kg/m³)	OMC (%)	Dry Dens. (kg/m³)	MC (%)	Comp. (%)		2.5	5.0	7.5	Compaction (%)	CBR
2095	10.7	2039	10.8	100.0	0.0	24	21	19	100	24
									98	22
									97	21
2095	10.7	1950	10.8	95.6	0.0	19	21	21	95	19
									93	16
2095	10.7	1871	10.8	91.8	0.0	15	12	10	90	13



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Gerrie | 082 309 4448 | gerrie@stlab.co.za

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Client Name: BAV Consulting (Pty) Ltd
Project Name: Komati Power Station Repurposing
Sample: KTP21
Depth: (m) 0.1 - 2.4

Job Number: PVT-291
Lab Number: PVT-291-2042
Method: SANS 3001 GR30
Date: 26-Oct-23

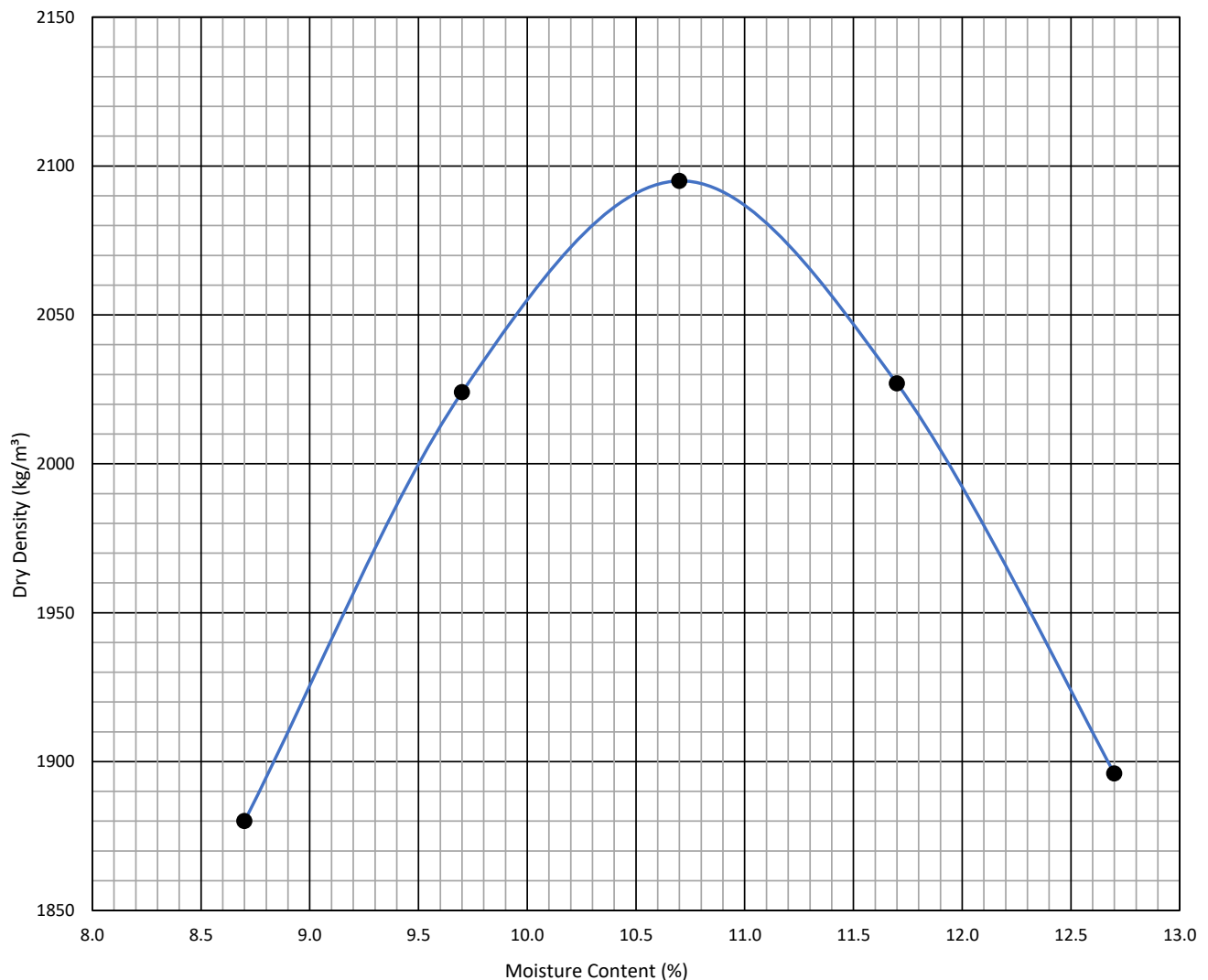
MDD & OMC DETERMINATION (Mod. AASHTO)

Sheet Reference:
R-STL-013 Rev01

Maximum Dry Density: **2095** kg/m³

Optimum Moisture Content: **10.7** %

Moisture Content (%)	8.7	9.7	10.7	11.7	12.7			
Dry Density (kg/m ³)	1880	2024	2095	2027	1896			



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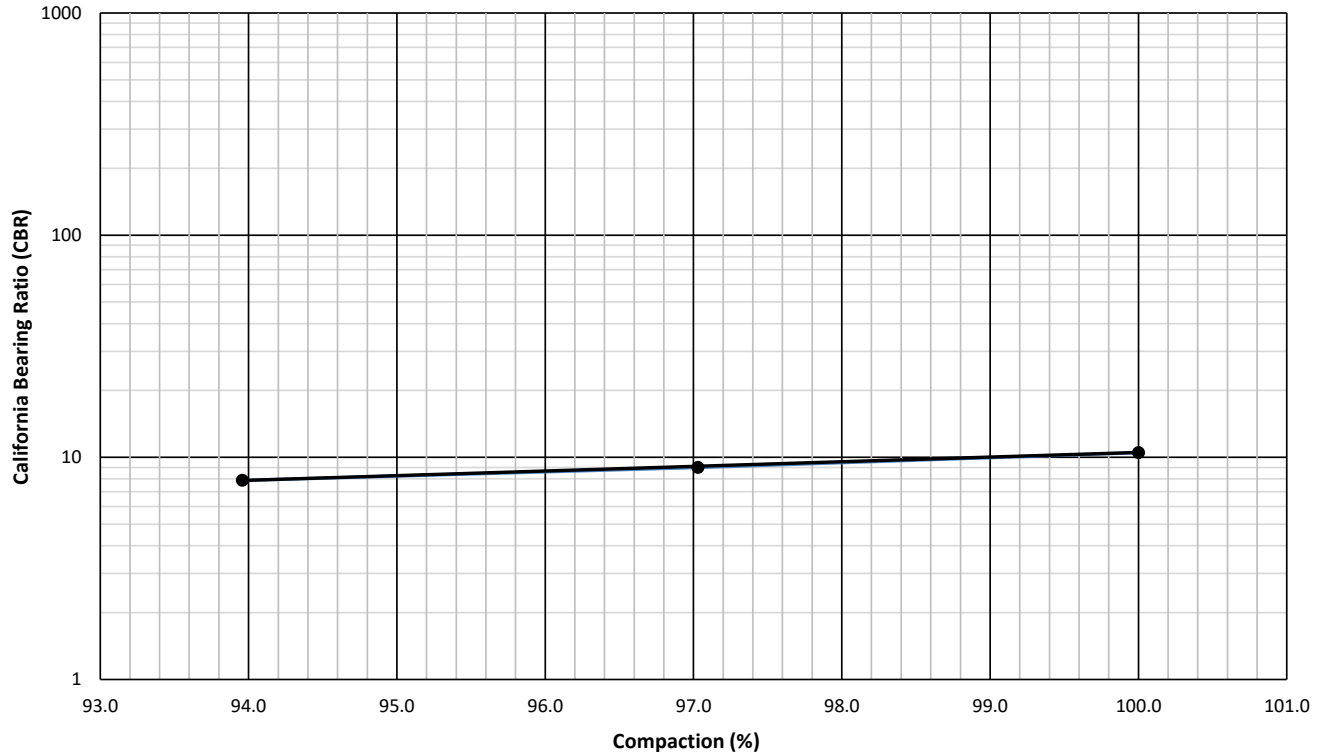
Client Name: BAV Consulting (Pty) Ltd
Project Name: Komati Power Station Repurposing
Sample: KTP23
Depth: (m) 0.3 - 2.4

Job Number: PVT-291
Lab Number: PVT-291-2044
Method: SANS 3001 GR40
Date: 26-Oct-23

CALIFORNIA BEARING RATIO

Sheet Reference:
R-STL-014 Rev01

Mod. AASHTO Values		Compaction Data: CBR			Swell	CBR at (mm)			CBR Values	
MDD	OMC	Dry Dens.	MC	Comp.		2.5	5.0	7.5	Compaction (%)	CBR
(kg/m ³)	(%)	(kg/m ³)	(%)	(%)	(%)					
1991	11.7	1986	11.6	100.0	0.2	10	9	9	100	10.5
									98	9.5
									97	9.0
1991	11.7	1927	11.6	97.0	0.2	9	9	9	95	8.2
									93	7.5
1991	11.7	1866	11.6	94.0	0.3	8	7	7	90	6.6



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Client Name: BAV Consulting (Pty) Ltd
Project Name: Komati Power Station Repurposing
Sample: KTP23
Depth: (m) 0.3 - 2.4

Job Number: PVT-291
Lab Number: PVT-291-2044
Method: SANS 3001 GR30
Date: 26-Oct-23

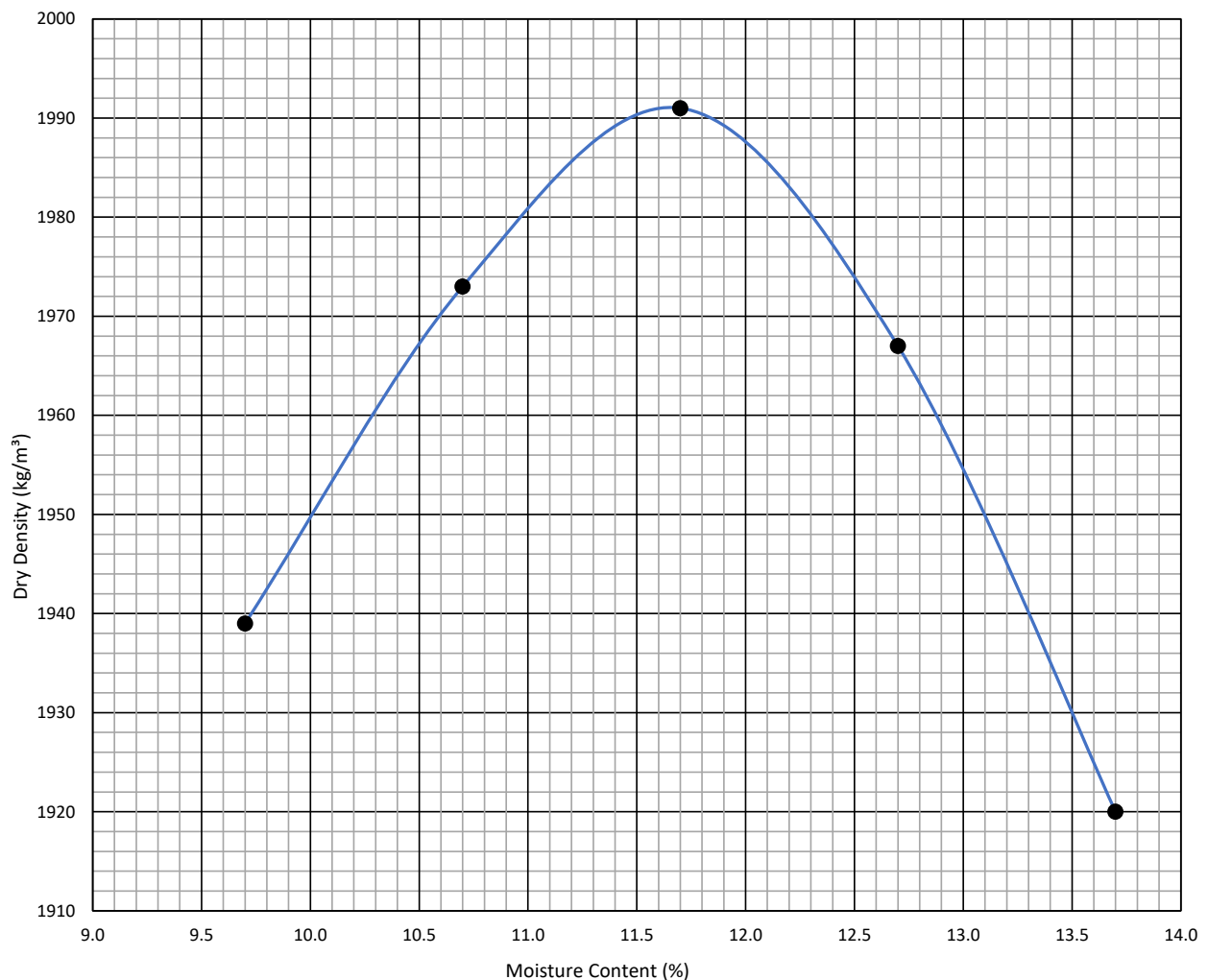
MDD & OMC DETERMINATION (Mod. AASHTO)

Sheet Reference:
R-STL-013 Rev01

Maximum Dry Density: **1991** kg/m³

Optimum Moisture Content: **11.7** %

Moisture Content (%)	9.7	10.7	11.7	12.7	13.7			
Dry Density (kg/m ³)	1939	1973	1991	1967	1920			



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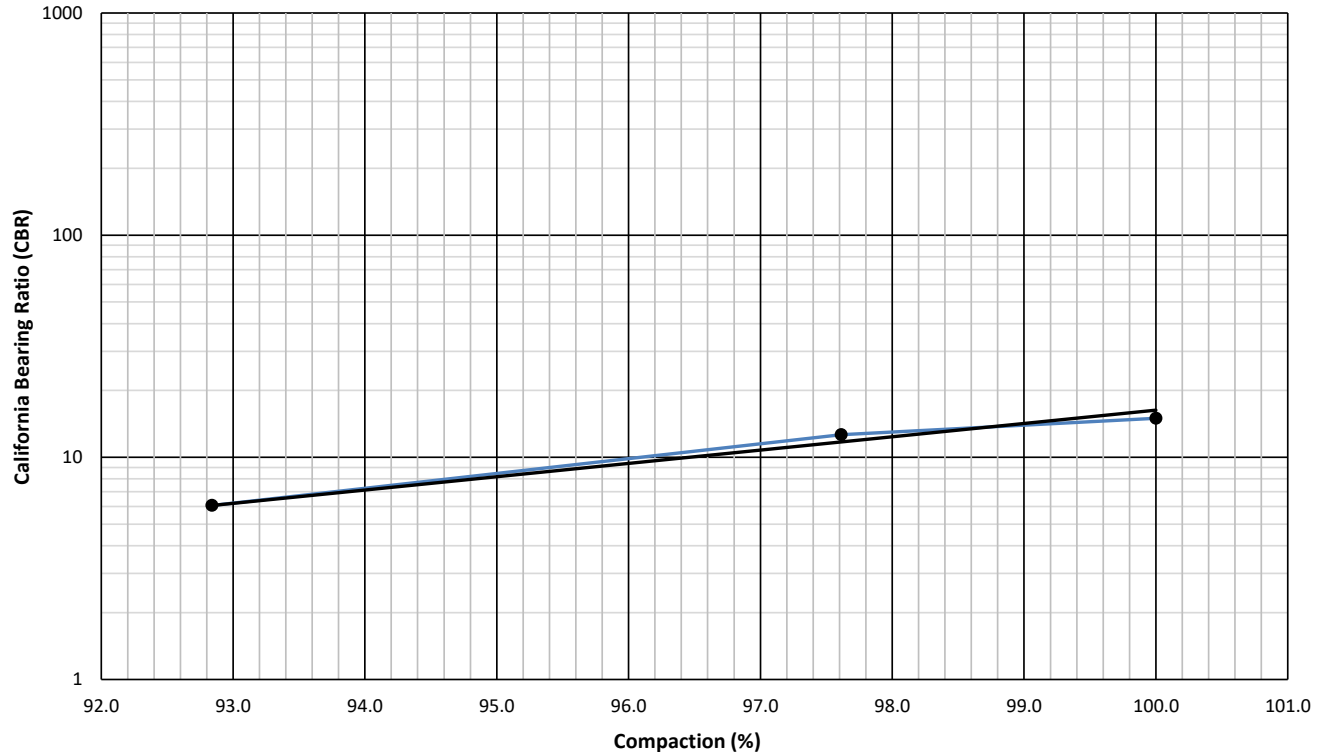
Client Name: BAV Consulting (Pty) Ltd
Project Name: Komati Power Station Repurposing
Sample: KTP25
Depth: (m) 0.4 - 2.3

Job Number: PVT-291
Lab Number: PVT-291-2046
Method: SANS 3001 GR40
Date: 26-Oct-23

CALIFORNIA BEARING RATIO

Sheet Reference:
R-STL-014 Rev01

Mod. AASHTO Values		Compaction Data: CBR			Swell	CBR at (mm)			CBR Values	
MDD	OMC	Dry Dens.	MC	Comp.		2.5	5.0	7.5	Compaction (%)	CBR
(kg/m ³)	(%)	(kg/m ³)	(%)	(%)	(%)					
2000	12.5	2053	9.9	100.0	0.0	15	15	14	100	15
									98	13
									97	12
2000	12.5	2004	9.9	97.6	0.1	13	17	20	95	9
									93	6
2000	12.5	1906	9.9	92.8	0.1	6	9	11	90	4



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Client Name: BAV Consulting (Pty) Ltd
Project Name: Komati Power Station Repurposing
Sample: KTP25
Depth: (m) 0.4 - 2.3

Job Number: PVT-291
Lab Number: PVT-291-2046
Method: SANS 3001 GR30
Date: 26-Oct-23

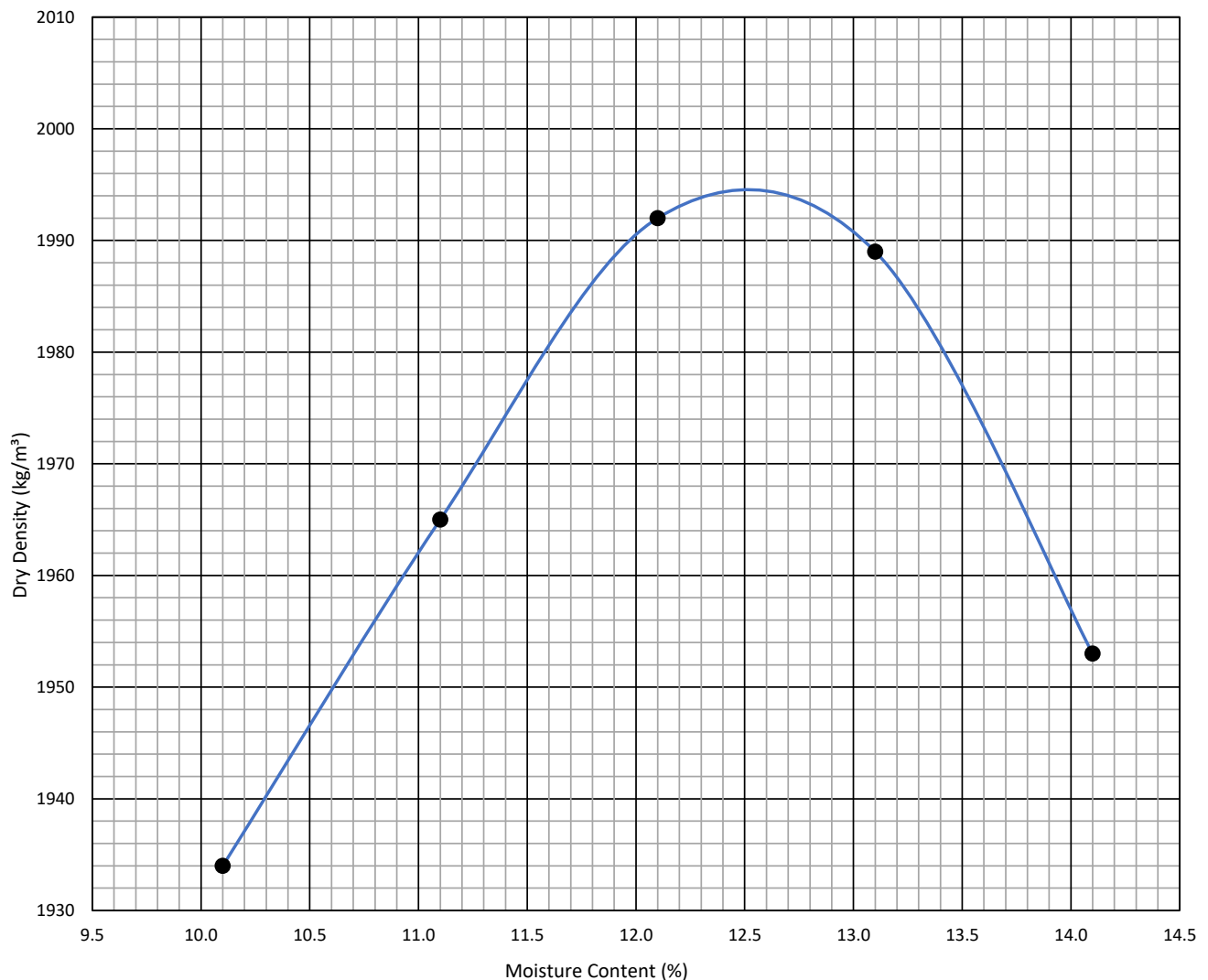
MDD & OMC DETERMINATION (Mod. AASHTO)

Sheet Reference:
R-STL-013 Rev01

Maximum Dry Density: **2000** kg/m³

Optimum Moisture Content: **12.5** %

Moisture Content (%)	10.1	11.1	12.1	13.1	14.1			
Dry Density (kg/m ³)	1934	1965	1992	1989	1953			



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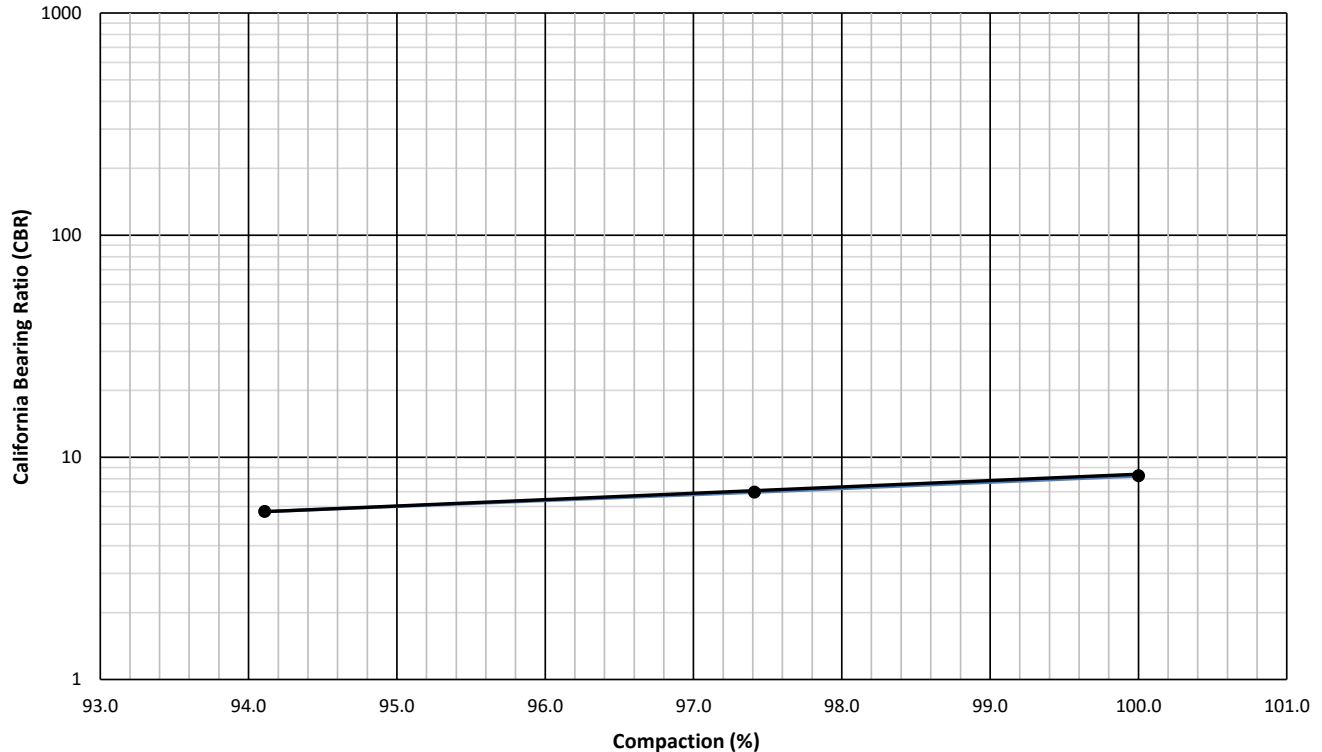
Client Name: BAV Consulting (Pty) Ltd
Project Name: Komati Power Station Repurposing
Sample: KTP26
Depth: (m) 0.6 - 2.1

Job Number: PVT-291
Lab Number: PVT-291-2047
Method: SANS 3001 GR40
Date: 26-Oct-23

CALIFORNIA BEARING RATIO

Sheet Reference:
R-STL-014 Rev01

Mod. AASHTO Values		Compaction Data: CBR			Swell	CBR at (mm)			CBR Values	
MDD	OMC	Dry Dens.	MC	Comp.		2.5	5.0	7.5	Compaction (%)	CBR
(kg/m ³)	(%)	(kg/m ³)	(%)	(%)	(%)					
1968	11.6	1969	11.9	100.0	0.1	8	7	6	100	8.2
									98	7.2
									97	6.8
1968	11.6	1918	11.9	97.4	0.2	7	7	6	95	6.0
									93	5.3
1968	11.6	1853	11.9	94.1	0.2	6	5	5	90	4.4



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Client Name: BAV Consulting (Pty) Ltd
Project Name: Komati Power Station Repurposing
Sample: KTP26
Depth: (m) 0.6 - 2.1

Job Number: PVT-291
Lab Number: PVT-291-2047
Method: SANS 3001 GR30
Date: 26-Oct-23

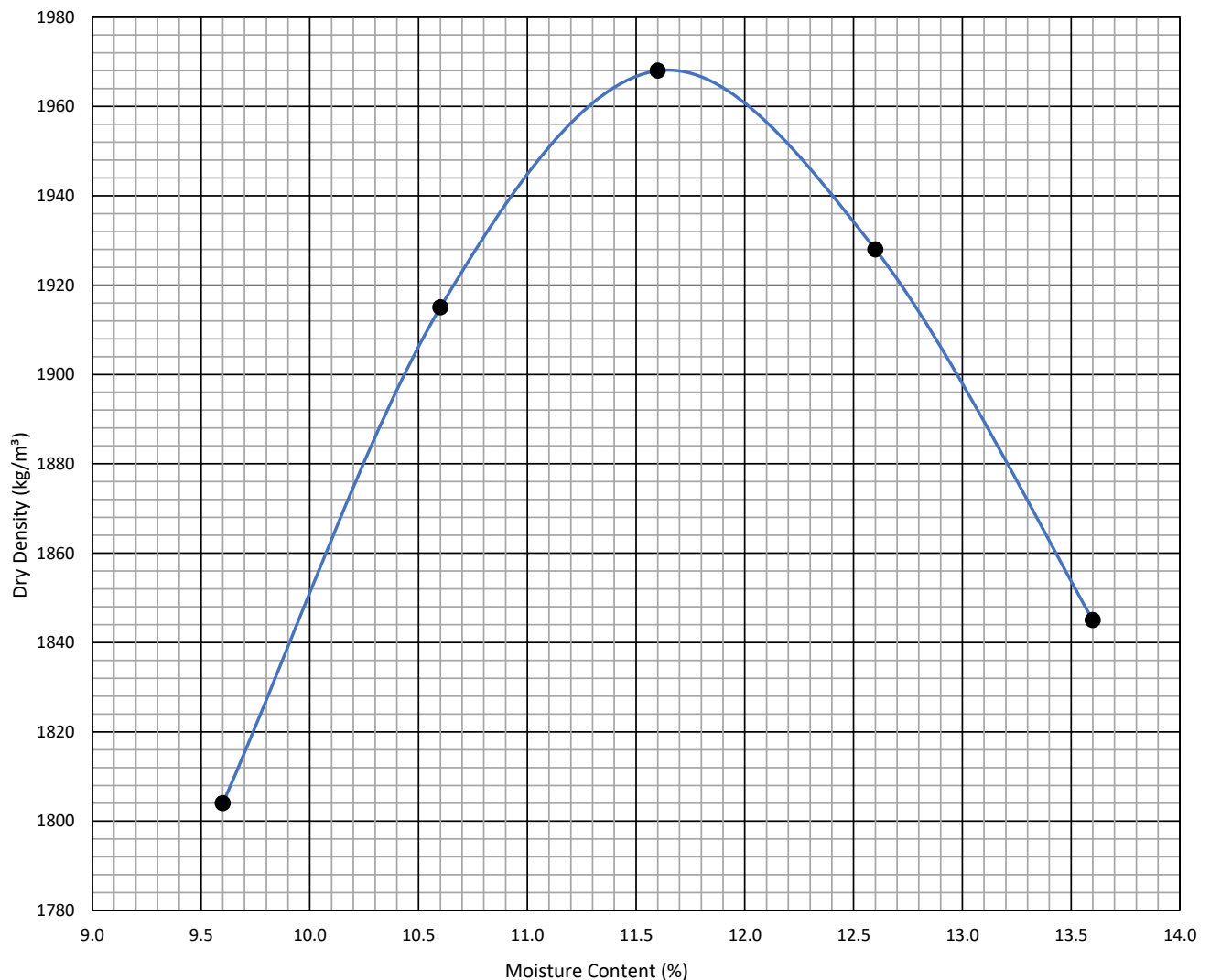
MDD & OMC DETERMINATION (Mod. AASHTO)

Sheet Reference:
R-STL-013 Rev01

Maximum Dry Density: **1968** kg/m³

Optimum Moisture Content: **11.6** %

Moisture Content (%)	9.6	10.6	11.6	12.6	13.6			
Dry Density (kg/m ³)	1804	1915	1968	1928	1845			



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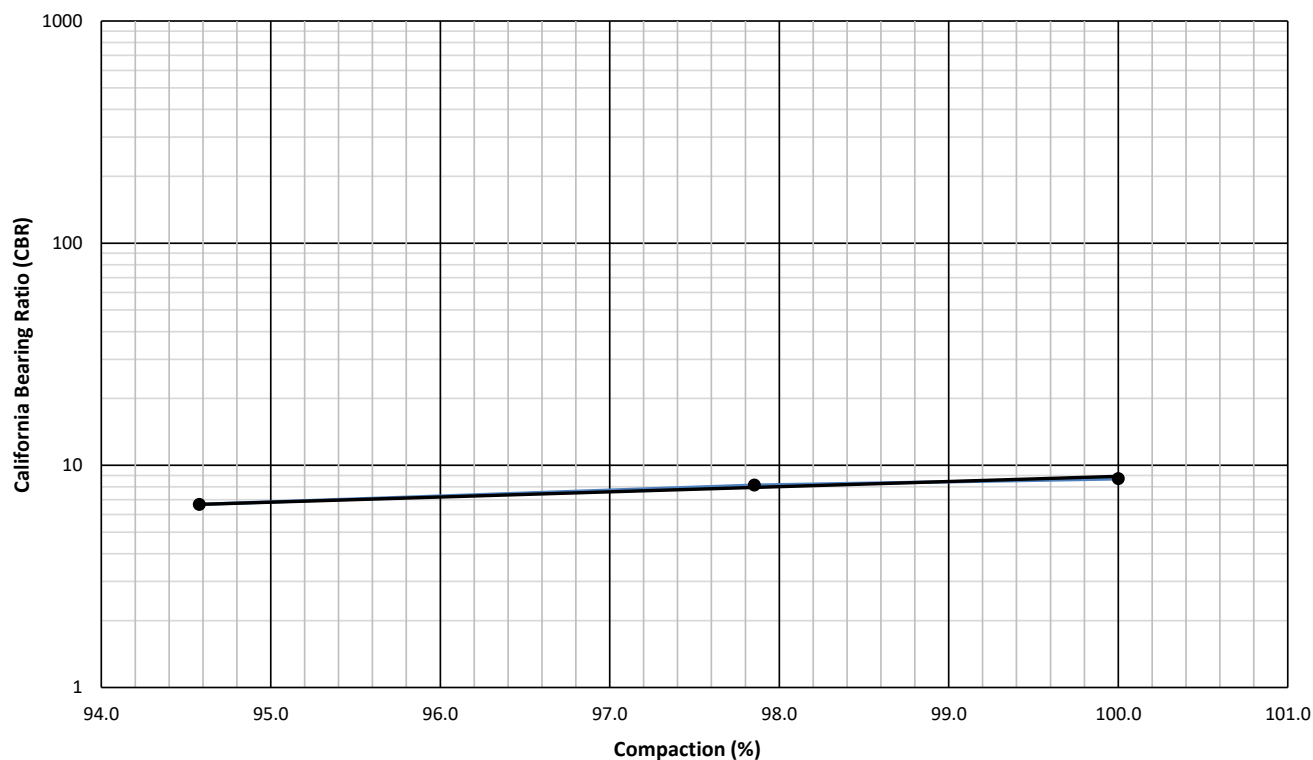
Client Name: BAV Consulting (Pty) Ltd
Project Name: Komati Power Station Repurposing
Sample: KTP29
Depth: (m) 0.5 - 2.8

Job Number: PVT-291
Lab Number: PVT-291-2050
Method: SANS 3001 GR40
Date: 26-Oct-23

CALIFORNIA BEARING RATIO

Sheet Reference:
R-STL-014 Rev01

Mod. AASHTO Values		Compaction Data: CBR			Swell	CBR at (mm)			CBR Values	
MDD	OMC	Dry Dens.	MC	Comp.		2.5	5.0	7.5	Compaction (%)	CBR
(kg/m ³)	(%)	(kg/m ³)	(%)	(%)	(%)					
1872	13.5	1863	13.8	100.0	0.0	9	8	7	100	8.7
									98	8.2
									97	7.7
1872	13.5	1823	13.8	97.9	0.1	8	7	6	95	6.8
									93	6.1
1872	13.5	1762	13.8	94.6	0.1	7	6	6	90	5.1



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Client Name: BAV Consulting (Pty) Ltd
Project Name: Komati Power Station Repurposing
Sample: KTP29
Depth: (m) 0.5 - 2.8

Job Number: PVT-291
Lab Number: PVT-291-2050
Method: SANS 3001 GR30
Date: 26-Oct-23

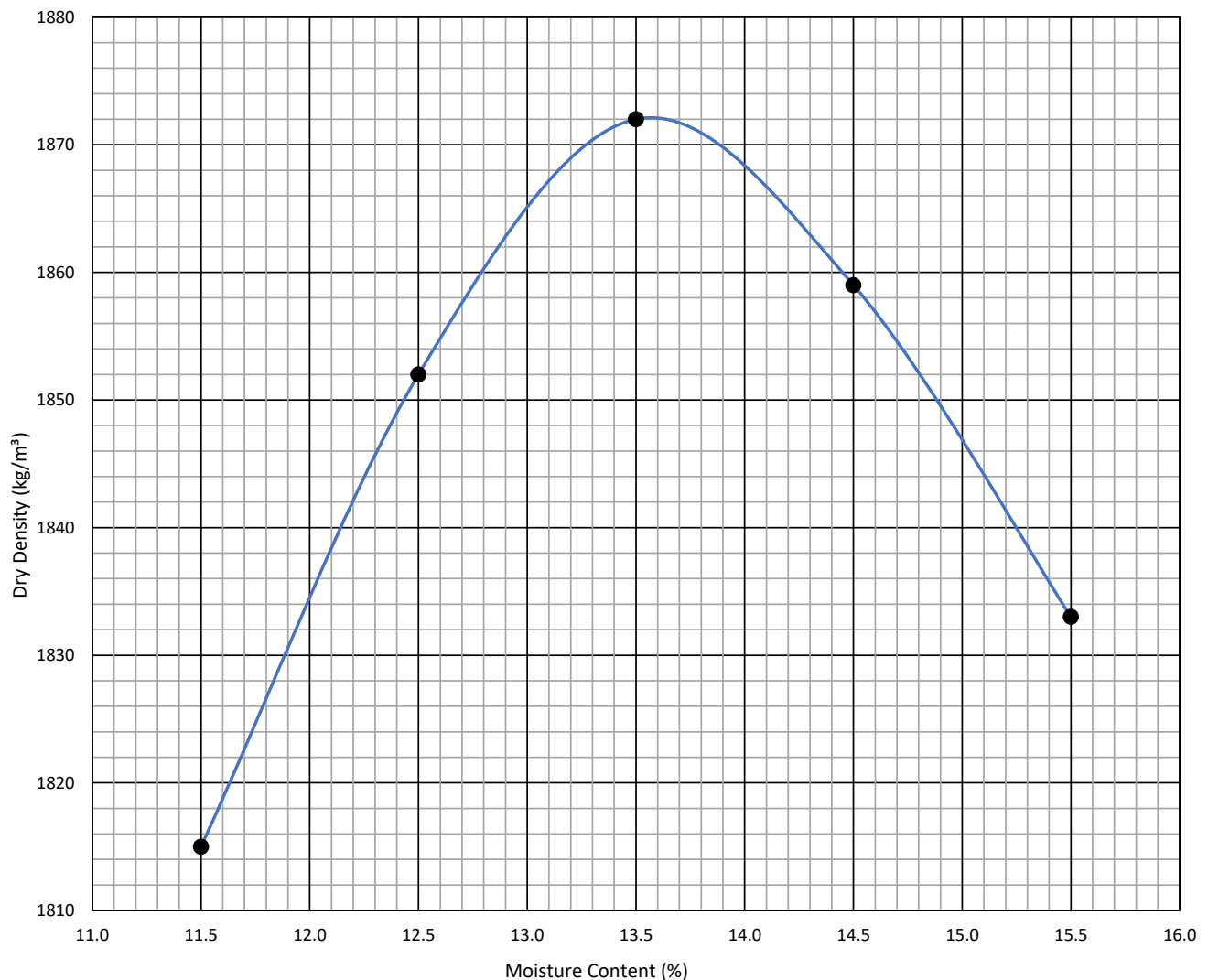
MDD & OMC DETERMINATION (Mod. AASHTO)

Sheet Reference:
R-STL-013 Rev01

Maximum Dry Density: **1872** kg/m³

Optimum Moisture Content: **13.5** %

Moisture Content (%)	11.5	12.5	13.5	14.5	15.5			
Dry Density (kg/m ³)	1815	1852	1872	1859	1833			



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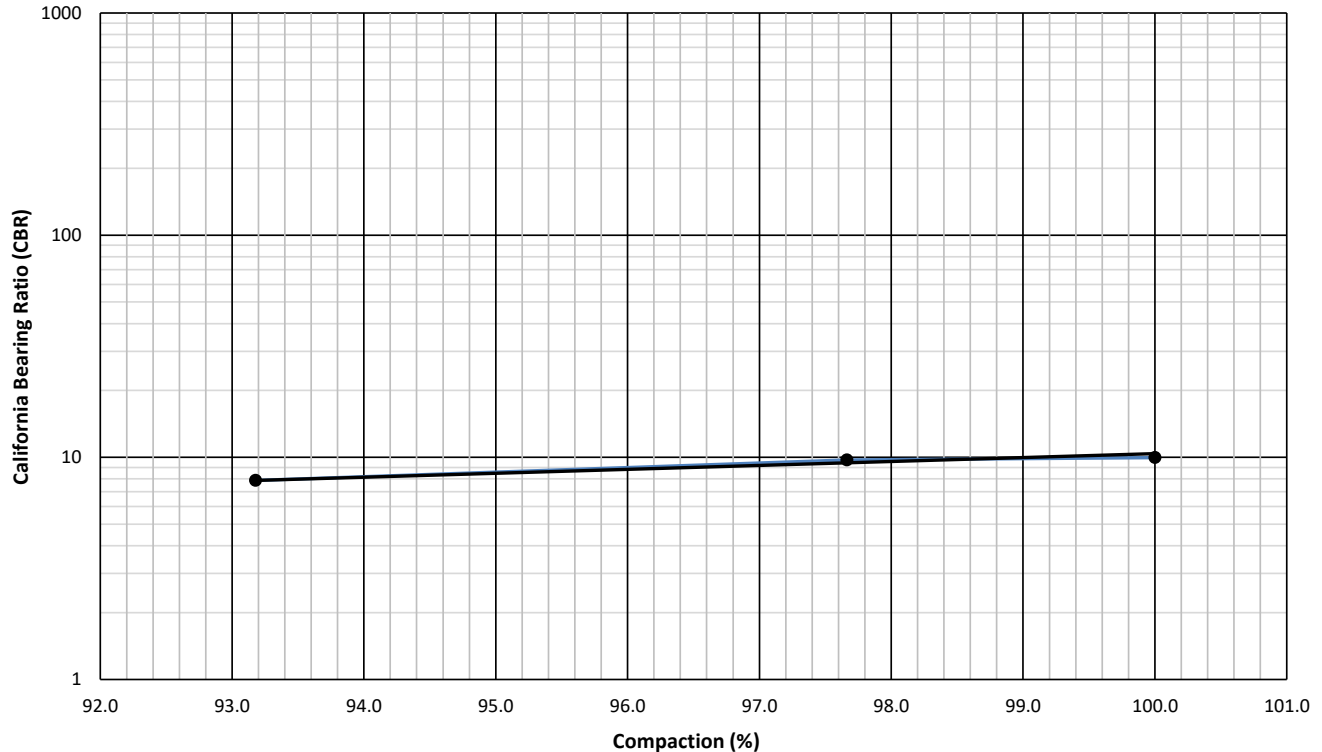
Client Name: BAV Consulting (Pty) Ltd
Project Name: Komati Power Station Repurposing
Sample: KTP30
Depth: (m) 0.2 - 2.4

Job Number: PVT-291
Lab Number: PVT-291-2051
Method: SANS 3001 GR40
Date: 26-Oct-23

CALIFORNIA BEARING RATIO

Sheet Reference:
R-STL-014 Rev01

Mod. AASHTO Values		Compaction Data: CBR			Swell	CBR at (mm)			CBR Values	
MDD	OMC	Dry Dens.	MC	Comp.		2.5	5.0	7.5	Compaction (%)	CBR
(kg/m ³)	(%)	(kg/m ³)	(%)	(%)	(%)					
1675	18.9	1627	19.9	100.0	0.4	10	10	9	100	10.0
									98	9.8
									97	9.4
1675	18.9	1589	19.9	97.7	0.5	10	10	10	95	8.6
									93	7.8
1675	18.9	1516	19.9	93.2	0.7	8	8	8	90	6.8



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Client Name: BAV Consulting (Pty) Ltd
Project Name: Komati Power Station Repurposing
Sample: KTP30
Depth: (m) 0.2 - 2.4

Job Number: PVT-291
Lab Number: PVT-291-2051
Method: SANS 3001 GR30
Date: 26-Oct-23

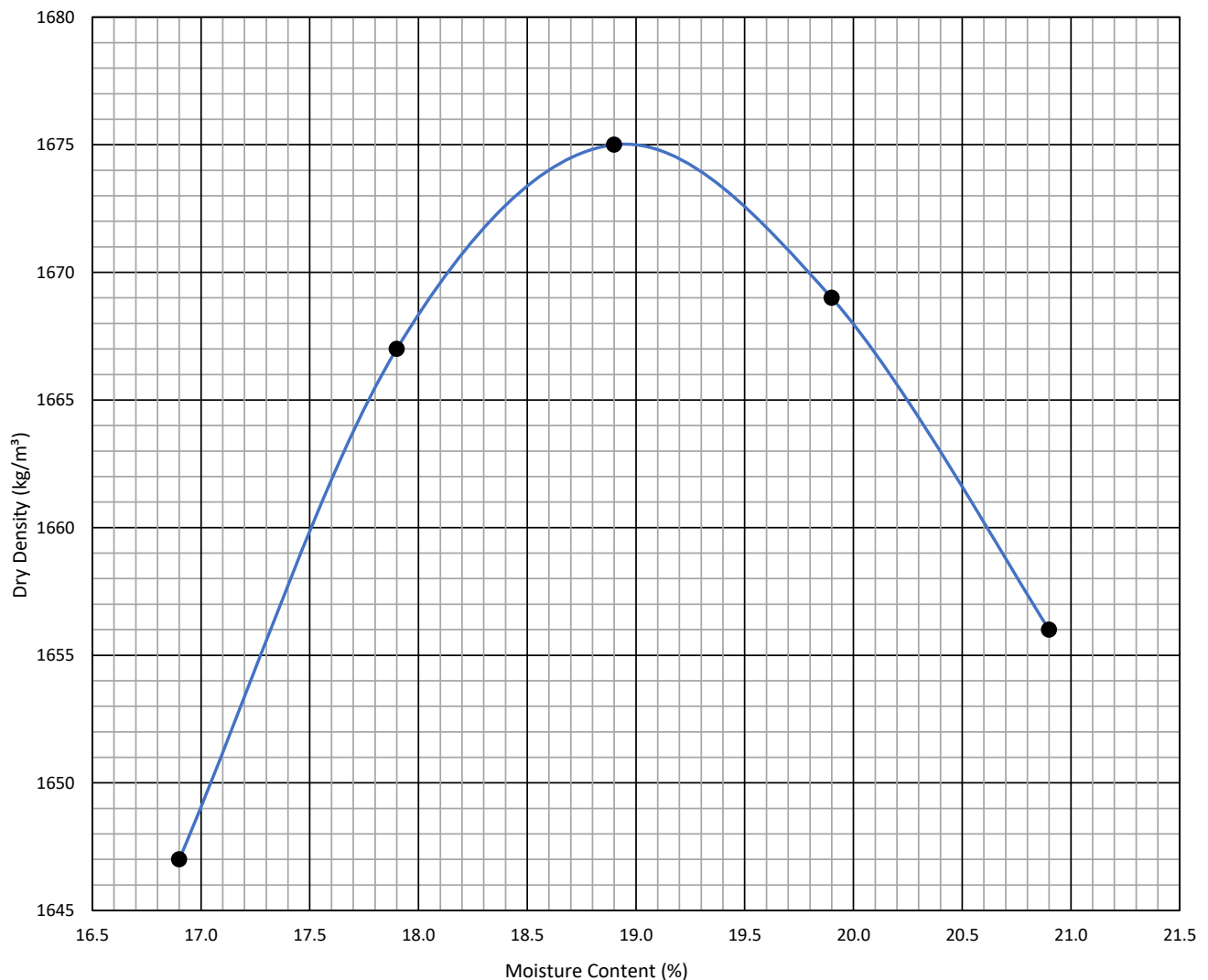
MDD & OMC DETERMINATION (Mod. AASHTO)

Sheet Reference:
R-STL-013 Rev01

Maximum Dry Density: **1675** kg/m³

Optimum Moisture Content: **18.9** %

Moisture Content (%)	16.9	17.9	18.9	19.9	20.9			
Dry Density (kg/m ³)	1647	1667	1675	1669	1656			



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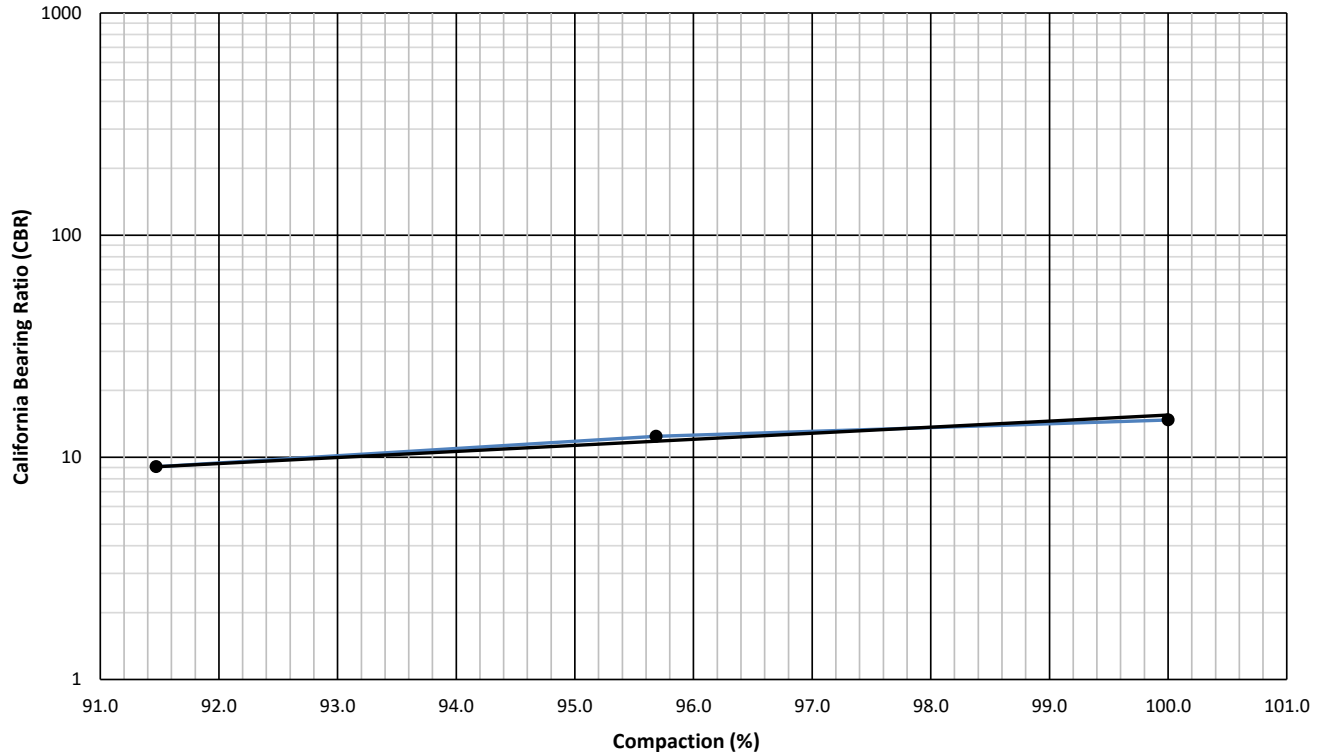
Client Name: BAV Consulting (Pty) Ltd
Project Name: Komati Power Station Repurposing
Sample: KTP32
Depth: (m) 0.2 - 2.2

Job Number: PVT-291
Lab Number: PVT-291-2053
Method: SANS 3001 GR40
Date: 26-Oct-23

CALIFORNIA BEARING RATIO

Sheet Reference:
R-STL-014 Rev01

Mod. AASHTO Values		Compaction Data: CBR			Swell	CBR at (mm)			CBR Values	
MDD	OMC	Dry Dens.	MC	Comp.		2.5	5.0	7.5	Compaction (%)	CBR
(kg/m³)	(%)	(kg/m³)	(%)	(%)	(%)					
2022	11.1	1993	11.0	100.0	0.1	15	15	15	100	15
									98	14
									97	13
2022	11.1	1907	11.0	95.7	0.1	12	13	12	95	12
									93	10
2022	11.1	1823	11.0	91.5	0.2	9	8	7	90	8



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Client Name: BAV Consulting (Pty) Ltd
Project Name: Komati Power Station Repurposing
Sample: KTP32
Depth: (m) 0.2 - 2.2

Job Number: PVT-291
Lab Number: PVT-291-2053
Method: SANS 3001 GR30
Date: 26-Oct-23

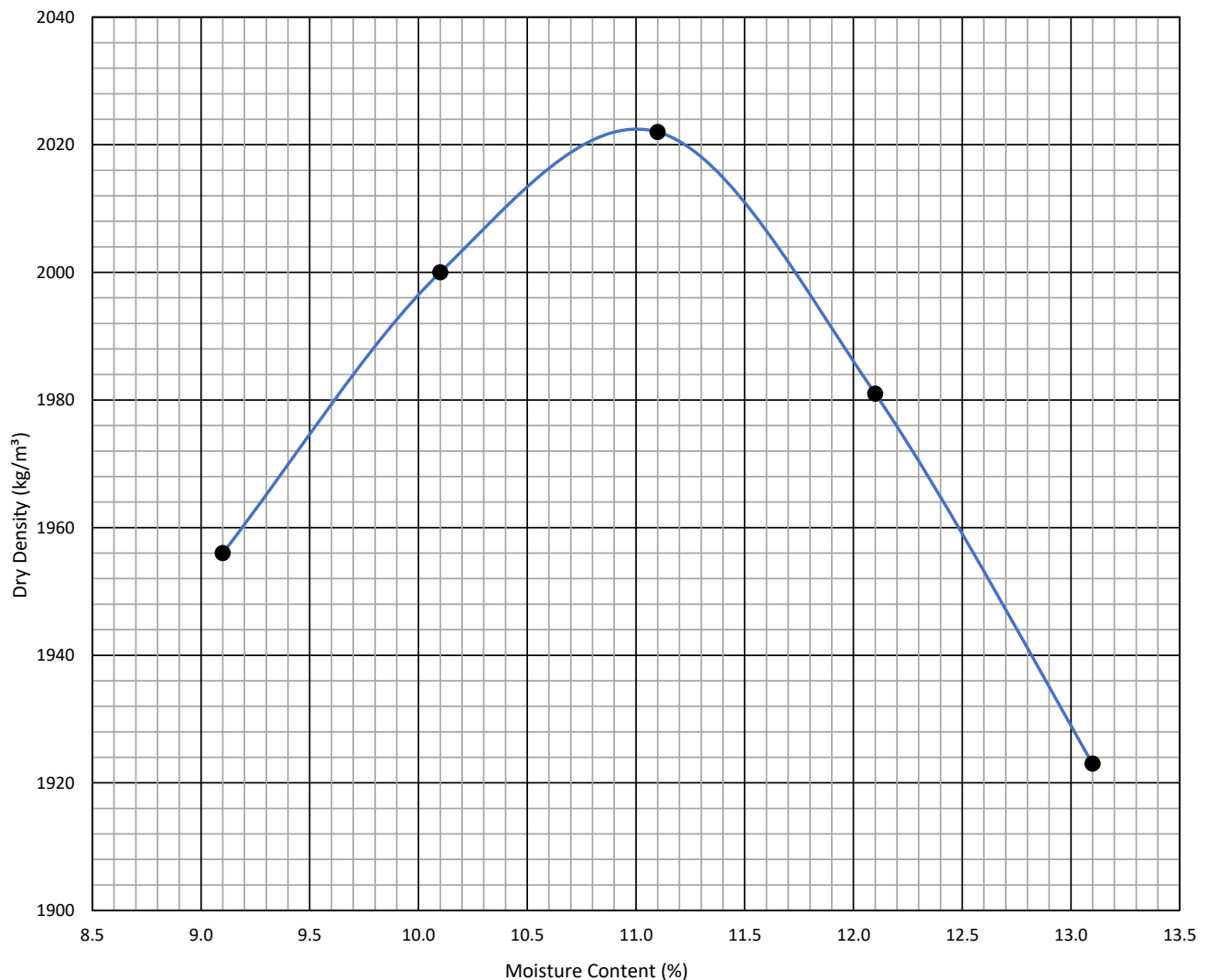
MDD & OMC DETERMINATION (Mod. AASHTO)

Sheet Reference:
R-STL-013 Rev01

Maximum Dry Density: **2022** kg/m³

Optimum Moisture Content: **11.1** %

Moisture Content (%)	9.1	10.1	11.1	12.1	13.1			
Dry Density (kg/m ³)	1956	2000	2022	1981	1923			



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Gerrie | 082 309 4448 | gerrie@stlab.co.za
www.stlab.co.za

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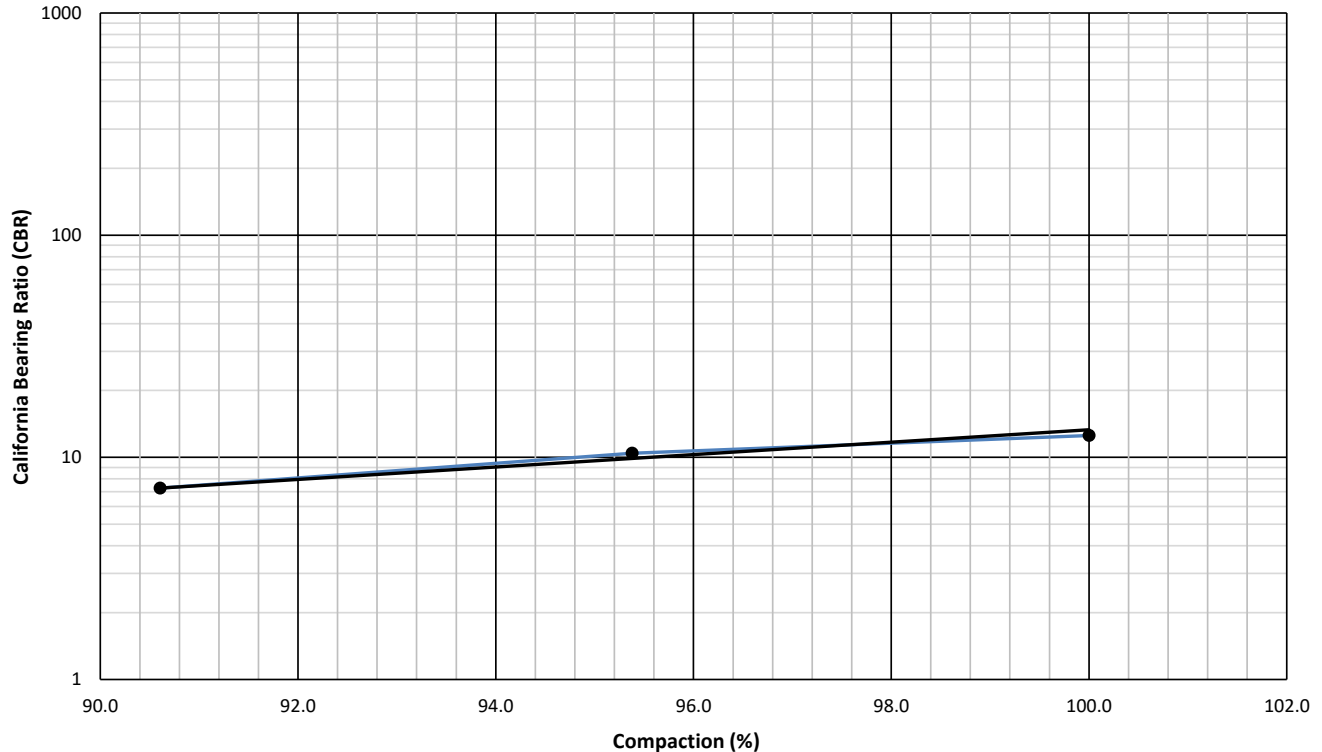
Client Name: BAV Consulting (Pty) Ltd
Project Name: Komati Power Station Repurposing
Sample: KTP33
Depth: (m) 0.2 - 2.4

Job Number: PVT-291
Lab Number: PVT-291-2054
Method: SANS 3001 GR40
Date: 26-Oct-23

CALIFORNIA BEARING RATIO

Sheet Reference:
R-STL-014 Rev01

Mod. AASHTO Values		Compaction Data: CBR			Swell	CBR at (mm)			CBR Values	
MDD	OMC	Dry Dens.	MC	Comp.		2.5	5.0	7.5	Compaction (%)	CBR
(kg/m ³)	(%)	(kg/m ³)	(%)	(%)	(%)					
1965	11.7	1948	11.5	100.0	0.3	13	12	11	100	13
									98	12
									97	11
1965	11.7	1858	11.5	95.4	0.3	10	10	9	95	10
									93	9
1965	11.7	1765	11.5	90.6	0.4	7	6	5	90	7



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Client Name: BAV Consulting (Pty) Ltd
Project Name: Komati Power Station Repurposing
Sample: KTP33
Depth: (m) 0.2 - 2.4

Job Number: PVT-291
Lab Number: PVT-291-2054
Method: SANS 3001 GR30
Date: 26-Oct-23

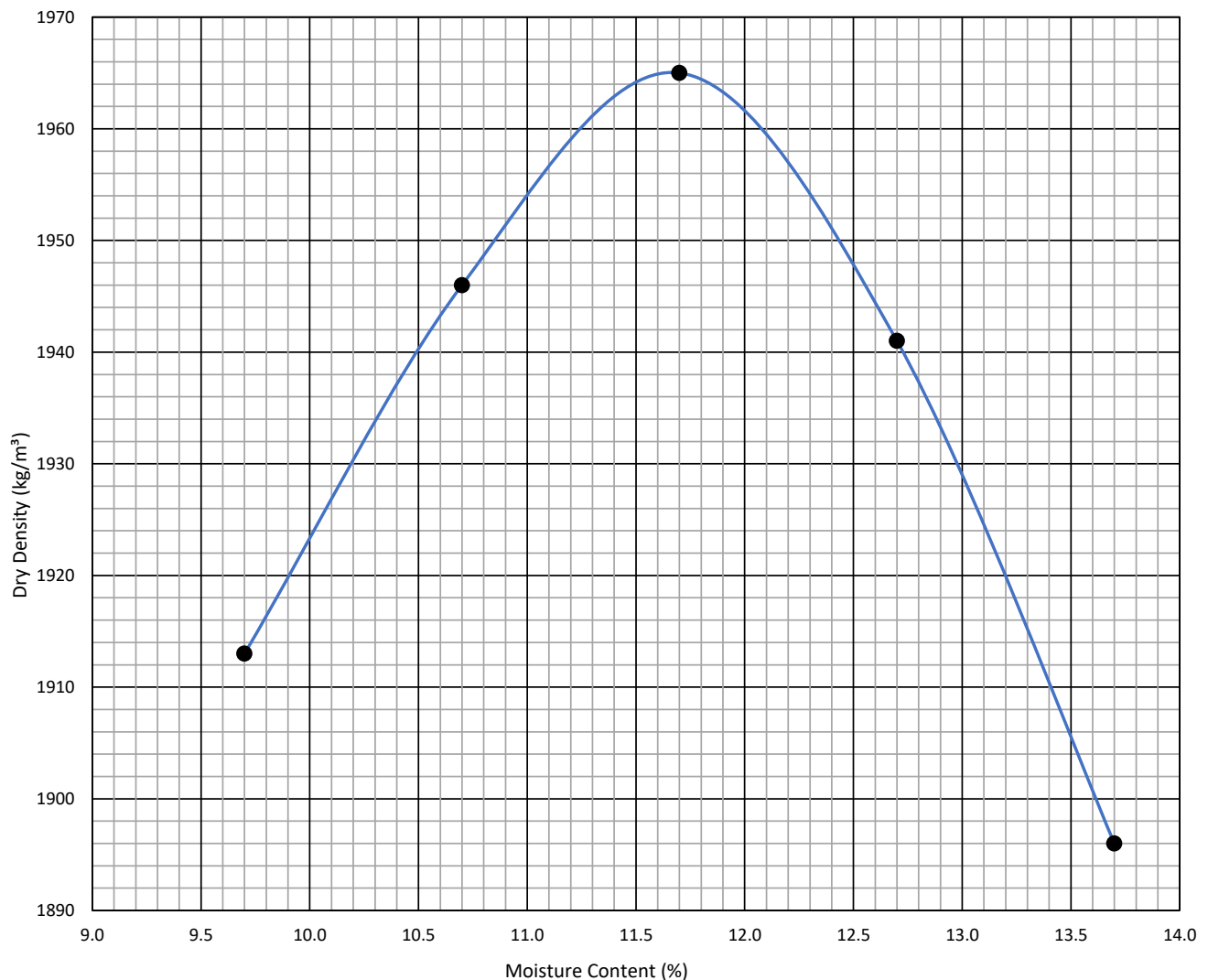
MDD & OMC DETERMINATION (Mod. AASHTO)

Sheet Reference:
R-STL-013 Rev01

Maximum Dry Density: **1965** kg/m³

Optimum Moisture Content: **11.7** %

Moisture Content (%)	9.7	10.7	11.7	12.7	13.7			
Dry Density (kg/m ³)	1913	1946	1965	1941	1896			



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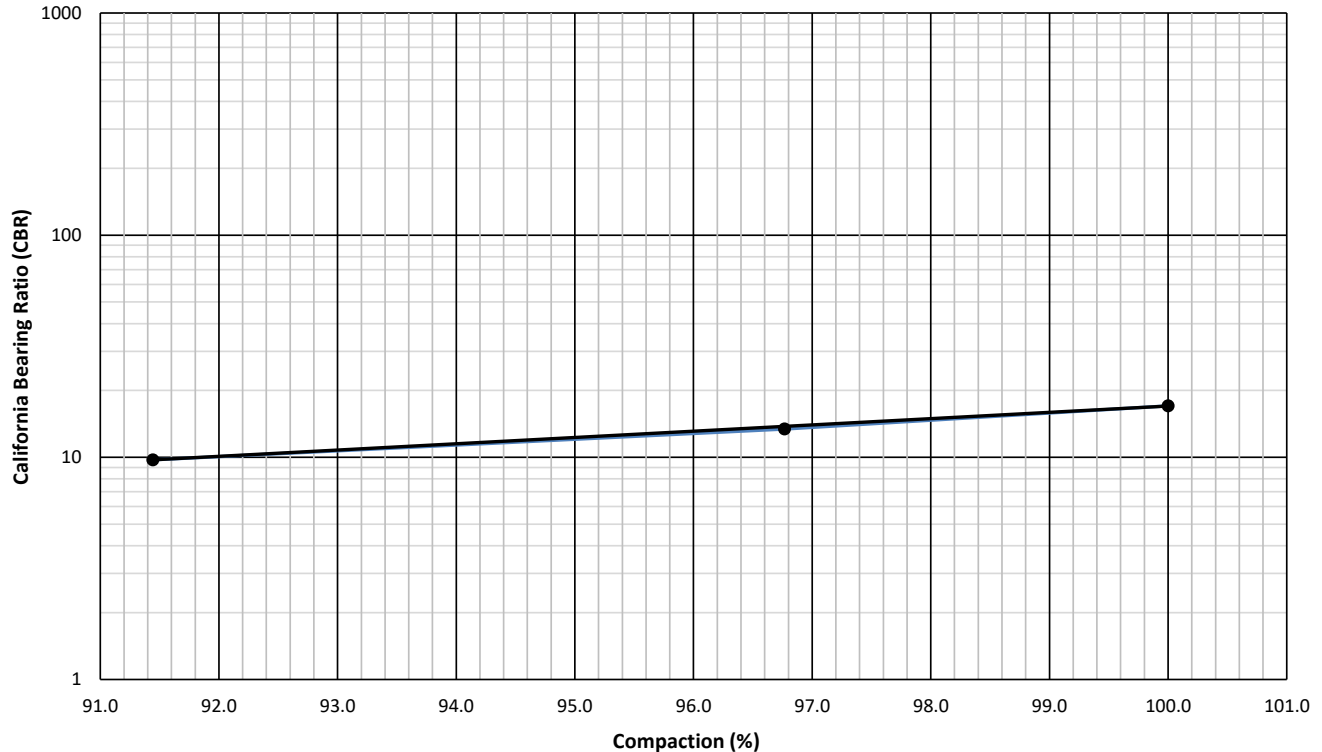
Client Name: BAV Consulting (Pty) Ltd
Project Name: Komati Power Station Repurposing
Sample: KTP35
Depth: (m) 0.2 - 2.6

Job Number: PVT-291
Lab Number: PVT-291-2056
Method: SANS 3001 GR40
Date: 26-Oct-23

CALIFORNIA BEARING RATIO

Sheet Reference:
R-STL-014 Rev01

Mod. AASHTO Values		Compaction Data: CBR			Swell	CBR at (mm)			CBR Values	
MDD	OMC	Dry Dens.	MC	Comp.		2.5	5.0	7.5	Compaction (%)	CBR
(kg/m ³)	(%)	(kg/m ³)	(%)	(%)	(%)					
2030	10.3	2010	10.5	100.0	0.1	17	17	17	100	17
									98	15
									97	14
2030	10.3	1945	10.5	96.8	0.1	13	14	14	95	12
									93	11
2030	10.3	1838	10.5	91.4	0.2	10	8	7	90	9



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Client Name: BAV Consulting (Pty) Ltd
Project Name: Komati Power Station Repurposing
Sample: KTP35
Depth: (m) 0.2 - 2.6

Job Number: PVT-291
Lab Number: PVT-291-2056
Method: SANS 3001 GR30
Date: 26-Oct-23

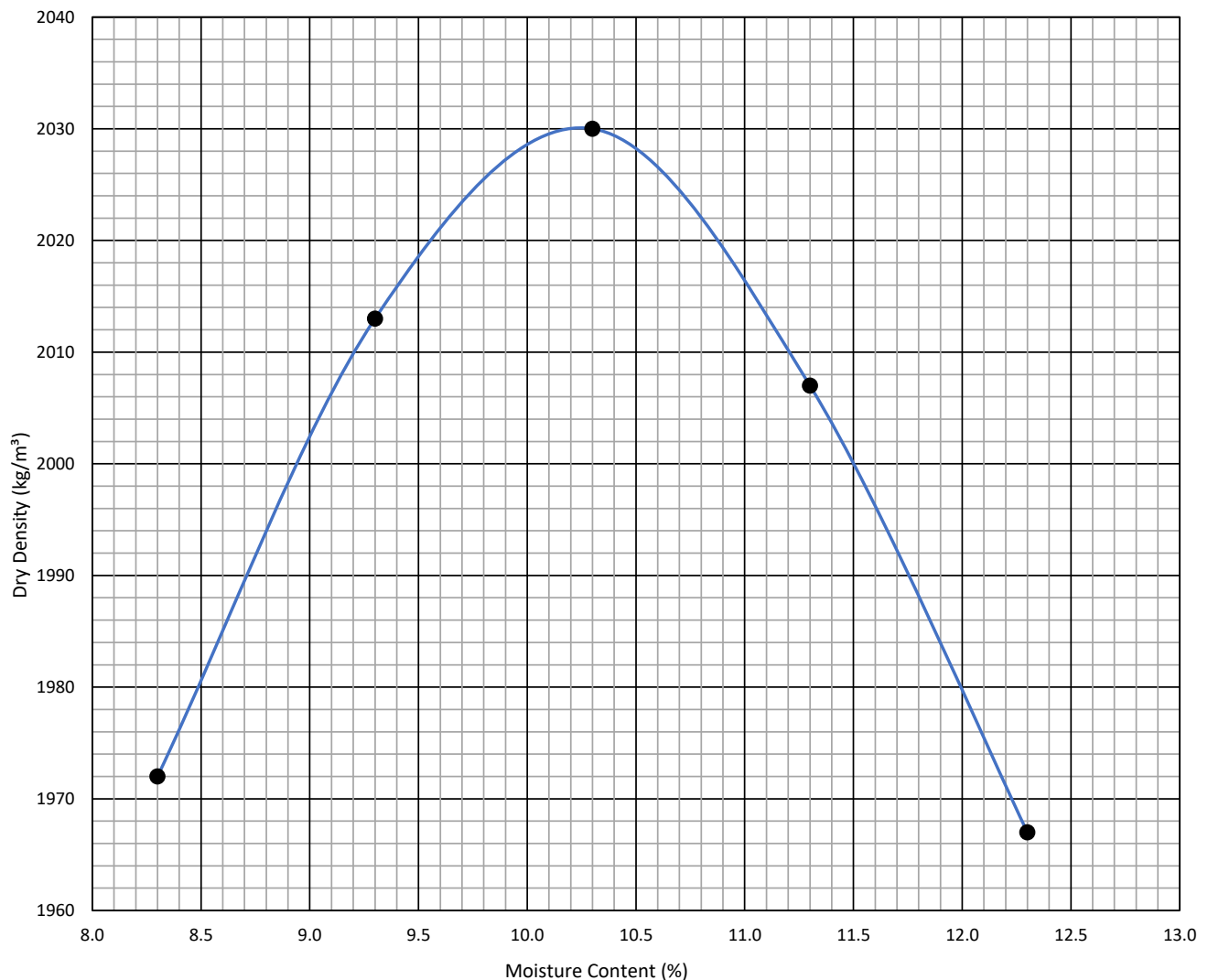
MDD & OMC DETERMINATION (Mod. AASHTO)

Sheet Reference:
R-STL-013 Rev01

Maximum Dry Density: **2030** kg/m³

Optimum Moisture Content: **10.3** %

Moisture Content (%)	8.3	9.3	10.3	11.3	12.3			
Dry Density (kg/m ³)	1972	2013	2030	2007	1967			



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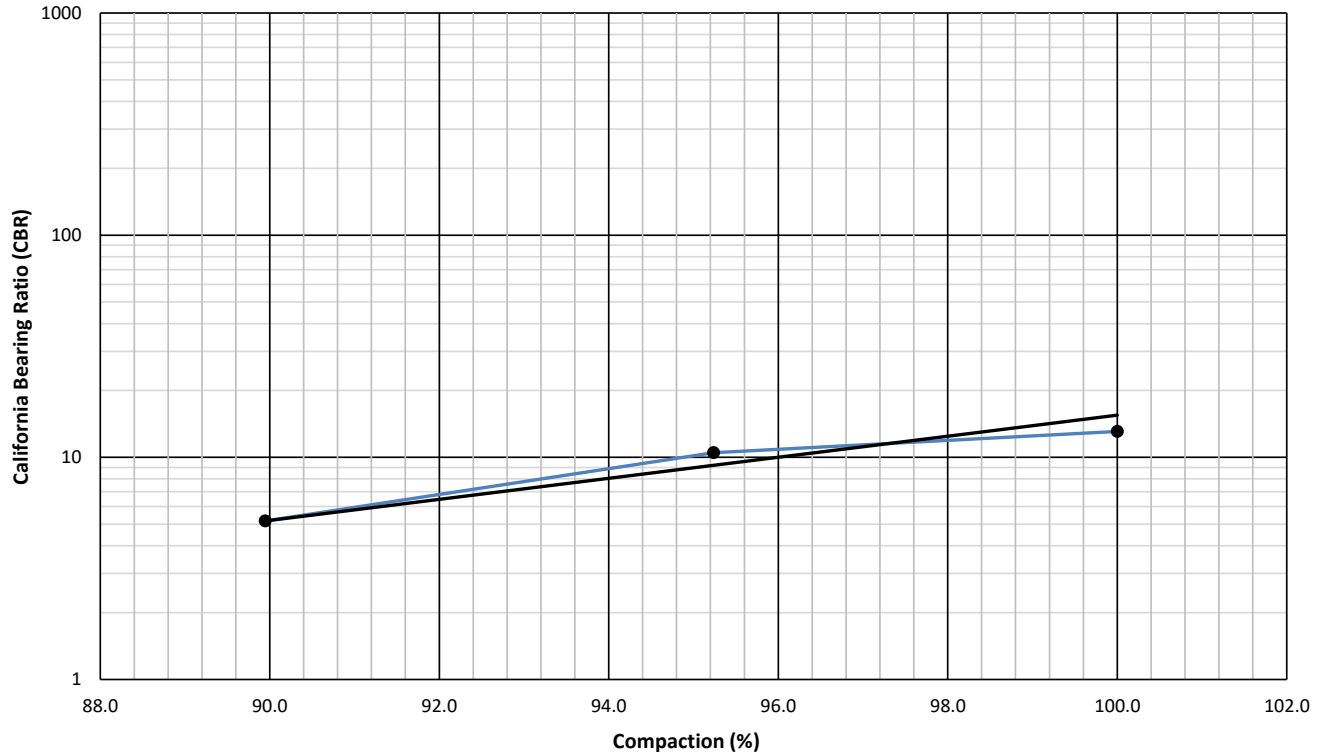
Client Name: BAV Consulting (Pty) Ltd
Project Name: Komati Power Station Repurposing
Sample: KTP36
Depth: (m) 0.9 - 1.9

Job Number: PVT-291
Lab Number: PVT-291-2057
Method: SANS 3001 GR40
Date: 26-Oct-23

CALIFORNIA BEARING RATIO

Sheet Reference:
R-STL-014 Rev01

Mod. AASHTO Values		Compaction Data: CBR			Swell	CBR at (mm)			CBR Values	
MDD	OMC	Dry Dens.	MC	Comp.		2.5	5.0	7.5	Compaction (%)	CBR
(kg/m ³)	(%)	(kg/m ³)	(%)	(%)	(%)					
1914	10.5	1890	10.3	100.0	0.0	13	19	24	100	13
									98	12
									97	11
1914	10.5	1800	10.3	95.2	0.1	10	15	18	95	10
									93	8
1914	10.5	1700	10.3	89.9	0.1	5	6	7	90	5



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Client Name: BAV Consulting (Pty) Ltd
Project Name: Komati Power Station Repurposing
Sample: KTP36
Depth: (m) 0.9 - 1.9

Job Number: PVT-291
Lab Number: PVT-291-2057
Method: SANS 3001 GR30
Date: 26-Oct-23

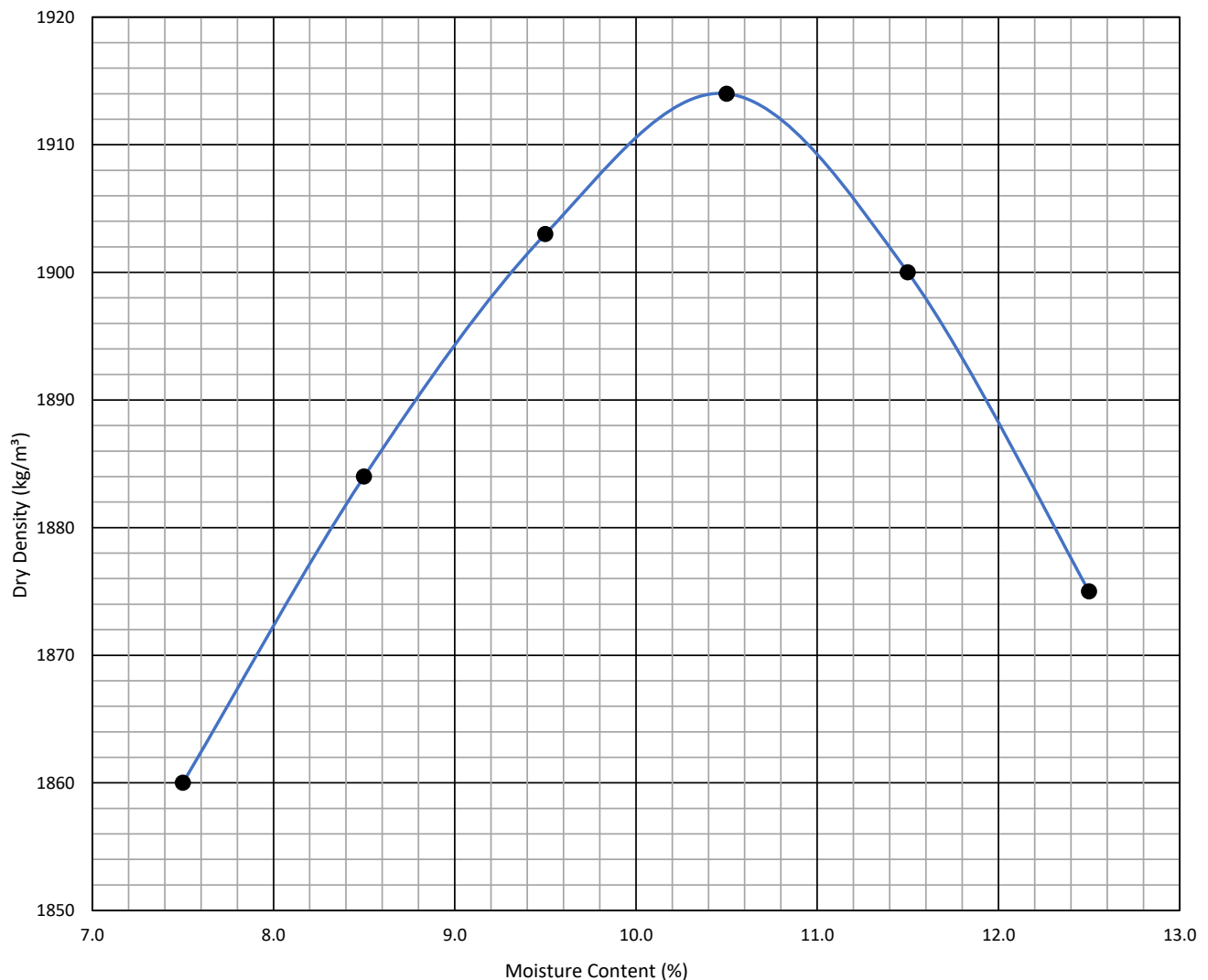
MDD & OMC DETERMINATION (Mod. AASHTO)

Sheet Reference:
R-STL-013 Rev01

Maximum Dry Density: **1914** kg/m³

Optimum Moisture Content: **10.5** %

Moisture Content (%)	7.5	8.5	9.5	10.5	11.5	12.5		
Dry Density (kg/m ³)	1860	1884	1903	1914	1900	1875		



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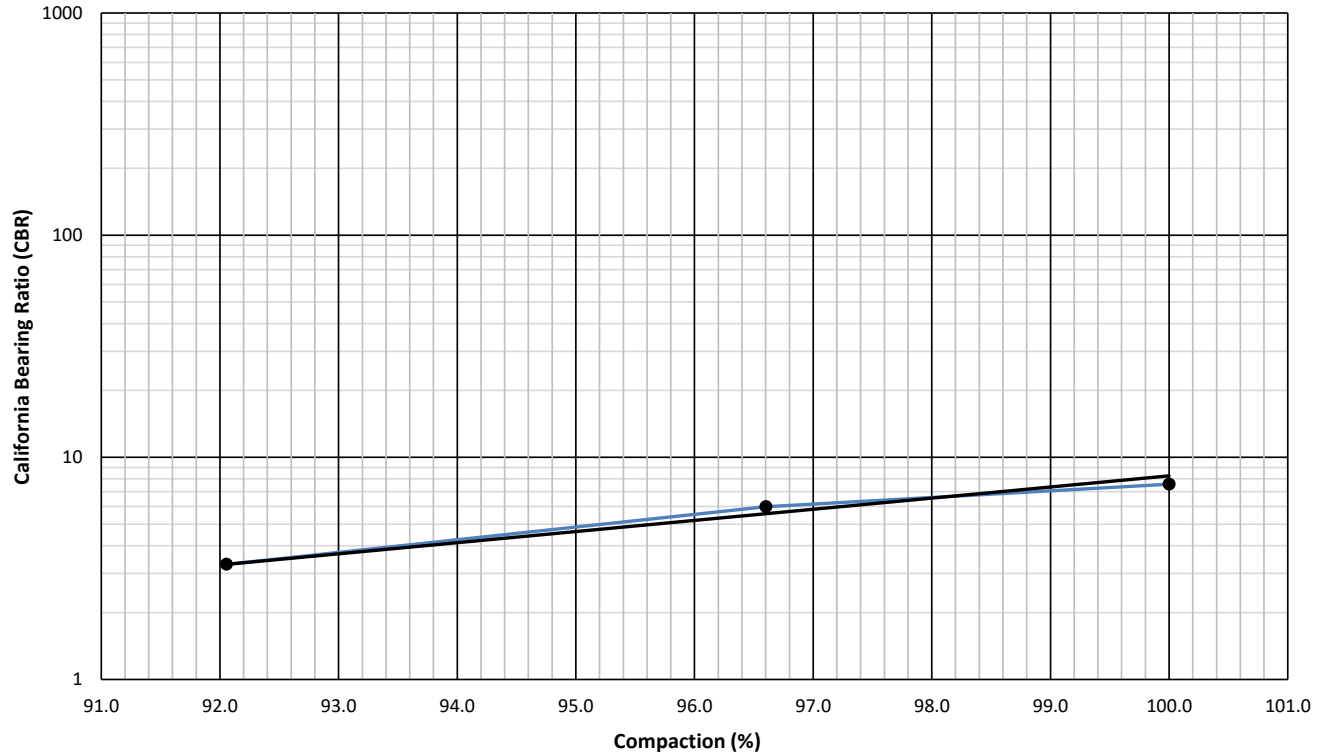
Client Name: BAV Consulting (Pty) Ltd
Project Name: Komati Power Station Repurposing
Sample: KTP39
Depth: (m) 1.1 - 3.2

Job Number: PVT-291
Lab Number: PVT-291-2060
Method: SANS 3001 GR40
Date: 26-Oct-23

CALIFORNIA BEARING RATIO

Sheet Reference:
R-STL-014 Rev01

Mod. AASHTO Values		Compaction Data: CBR			Swell	CBR at (mm)			CBR Values	
MDD	OMC	Dry Dens.	MC	Comp.		2.5	5.0	7.5	Compaction (%)	CBR
(kg/m ³)	(%)	(kg/m ³)	(%)	(%)	(%)					
1910	12.0	1913	11.8	100.0	0.7	8	7	6	100	7.6
									98	6.6
									97	6.2
1910	12.0	1848	11.8	96.6	0.8	6	6	5	95	4.9
									93	3.7
1910	12.0	1761	11.8	92.1	0.9	3	4	3	90	2.5



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Client Name: BAV Consulting (Pty) Ltd
Project Name: Komati Power Station Repurposing
Sample: KTP39
Depth: (m) 1.1 - 3.2

Job Number: PVT-291
Lab Number: PVT-291-2060
Method: SANS 3001 GR30
Date: 26-Oct-23

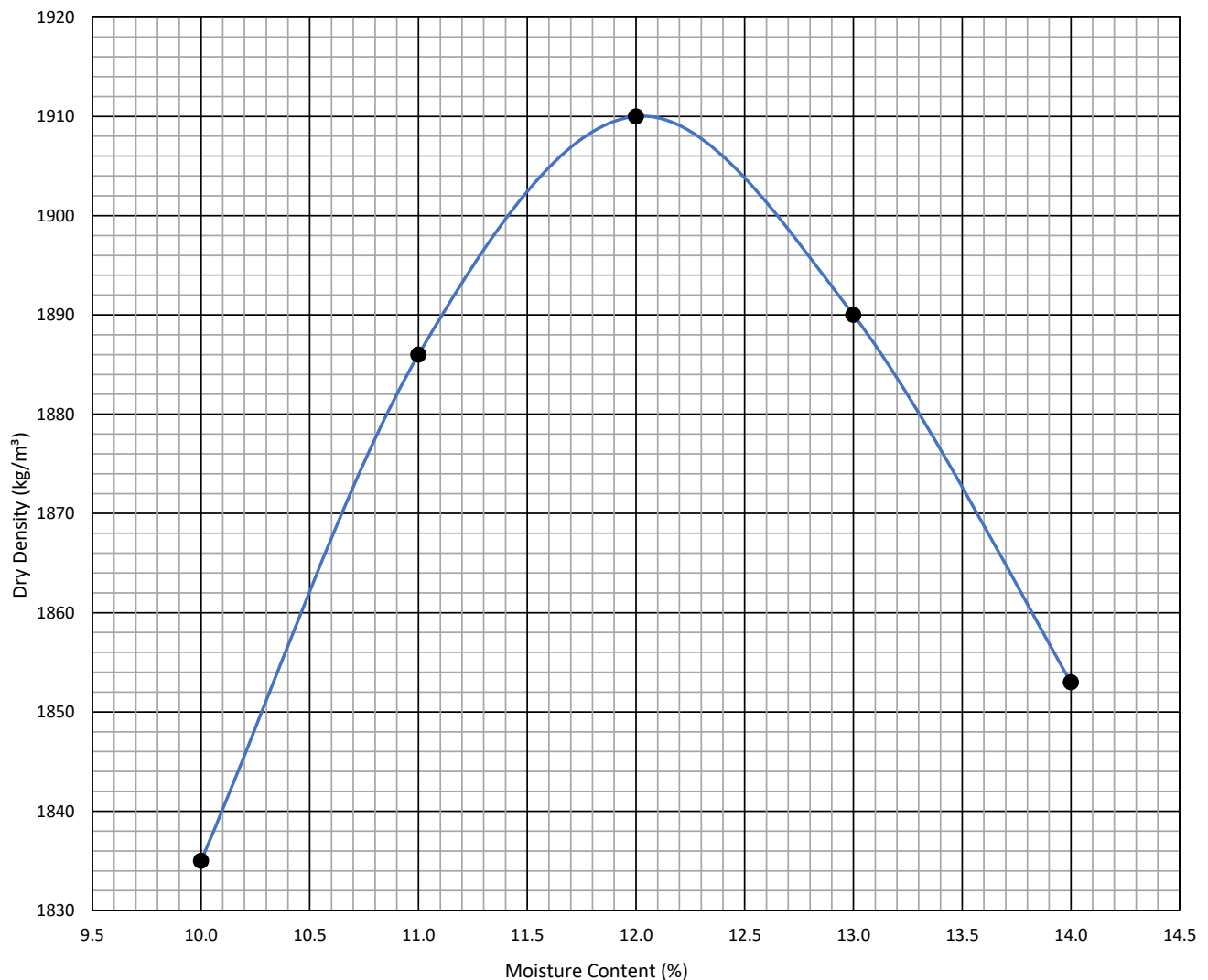
MDD & OMC DETERMINATION (Mod. AASHTO)

Sheet Reference:
R-STL-013 Rev01

Maximum Dry Density: **1910** kg/m³

Optimum Moisture Content: **12.0** %

Moisture Content (%)	10.0	11.0	12.0	13.0	14.0			
Dry Density (kg/m ³)	1835	1886	1910	1890	1853			



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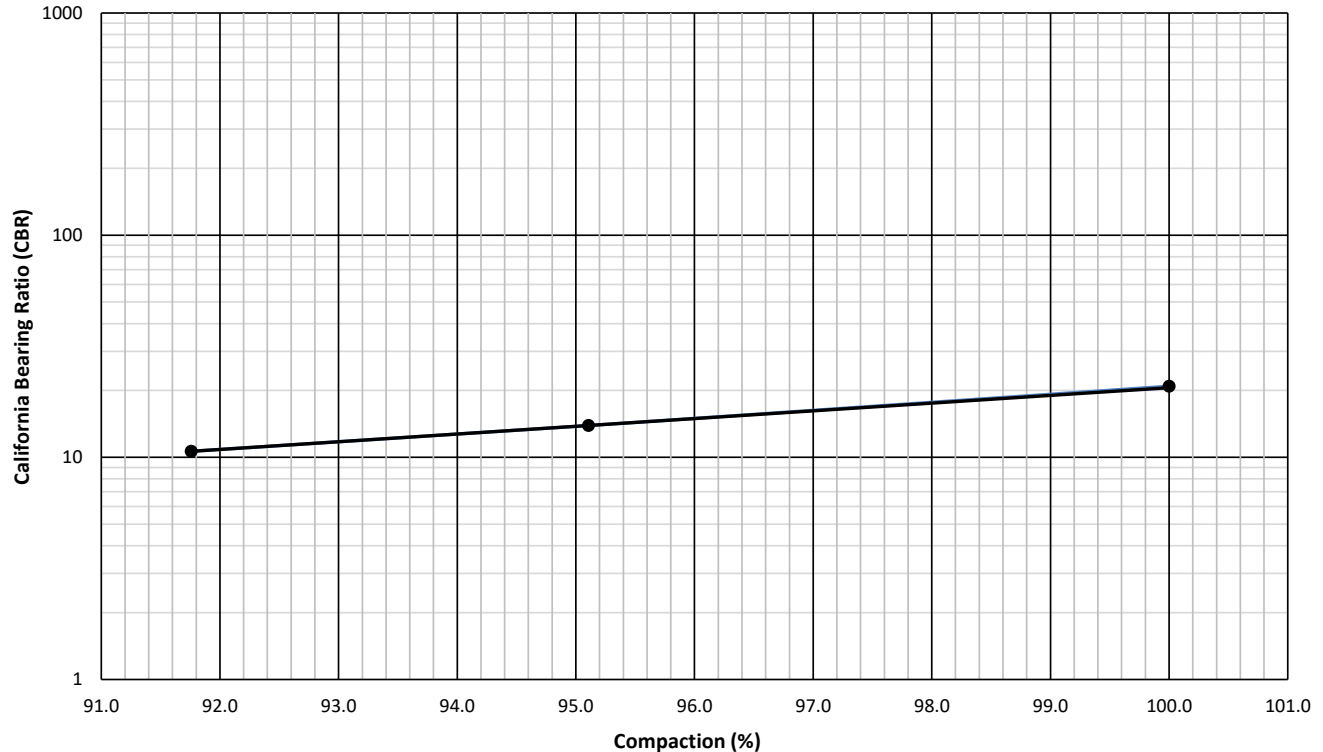
Client Name: BAV Consulting (Pty) Ltd
Project Name: Komati Power Station Repurposing
Sample: KTP41
Depth: (m) 1.4 - 2.0

Job Number: PVT-291
Lab Number: PVT-291-2062
Method: SANS 3001 GR40
Date: 26-Oct-23

CALIFORNIA BEARING RATIO

Sheet Reference:
R-STL-014 Rev01

Mod. AASHTO Values		Compaction Data: CBR			Swell	CBR at (mm)			CBR Values	
MDD	OMC	Dry Dens.	MC	Comp.		2.5	5.0	7.5	Compaction (%)	CBR
(kg/m ³)	(%)	(kg/m ³)	(%)	(%)	(%)					
1937	10.0	1941	9.9	100.0	0.1	21	28	32	100	21
									98	18
									97	16
1937	10.0	1846	9.9	95.1	0.2	14	14	14	95	14
									93	12
1937	10.0	1781	9.9	91.8	0.3	11	13	15	90	9



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Client Name: BAV Consulting (Pty) Ltd
Project Name: Komati Power Station Repurposing
Sample: KTP41
Depth: (m) 1.4 - 2.0

Job Number: PVT-291
Lab Number: PVT-291-2062
Method: SANS 3001 GR30
Date: 26-Oct-23

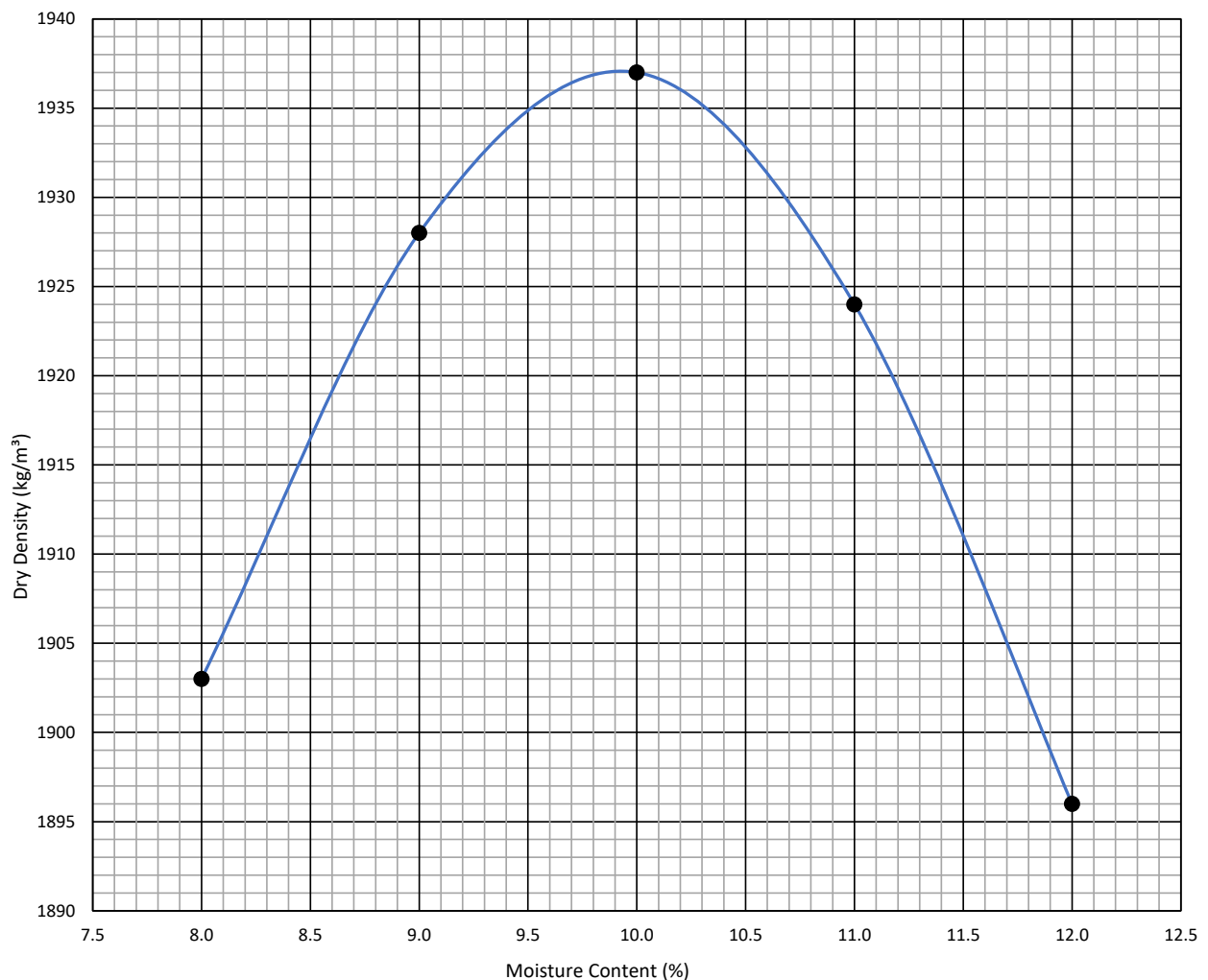
MDD & OMC DETERMINATION (Mod. AASHTO)

Sheet Reference:
R-STL-013 Rev01

Maximum Dry Density: **1937** kg/m³

Optimum Moisture Content: **10.0** %

Moisture Content (%)	8.0	9.0	10.0	11.0	12.0			
Dry Density (kg/m ³)	1903	1928	1937	1924	1896			



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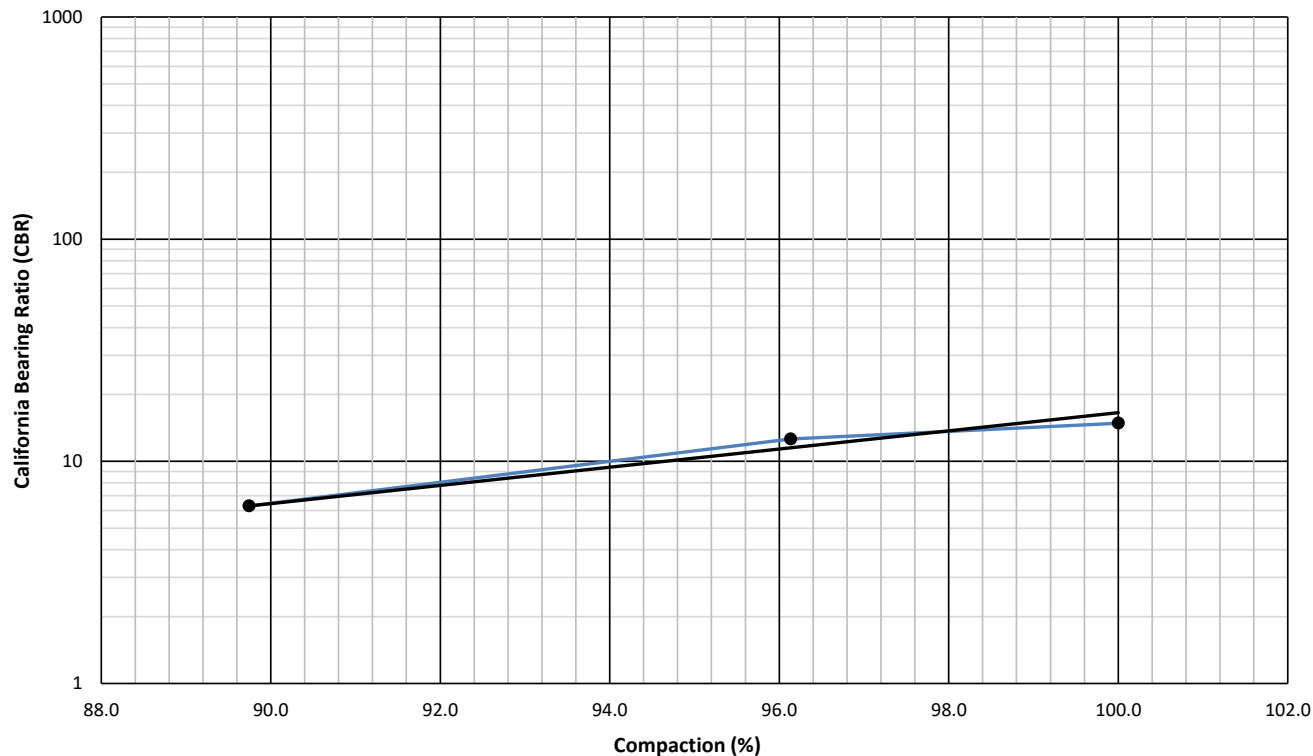
Client Name: BAV Consulting (Pty) Ltd
Project Name: Komati Power Station Repurposing
Sample: KTP42
Depth: (m) 0.7 - 2.5

Job Number: PVT-291
Lab Number: PVT-291-2063
Method: SANS 3001 GR40
Date: 26-Oct-23

CALIFORNIA BEARING RATIO

Sheet Reference:
R-STL-014 Rev01

Mod. AASHTO Values		Compaction Data: CBR			Swell	CBR at (mm)			CBR Values	
MDD	OMC	Dry Dens.	MC	Comp.		2.5	5.0	7.5	Compaction (%)	CBR
(kg/m ³)	(%)	(kg/m ³)	(%)	(%)	(%)					
1889	13.9	1862	13.6	100.0	0.0	15	12	10	100	15
									98	14
									97	13
1889	13.9	1790	13.6	96.1	0.0	13	11	10	95	11
									93	9
1889	13.9	1671	13.6	89.7	0.1	6	5	4	90	7



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Client Name: BAV Consulting (Pty) Ltd
Project Name: Komati Power Station Repurposing
Sample: KTP42
Depth: (m) 0.7 - 2.5

Job Number: PVT-291
Lab Number: PVT-291-2063
Method: SANS 3001 GR30
Date: 26-Oct-23

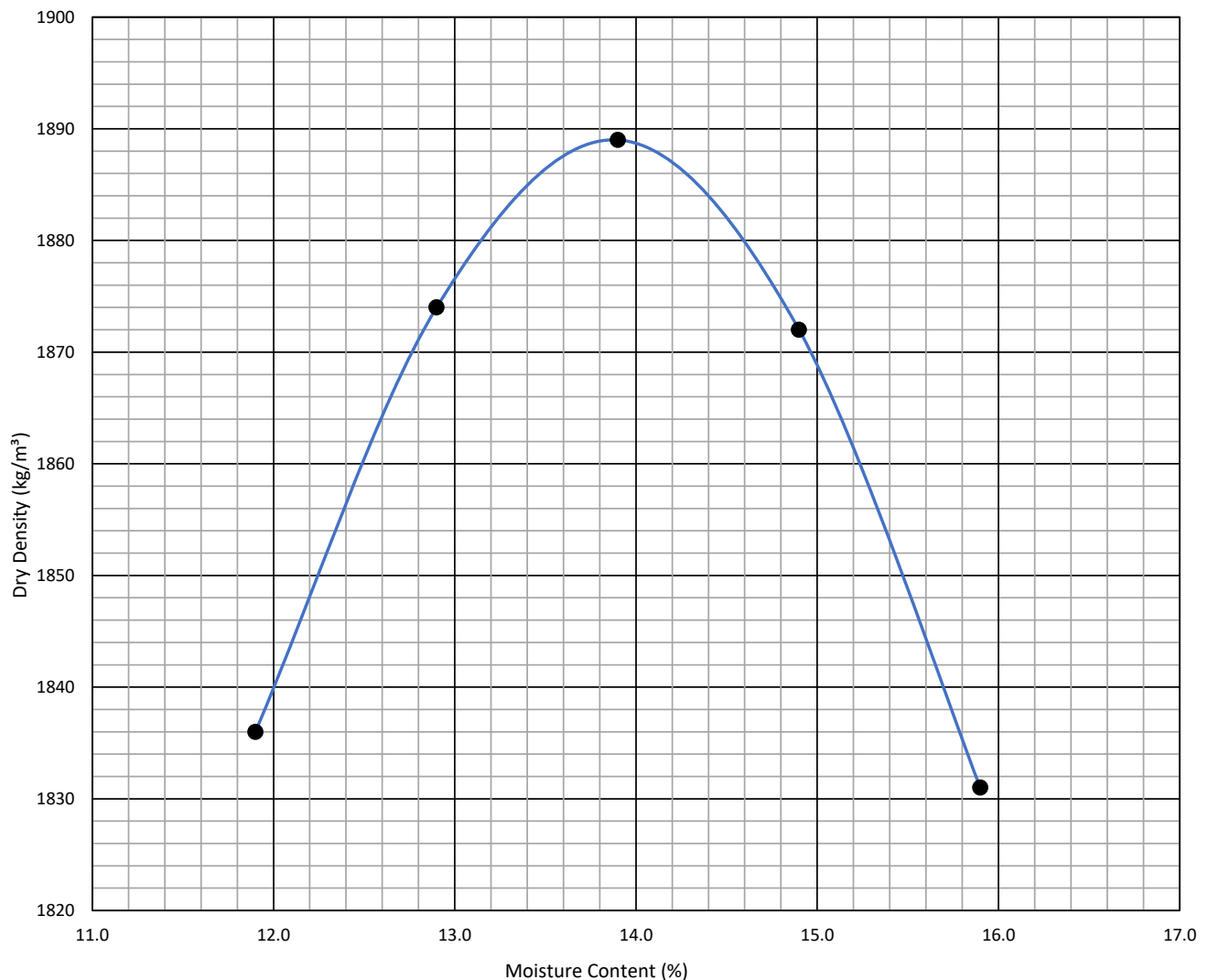
MDD & OMC DETERMINATION (Mod. AASHTO)

Sheet Reference:
R-STL-013 Rev01

Maximum Dry Density: **1889** kg/m³

Optimum Moisture Content: **13.9** %

Moisture Content (%)	11.9	12.9	13.9	14.9	15.9			
Dry Density (kg/m ³)	1836	1874	1889	1872	1831			



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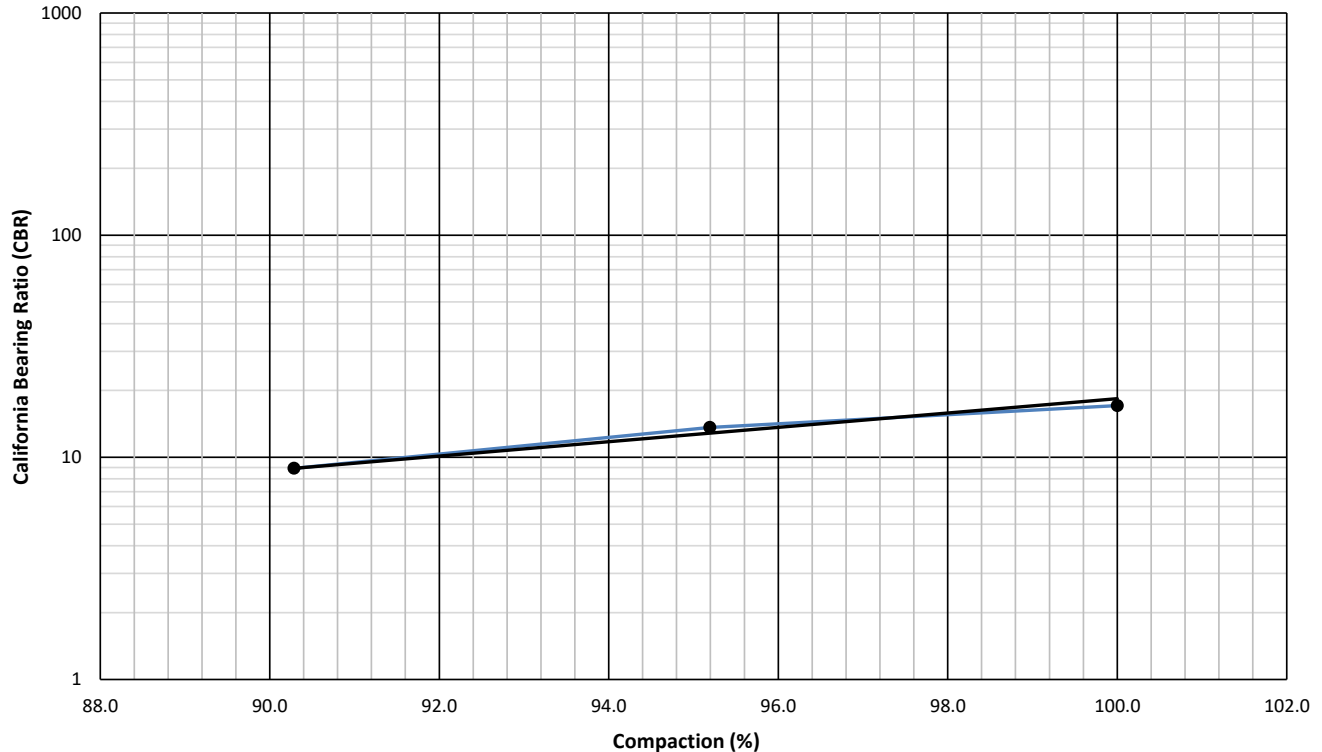
Client Name: BAV Consulting (Pty) Ltd
Project Name: Komati Power Station Repurposing
Sample: KTP44
Depth: (m) 0.1 - 2.6

Job Number: PVT-291
Lab Number: PVT-291-2065
Method: SANS 3001 GR40
Date: 26-Oct-23

CALIFORNIA BEARING RATIO

Sheet Reference:
R-STL-014 Rev01

Mod. AASHTO Values		Compaction Data: CBR			Swell (%)	CBR at (mm)			CBR Values	
MDD (kg/m³)	OMC (%)	Dry Dens. (kg/m³)	MC (%)	Comp. (%)		2.5	5.0	7.5	Compaction (%)	CBR
1942	11.1	1935	11.9	100.0	0.0	17	16	15	100	17
									98	16
									97	15
1942	11.1	1842	11.9	95.2	0.0	14	12	11	95	13
									93	11
1942	11.1	1747	11.9	90.3	0.0	9	7	6	90	9



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Client Name: BAV Consulting (Pty) Ltd
Project Name: Komati Power Station Repurposing
Sample: KTP44
Depth: (m) 0.1 - 2.6

Job Number: PVT-291
Lab Number: PVT-291-2065
Method: SANS 3001 GR30
Date: 26-Oct-23

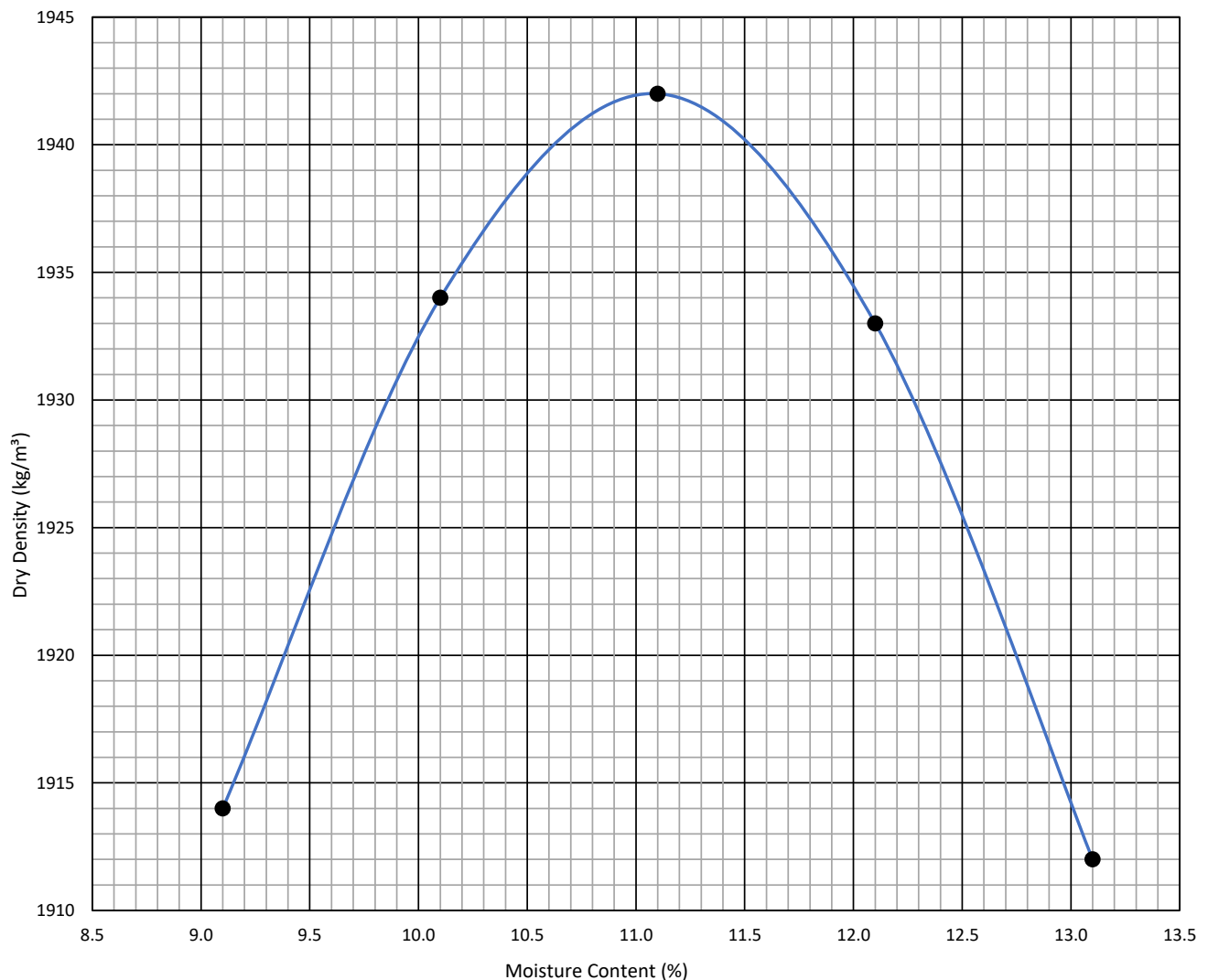
MDD & OMC DETERMINATION (Mod. AASHTO)

Sheet Reference:
R-STL-013 Rev01

Maximum Dry Density: **1942** kg/m³

Optimum Moisture Content: **11.1** %

Moisture Content (%)	9.1	10.1	11.1	12.1	13.1			
Dry Density (kg/m ³)	1914	1934	1942	1933	1912			



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Gerrie | 082 309 4448 | gerrie@stlab.co.za
www.stlab.co.za

Quality | Excellence | On Time

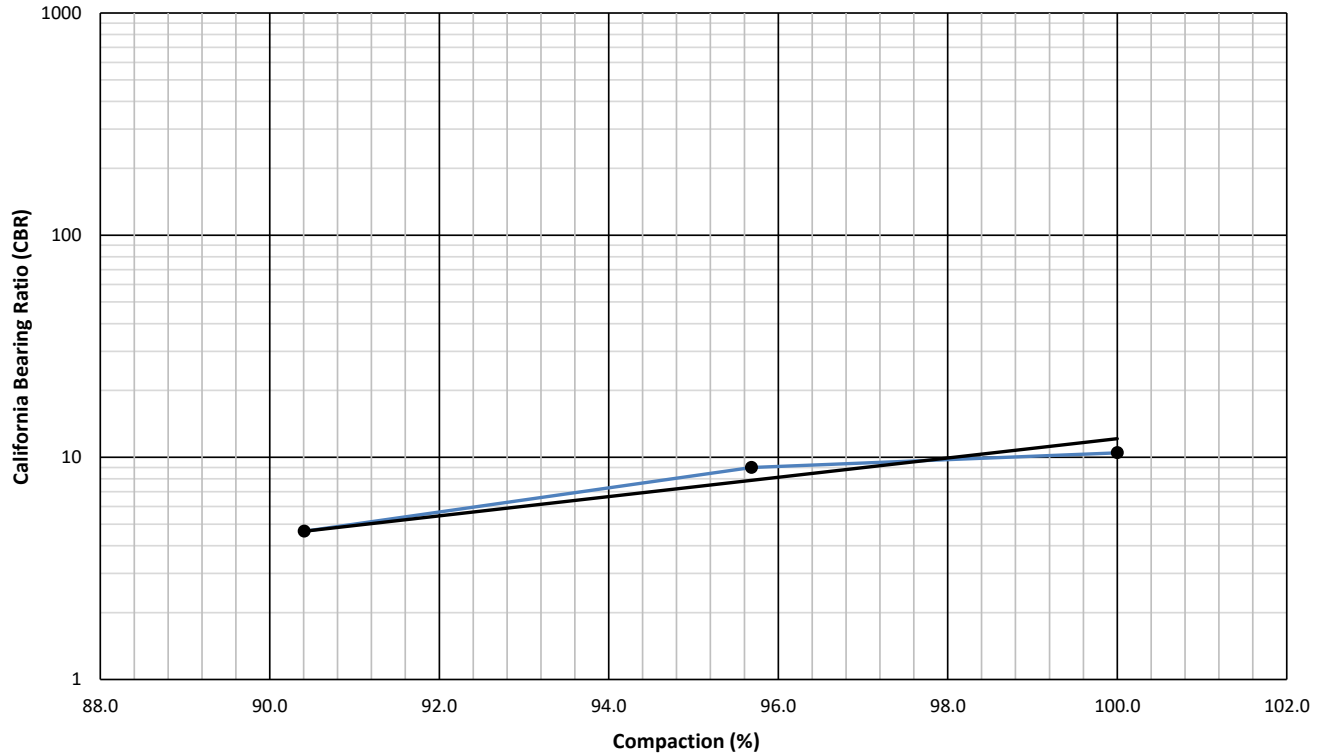
Client Name: BAV Consulting (Pty) Ltd
Project Name: Komati Power Station Repurposing
Sample: KTP46
Depth: (m) 1.0 - 2.3

Job Number: PVT-291
Lab Number: PVT-291-2067
Method: SANS 3001 GR40
Date: 26-Oct-23

CALIFORNIA BEARING RATIO

Sheet Reference:
R-STL-014 Rev01

Mod. AASHTO Values		Compaction Data: CBR			Swell	CBR at (mm)			CBR Values	
MDD	OMC	Dry Dens.	MC	Comp.		2.5	5.0	7.5	Compaction (%)	CBR
(kg/m³)	(%)	(kg/m³)	(%)	(%)	(%)					
1911	12.9	1876	13.0	100.0	0.1	10	9	8	100	11
									98	10
									97	9
1911	12.9	1795	13.0	95.7	0.1	9	7	6	95	8
									93	6
1911	12.9	1696	13.0	90.4	0.1	5	4	3	90	4



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Client Name: BAV Consulting (Pty) Ltd
Project Name: Komati Power Station Repurposing
Sample: KTP46
Depth: (m) 1.0 - 2.3

Job Number: PVT-291
Lab Number: PVT-291-2067
Method: SANS 3001 GR30
Date: 26-Oct-23

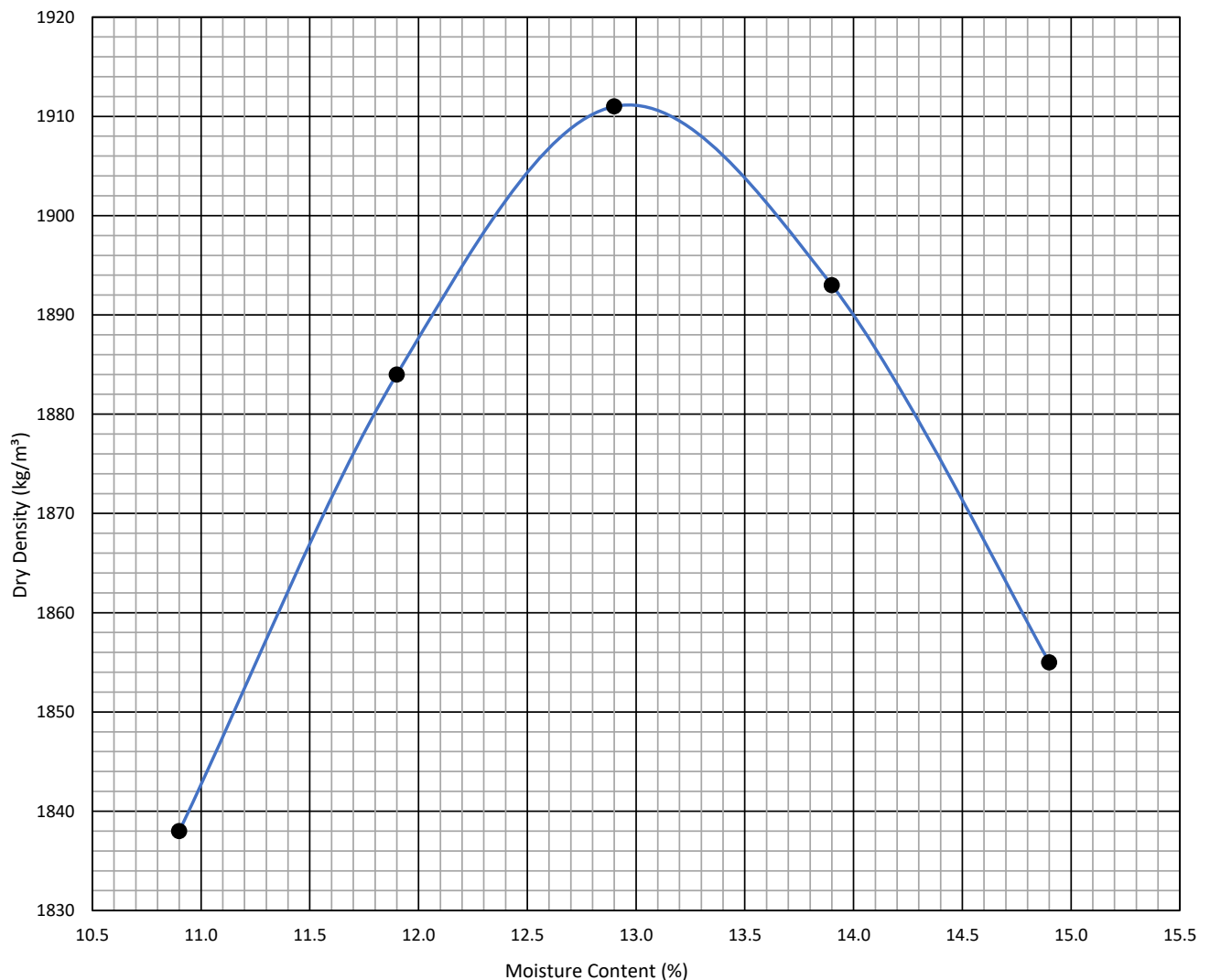
MDD & OMC DETERMINATION (Mod. AASHTO)

Sheet Reference:
R-STL-013 Rev01

Maximum Dry Density: **1911** kg/m³

Optimum Moisture Content: **12.9** %

Moisture Content (%)	10.9	11.9	12.9	13.9	14.9			
Dry Density (kg/m ³)	1838	1884	1911	1893	1855			



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CERTIFICATE OF ANALYSES
BASSON INDEX

Date received: 2023-10-18

Project number: 1000

Report number: 125716

Date completed: 2023-10-30

Order number: T 050

Client name: Specialised Testing Laboratory

Address: Unit 1, 13 Bloubokkie Street, Koedoespoort, 0186

Telephone: ---

Contact person: Thinus Hofsink

Email: thinus@stlab.co.za

Cell: 082 309 4448 (Gerrie)

Analyses in mg/ℓ (Unless specified otherwise)	Method Identification	Sample Identification:	
		KTP6 0.0 - 1.0m PVT-291-2031	KTP11 0.7 - 2.4m PVT-291-2034
Sample Number		23-25483	23-25484
Leachate used	WLAB075	Distilled Water	Distilled Water
Mass Used (g)	---	500	500
Volume Used (mℓ)	---	1000	1000
pH Value at 25°C	WLAB001	6.4	6.7
pHs Value at 20°C (calc)	WLAB053b	9.5	9.8
Electrical Conductivity in mS/m at 25°C	WLAB002	10.7	3.4
Total Dissolved Solids (calc)	WLAB068	72	23
Total Alkalinity as CaCO ₃	WLAB007	16	24
Total Hardness as CaCO ₃ (calc)	WLAB051b	36	9
Calcium Hardness as CaCO ₃ (calc)	WLAB051a	20	5
Calcium as Ca	WLAB015	8	2
Magnesium as Mg	WLAB015	4	1
Free & Saline Ammonia	WLAB046	<0.1	<0.1
Ammonium as NH ₄ (calc)	WLAB068	<0.3	<0.3
Sulphate as SO ₄	WLAB046	31	8
Chloride as Cl	WLAB046	4	<2
Langelier Index at 20°C (calc)	WLAB053c	-3.1	-3.1
Ryznar Index at 20°C (calc)	WLAB053d	12.5	13.0
Corrosivity Ratio (calc)	WLAB054	2.4	0.4
Leaching Index [LCSI] (calc)*	---	2603	2645
Spalling Index [SCSI] (calc)*	---	4	1
Aggressiveness Index [N _c] (calc)*	---	2607	2646

Please note:

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Cell: 082 309 4448 (Gerrie)

Analyses in mg/ℓ (Unless specified otherwise)	Method Identification	Sample Identification:	
		KTP17 0.2 - 2.2m PVT-291-2039	KTP24 0.2 - 2.4m PVT-291-2045
Sample Number		23-25485	23-25486
Leachate used	WLAB075	Distilled Water	Distilled Water
Mass Used (g)	---	500	500
Volume Used (mℓ)	---	1000	1000
pH Value at 25°C	WLAB001	6.1	6.6
pHs Value at 20°C (calc)	WLAB053b	10.4	9.9
Electrical Conductivity in mS/m at 25°C	WLAB002	1.4	8.4
Total Dissolved Solids (calc)	WLAB068	<10	5.6
Total Alkalinity as CaCO ₃	WLAB007	12	12
Total Hardness as CaCO ₃ (calc)	WLAB051b	7	26
Calcium Hardness as CaCO ₃ (calc)	WLAB051a	2	10
Calcium as Ca	WLAB015	<1	4
Magnesium as Mg	WLAB015	<1	4
Free & Saline Ammonia	WLAB046	<0.1	<0.1
Ammonium as NH ₄ (calc)	WLAB068	<0.3	<0.3
Sulphate as SO ₄	WLAB046	<2	23
Chloride as Cl	WLAB046	2	2
Langelier Index at 20°C (calc)	WLAB053c	-4.3	-3.3
Ryznar Index at 20°C (calc)	WLAB053d	14.7	13.2
Corrosivity Ratio (calc)	WLAB054	0.3	2.2
Leaching Index [LCSI] (calc)*	---	3462	2740
Spalling Index [SCSI] (calc)*	---	1	4
Aggressiveness Index [N _c] (calc)*	---	3463	2744

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Contact person: Thinus Hofsink

Email: thinus@stlab.co.za

Cell: 082 309 4448 (Gerrie)

Analyses in mg/ℓ (Unless specified otherwise)	Method Identification	Sample Identification:	
		KTP27 0.2 - 2.5m PVT-291-2048	KTP34 0.4 - 2.1m PVT-291-2055
Sample Number		23-25487	23-25488
Leachate used	WLAB075	Distilled Water	Distilled Water
Mass Used (g)	---	500	500
Volume Used (mℓ)	---	1000	1000
pH Value at 25°C	WLAB001	6.4	6.5
pHs Value at 20°C (calc)	WLAB053b	9.3	9.5
Electrical Conductivity in mS/m at 25°C	WLAB002	14.3	8.6
Total Dissolved Solids (calc)	WLAB068	96	58
Total Alkalinity as CaCO ₃	WLAB007	16	20
Total Hardness as CaCO ₃ (calc)	WLAB051b	61	31
Calcium Hardness as CaCO ₃ (calc)	WLAB051a	32	15
Calcium as Ca	WLAB015	13	6
Magnesium as Mg	WLAB015	7	4
Free & Saline Ammonia	WLAB046	<0.1	<0.1
Ammonium as NH ₄ (calc)	WLAB068	<0.3	<0.3
Sulphate as SO ₄	WLAB046	44	23
Chloride as Cl	WLAB046	3	3
Langelier Index at 20°C (calc)	WLAB053c	-2.9	-3.0
Ryznar Index at 20°C (calc)	WLAB053d	12.1	12.5
Corrosivity Ratio (calc)	WLAB054	3.1	1.4
Leaching Index [LCSI] (calc)*	---	2462	2546
Spalling Index [SCSI] (calc)*	---	6	4
Aggressiveness Index [N _c] (calc)*	---	2468	2549

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Cell: 082 309 4448 (Gerrie)

Analyses in mg/ℓ (Unless specified otherwise)	Method Identification	Sample Identification:	
		KTP38 0.4 - 2.7m PVT-291-2059	KTP40 0.1 - 1.1m PVT-291-2061
Sample Number		23-25489	23-25490
Leachate used	WLAB075	Distilled Water	Distilled Water
Mass Used (g)	---	500	500
Volume Used (mℓ)	---	1000	1000
pH Value at 25°C	WLAB001	6.3	7.5
pHs Value at 20°C (calc)	WLAB053b	9.9	8.6
Electrical Conductivity in mS/m at 25°C	WLAB002	12.7	27.6
Total Dissolved Solids (calc)	WLAB068	85	185
Total Alkalinity as CaCO ₃	WLAB007	12	52
Total Hardness as CaCO ₃ (calc)	WLAB051b	26	66
Calcium Hardness as CaCO ₃ (calc)	WLAB051a	10	50
Calcium as Ca	WLAB015	4	20
Magnesium as Mg	WLAB015	4	4
Free & Saline Ammonia	WLAB046	<0.1	<0.1
Ammonium as NH ₄ (calc)	WLAB068	<0.3	<0.3
Sulphate as SO ₄	WLAB046	47	75
Chloride as Cl	WLAB046	3	3
Langelier Index at 20°C (calc)	WLAB053c	-3.6	-1.1
Ryznar Index at 20°C (calc)	WLAB053d	13.5	9.7
Corrosivity Ratio (calc)	WLAB054	4.4	1.6
Leaching Index [LCSI] (calc)*	---	2972	1195
Spalling Index [SCSI] (calc)*	---	6	9
Aggressiveness Index [N _c] (calc)*	---	2978	1204

Please note:

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Telephone: ---

Contact person: Thinus Hofsink

Email: thinus@stlab.co.za

Cell: 082 309 4448 (Gerrie)

Analyses in mg/ℓ (Unless specified otherwise)	Method Identification	Sample Identification:
		KTP45 0.1 - 2.2m PVT-291-2066
Sample Number		23-25491
Leachate used	WLAB075	Distilled Water
Mass Used (g)	---	500
Volume Used (mℓ)	---	1000
pH Value at 25°C	WLAB001	5.4
pHs Value at 20°C (calc)	WLAB053b	10.0
Electrical Conductivity in mS/m at 25°C	WLAB002	15.6
Total Dissolved Solids (calc)	WLAB068	105
Total Alkalinity as CaCO ₃	WLAB007	<5
Total Hardness as CaCO ₃ (calc)	WLAB051b	55
Calcium Hardness as CaCO ₃ (calc)	WLAB051a	22
Calcium as Ca	WLAB015	9
Magnesium as Mg	WLAB015	8
Free & Saline Ammonia	WLAB046	<0.1
Ammonium as NH ₄ (calc)	WLAB046	<0.3
Sulphate as SO ₄	WLAB046	51
Chloride as Cl	WLAB046	5
Langelier Index at 20°C (calc)	WLAB053c	-4.6
Ryznar Index at 20°C (calc)	WLAB053d	14.7
Corrosivity Ratio (calc)	WLAB054	15
Leaching Index [LCSI] (calc)*	---	3713
Spalling Index [SCSI] (calc)*	---	7
Aggressiveness Index [N _c] (calc)*	---	3720

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Tel: +2712 – 349 – 1066
Fax: +2712 – 349 – 2064
e-mail: admin@waterlab.co.za



T0391

CERTIFICATE OF ANALYSES **BASSON INDEX**

Date received: 2023-10-18

Project number: 1000

Report number: 125716

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Order number: T 050

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Telephone: ---

Contact person: Thinus Hofsink

Email: thinus@stlab.co.za

Cell: 082 309 4448 (Gerrie)

Important notes (see table for corrections on p.8):

1. The above aggressiveness index is only applicable for conditions of laminar flow at a mean annual temperature of 20°C.
2. For stagnant/turbulent conditions the aggressiveness index must be corrected.
3. For wet/dry cycling conditions (for example in tidal zones) the aggressiveness index must be corrected.
4. For mean annual temperatures lower/higher than 20°C the aggressiveness index must be corrected.

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Guidelines for assessing overall aggressiveness (N_c):

N_c	Aggressiveness
Not greater than 300	None to mild
400-700	Mild to moderate
800-1000	High
= or > 1 100	Very high

Aggressiveness Towards Concrete and Fibre Cement Pipes

Index	Aggressive	Neutral	Non- Aggressive
a) Stability pH (pHs)	> pH	= pH	<pH
b) Langelier Index	Neg. Value	Zero	Pos. Value
c) Ryznar Index	>7.5	6-7	<6

Corrosiveness Towards metals

Corrosivity	>0.2
-------------	------

Sample Name	Sample Number	Corrosivity Indices	Basson Index
KTP6 0.0 - 1.0m PVT-291-2031	23-25483	Corrosive	Aggressive
KTP11 0.7 - 2.4m PVT-291-2034	23-25484	Corrosive	Aggressive
KTP17 0.2 - 2.2m PVT-291-2039	23-25485	Corrosive	Aggressive
KTP24 0.2 - 2.4m PVT-291-2045	23-25486	Corrosive	Aggressive
KTP27 0.2 - 2.5m PVT-291-2048	23-25487	Corrosive	Aggressive
KTP34 0.4 - 2.1m PVT-291-2055	23-25488	Corrosive	Aggressive
KTP38 0.4 - 2.7m PVT-291-2059	23-25489	Corrosive	Aggressive
KTP40 0.1 - 1.1m PVT-291-2061	23-25490	Corrosive	Aggressive
KTP45 0.1 - 2.2m PVT-291-2066	23-25491	Corrosive	Aggressive

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To correct for:	Multiply	By: (see Notes 2 to 5 below)
Turbulence	LCSI	1.75
Stagnance	LCSI	0.5
Temperature	LCSI, SCSi, N7 Where N7=0.2 x CI in mg/l	(1+ [0.05 x (T-20)])
Wet-dry cycles	SCSi	0.23 x 10 ⁻⁶ x TDS x DTF x CPA Where: DTF = Dry Time Fraction CPA = wet-dry cycles per annum

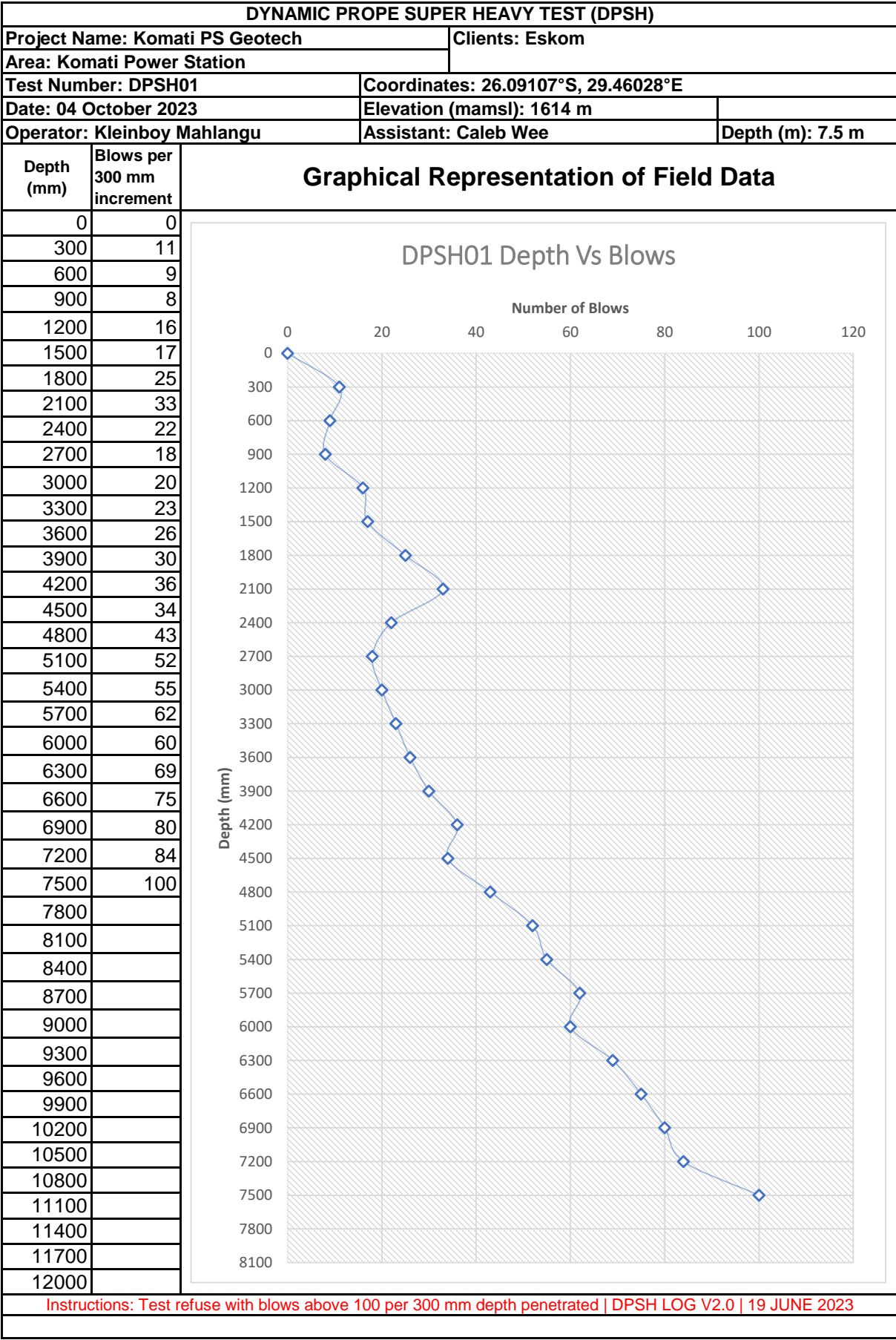
Note 1: Only if the concrete contains embedded steel.
Note 2: To preserve the correct logical relationships when dealing with the negative sub-indices (i.e. LCSI or SCSi having minus values) they should be multiplied by the reciprocal of the relevant factor indicated in this column
Note 3: If more than one correction is required, multiply by the product of the individual correction factors
Note 4: Use subscript c to indicate that the index has been corrected, e.g. for turbulent conditions LCSI_c = LCSI x 1.75
Note 5: Round off corrected indices to the nearest 100.

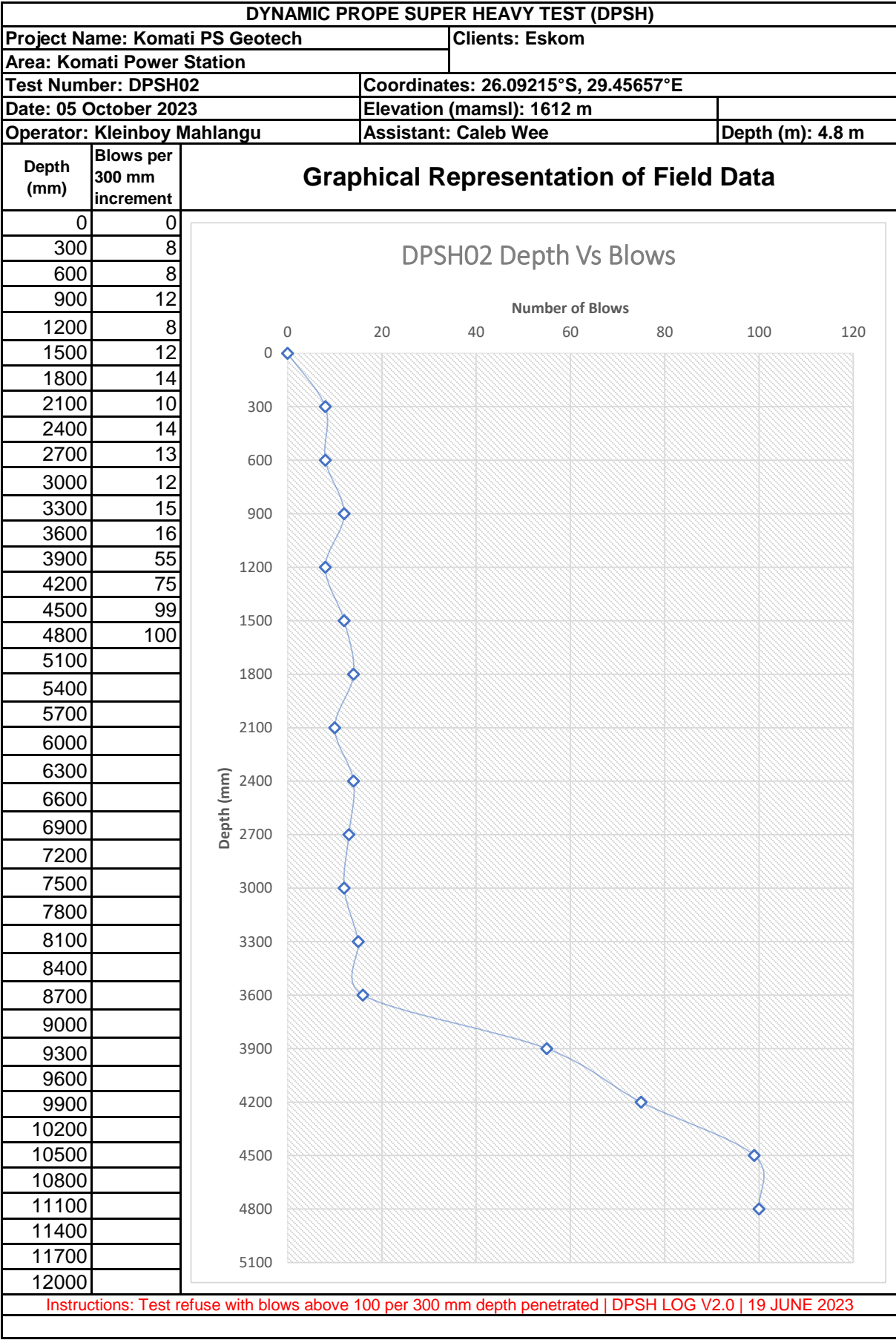
S. Laubscher

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APPENDIX D: DYNAMIC PROBING TEST RESULTS (DPSH)



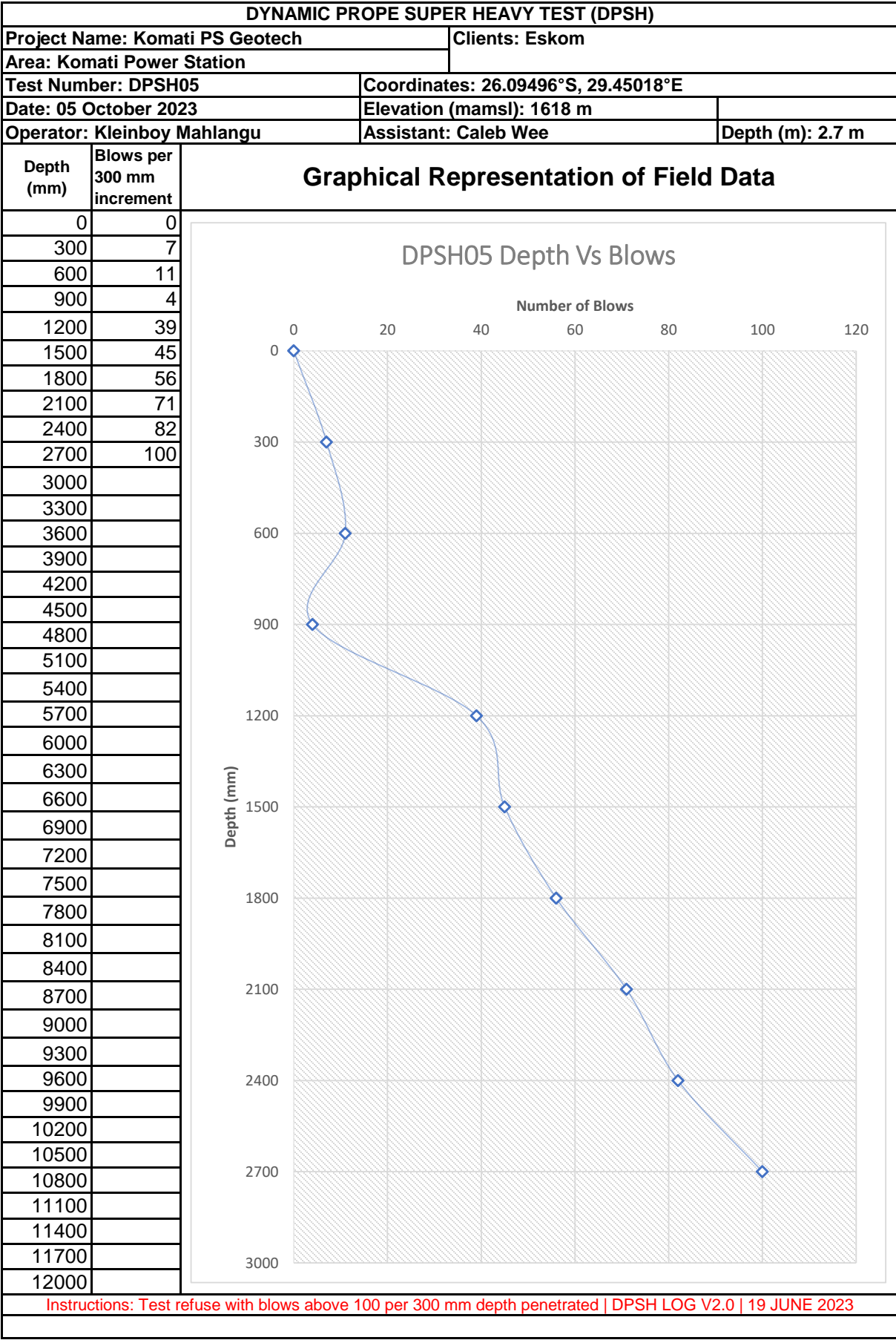


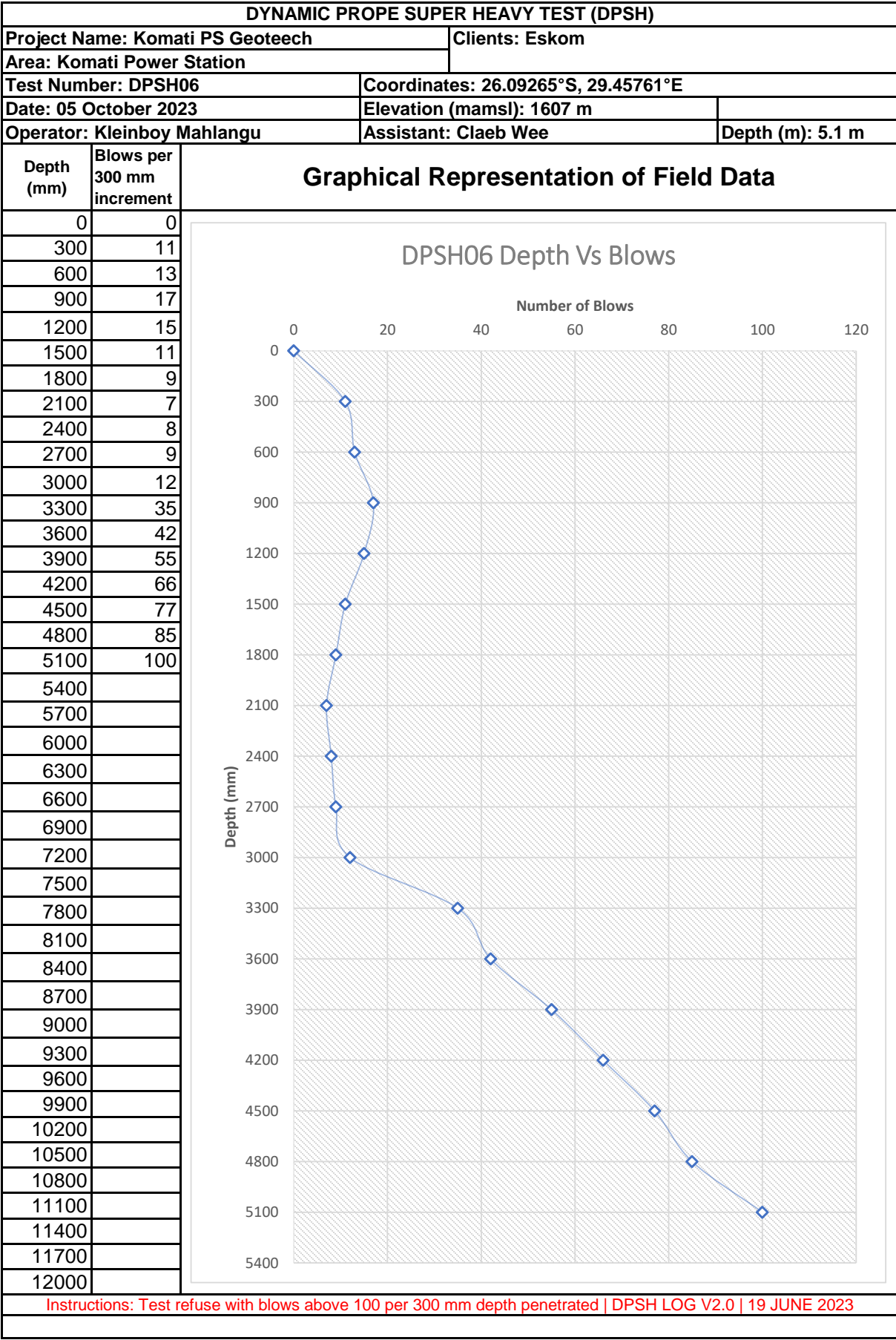
DYNAMIC PROPE SUPER HEAVY TEST (DPSH)		
Project Name: Komati PS Geotech		Clients: Eskom
Area: Komati Power Station		
Test Number: DPSH03		Coordinates: 26.09378°S, 29.45280°E
Date: 05 October 2023		Elevation (mamsl): 1617 m
Operator: Kleinboy Mahlangu		Assistant: Caleb Wee
		Depth (m): 1.8 m
Depth (mm)	Blows per 300 mm increment	<div>Graphical Representation of Field Data</div> <div><div>DPSH03 Depth Vs Blows</div><div><div>Number of Blows</div><div><div>020406080100120</div><div>03006009001200150018002100</div><div>Depth (mm)</div></div></div></div>
0	0	
300	7	
600	62	
900	71	
1200	80	
1500	90	
1800	100	
2100		
2400		
2700		
3000		
3300		
3600		
3900		
4200		
4500		
4800		
5100		
5400		
5700		
6000		
6300		
6600		
6900		
7200		
7500		
7800		
8100		
8400		
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9300		
9600		
9900		
10200		
10500		
10800		
11100		
11400		
11700		
12000		

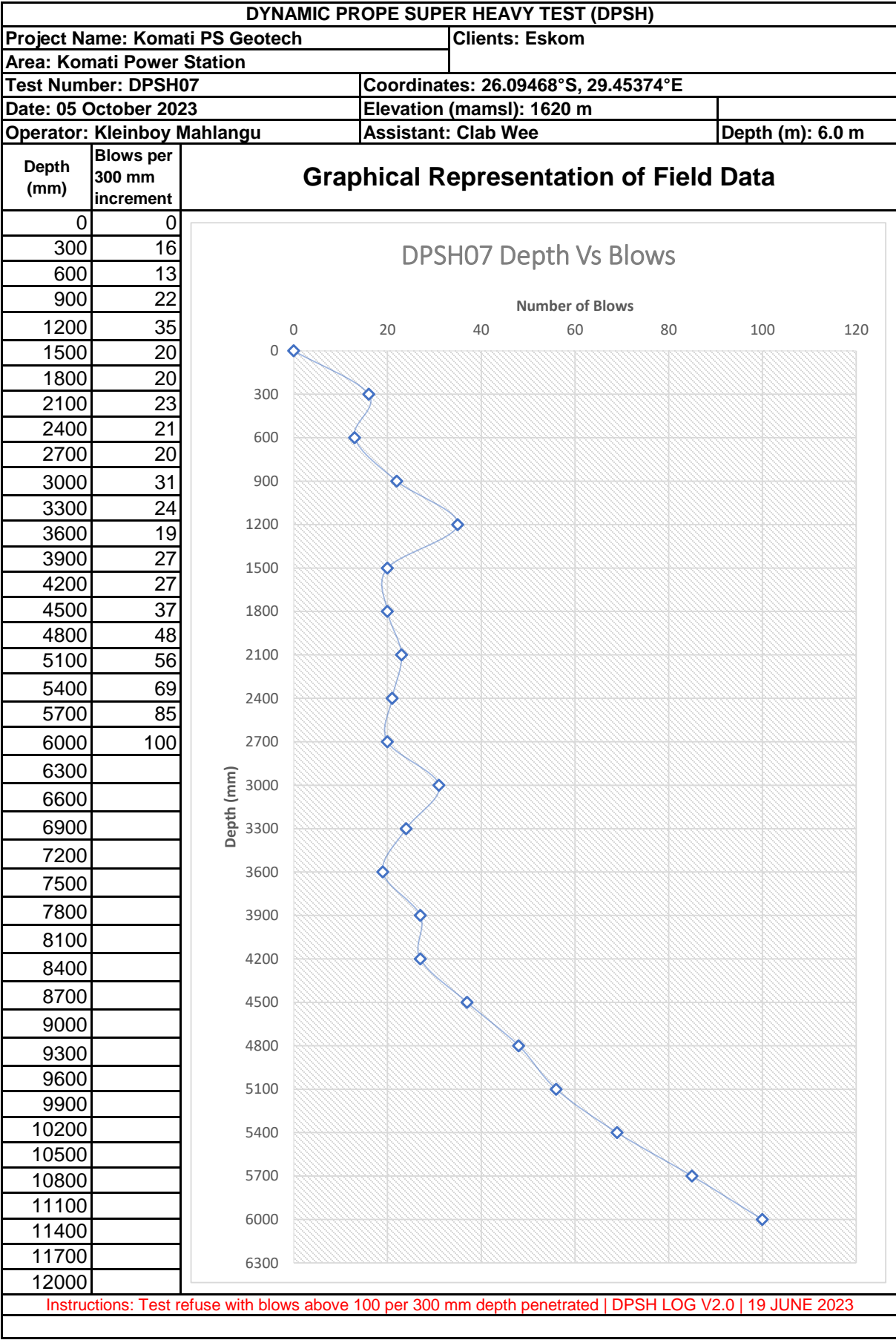
Instructions: Test refuse with blows above 100 per 300 mm depth penetrated | DPSH LOG V2.0 | 19 JUNE 2023

DYNAMIC PROPE SUPER HEAVY TEST (DPSH)																																																						
Project Name: Komati PS Geotech		Clients: Eskom																																																				
Area: Komati Power Station																																																						
Test Number: DPSH04		Coordinates: 26.09321°S, 29.45023°E																																																				
Date: 05 October 2023		Elevation (mamsl): 1616 m																																																				
Operator: Kleinboy Mahlangu		Assistant: Caleb Wee																																																				
		Depth (m): 7.2 m																																																				
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Instructions: Test refuse with blows above 100 per 300 mm depth penetrated | DPSH LOG V2.0 | 19 JUNE 2023

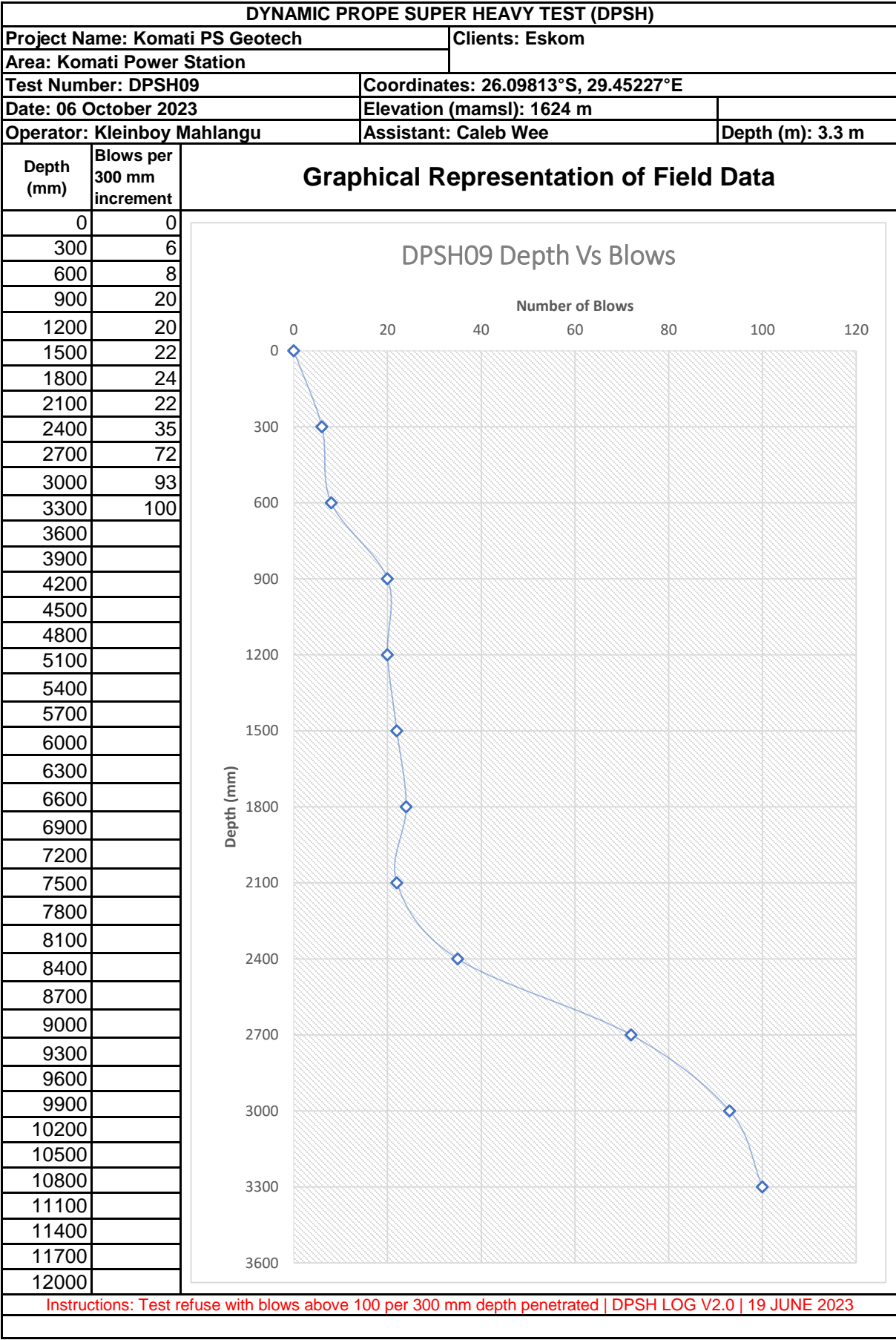






DYNAMIC PROPE SUPER HEAVY TEST (DPSH)																																																																																						
Project Name: Komati PS Geotech		Clients: Eskom																																																																																				
Area: Komati Power Station																																																																																						
Test Number: DPSH08		Coordinates: 22.09339°S, 29.45657°E																																																																																				
Date: 05 October 2023		Elevation (mamsl): 1616 m																																																																																				
Operator: Kleinboy Mahlangu		Assistant: Caleb Wee																																																																																				
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Project Name: Komati PS Geotch		Clients: Eskom																																																																																				
Area: Komati Power Station																																																																																						
Test Number: DPSH10		Coordinates: 26.10127°S, 29.45122°E																																																																																				
Date: 06 October 2023		Elevation (mamsl): 1632 m																																																																																				
Operator: Kleinboy Mahlangu		Assistant: Caleb Wee																																																																																				
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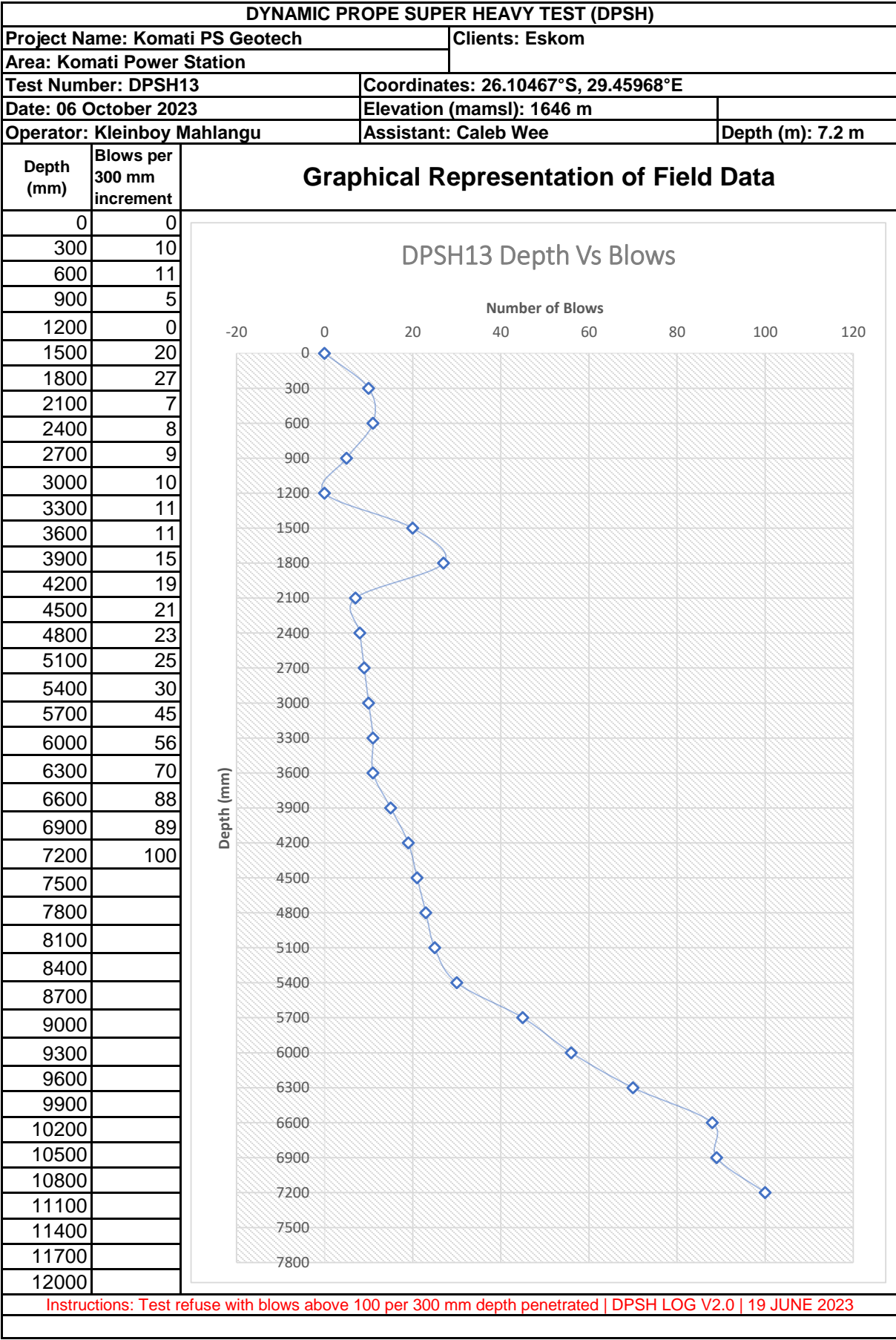
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Project Name: Komati PS Geotech		Clients: Eskom																																																																																																				
Area: Komati Power Station																																																																																																						
Test Number: DPSH11		Coordinates: 26.10517°S, 29.45193°E																																																																																																				
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Operator: Kleinboy Mahlangu		Assistant: Caleb Wee																																																																																																				
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Instructions: Test refuse with blows above 100 per 300 mm depth penetrated | DPSH LOG V2.0 | 19 JUNE 2023

DYNAMIC PROPE SUPER HEAVY TEST (DPSH)																				
Project Name: Komati PS Geotech		Clients: Eskom																		
Area: Komati Power Station																				
Test Number: DPSH12		Coordinates: 26.10551°S, 29.45532°E																		
Date: 06 October 2023		Elevation (mamsl): 1648 m																		
Operator: Kleinboy Mahlangu		Assistant: Caleb Wee																		
		Depth (m): 2.1 m																		
Depth (mm)	Blows per 300 mm increment	<div>Graphical Representation of Field Data</div> <div><div>DPSH12 Depth Vs Blows</div><table><tr><th>Depth (mm)</th><th>Number of Blows</th></tr><tr><td>0</td><td>0</td></tr><tr><td>300</td><td>6</td></tr><tr><td>600</td><td>5</td></tr><tr><td>900</td><td>6</td></tr><tr><td>1200</td><td>7</td></tr><tr><td>1500</td><td>40</td></tr><tr><td>1800</td><td>86</td></tr><tr><td>2100</td><td>100</td></tr></table></div>	Depth (mm)	Number of Blows	0	0	300	6	600	5	900	6	1200	7	1500	40	1800	86	2100	100
Depth (mm)	Number of Blows																			
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Instructions: Test refuse with blows above 100 per 300 mm depth penetrated | DPSH LOG V2.0 | 19 JUNE 2023

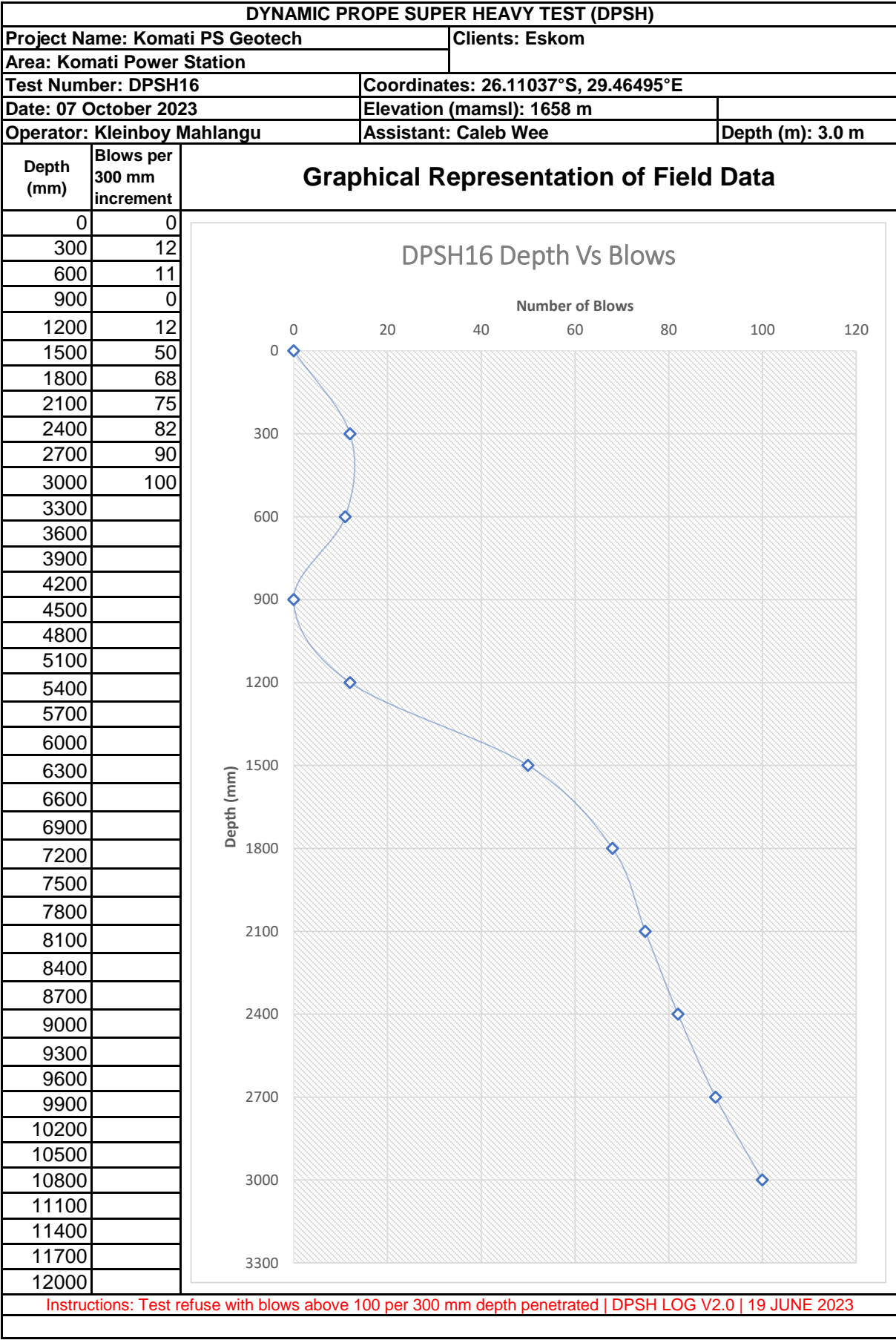


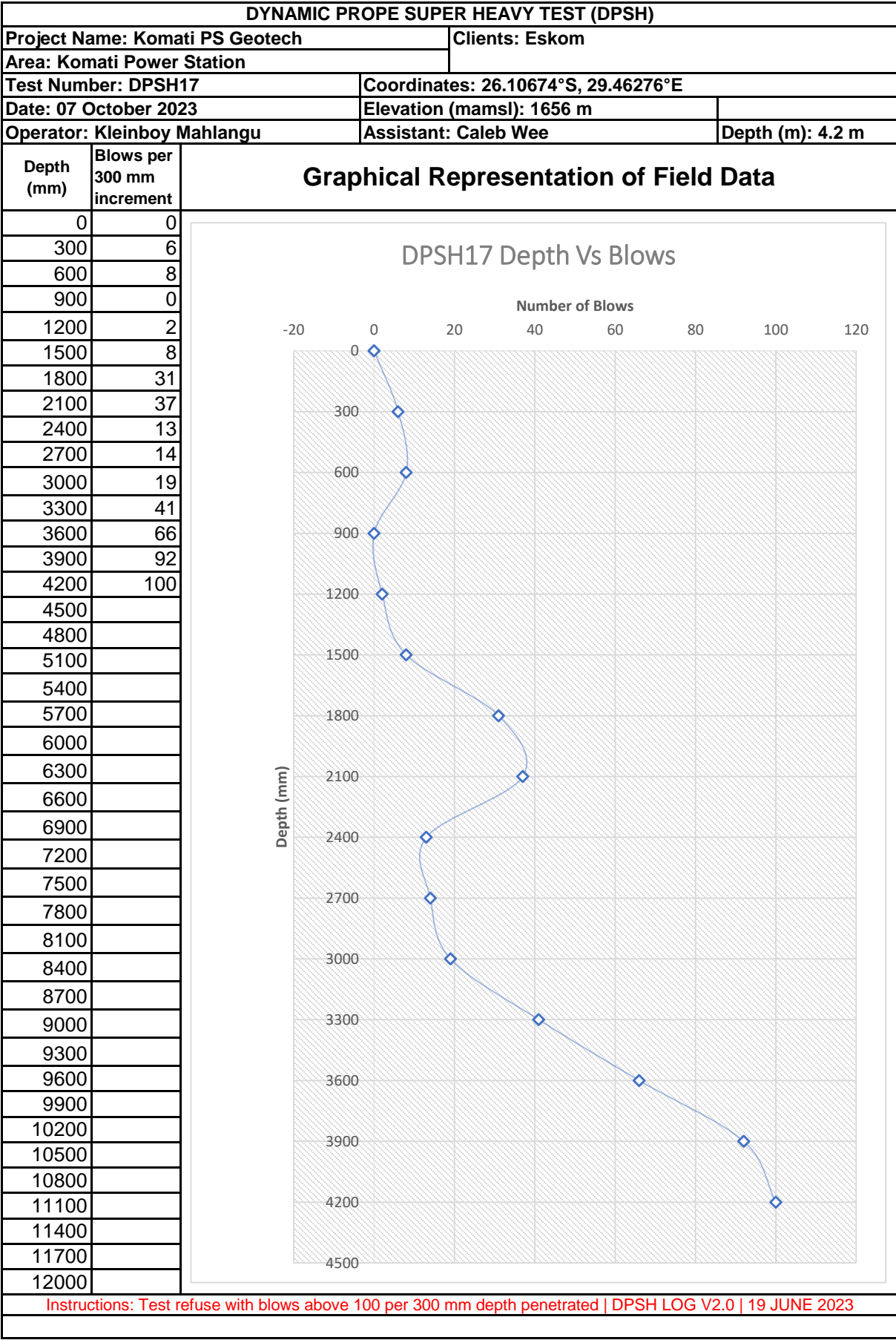
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Project Name: Komati PS Geotech		Clients: Eskom																																																																																				
Area: Komati Power Station																																																																																						
Test Number: DPSH14		Coordinates: 26.10458°S, 29.46482°E																																																																																				
Date: 06 October 2023		Elevation (mamsl): 1647 m																																																																																				
Operator: Kleinboy Mahlangu		Assistant: Caleb Wee																																																																																				
		Depth (m): 1.8 m																																																																																				
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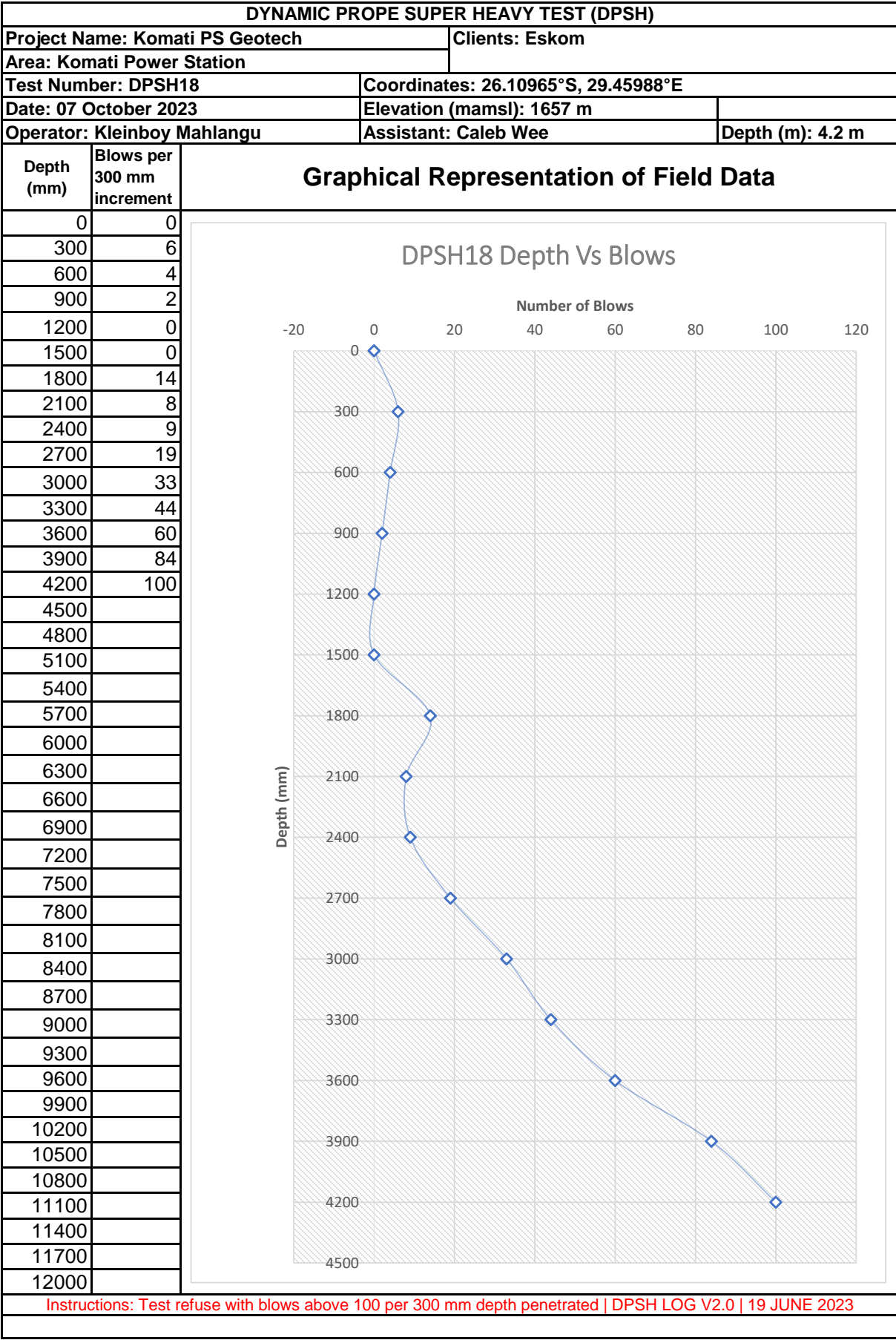
Instructions: Test refuse with blows above 100 per 300 mm depth penetrated | DPSH LOG V2.0 | 19 JUNE 2023

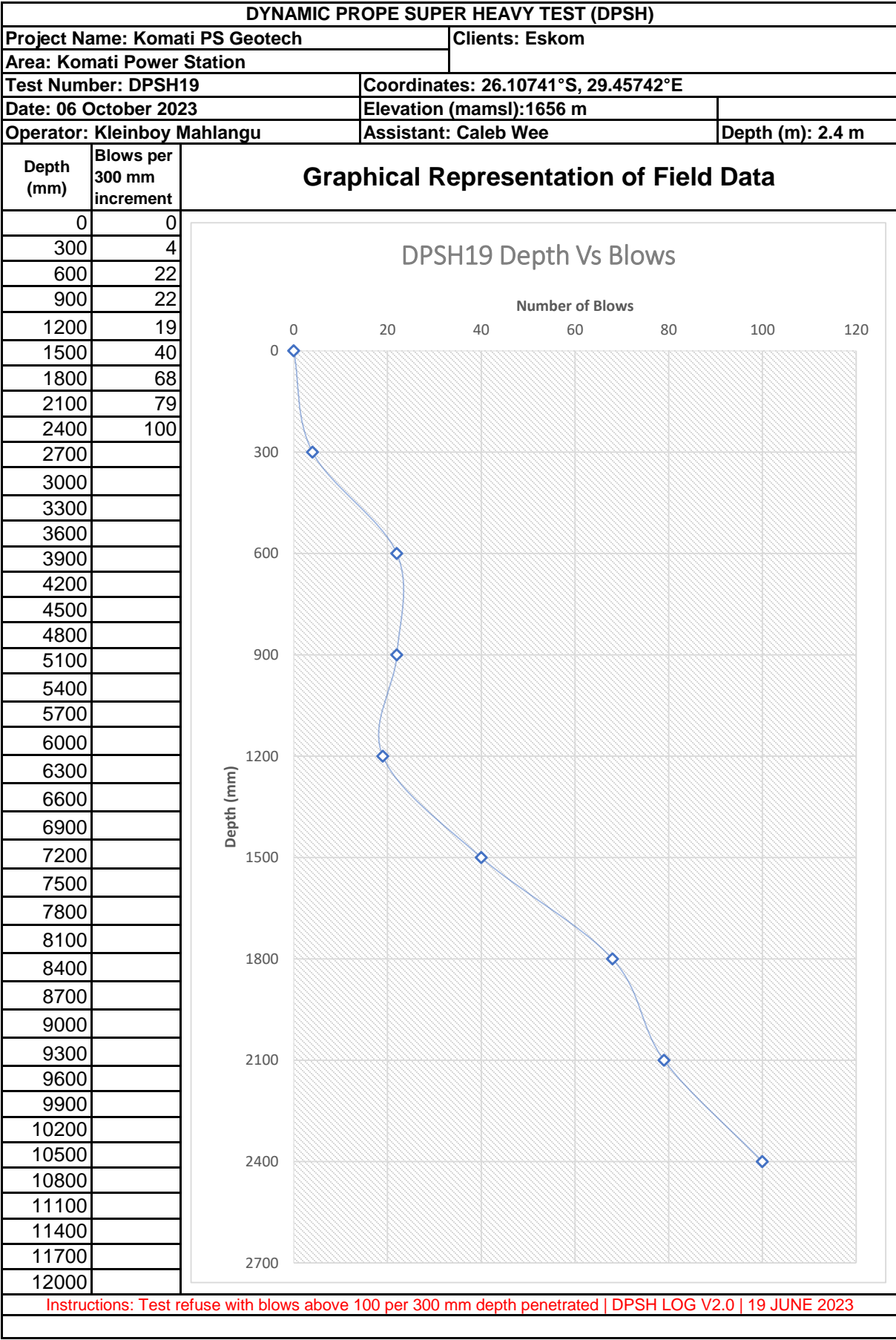
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Project Name: Komati PS Geotech		Clients: Eskom																																																						
Area: Komati Power Station																																																								
Test Number: DPSH15		Coordinates: 26.10772°S, 29.46569°E																																																						
Date: 07 October 2023		Elevation (mamsl): 1641 m																																																						
Operator: Kleinboy Mahlangu		Assistant: Caleb Wee																																																						
		Depth (m): 6.9 m																																																						
Depth (mm)	Blows per 300 mm increment	Graphical Representation of Field Data																																																						
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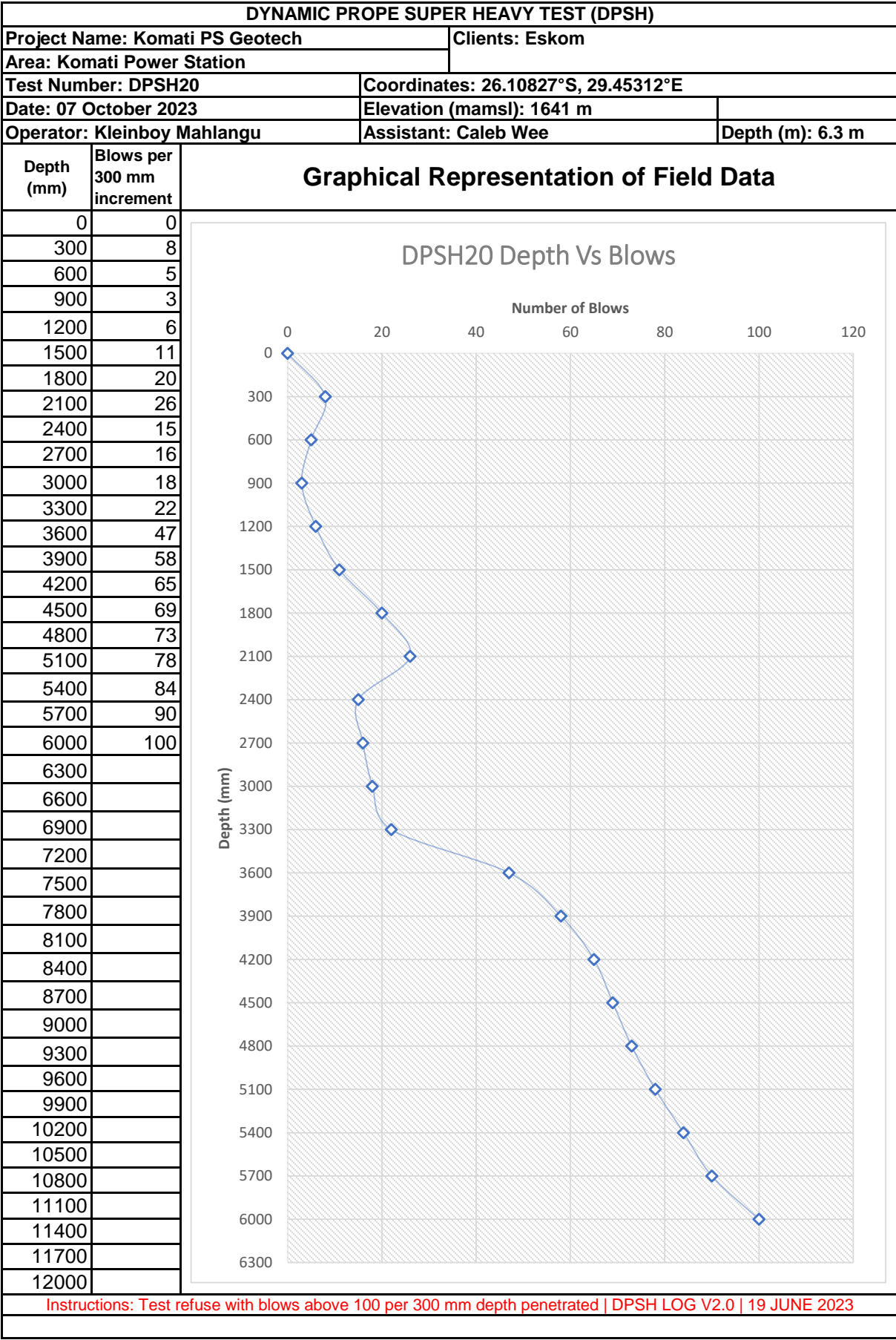
Instructions: Test refuse with blows above 100 per 300 mm depth penetrated | DPSH LOG V2.0 | 19 JUNE 2023

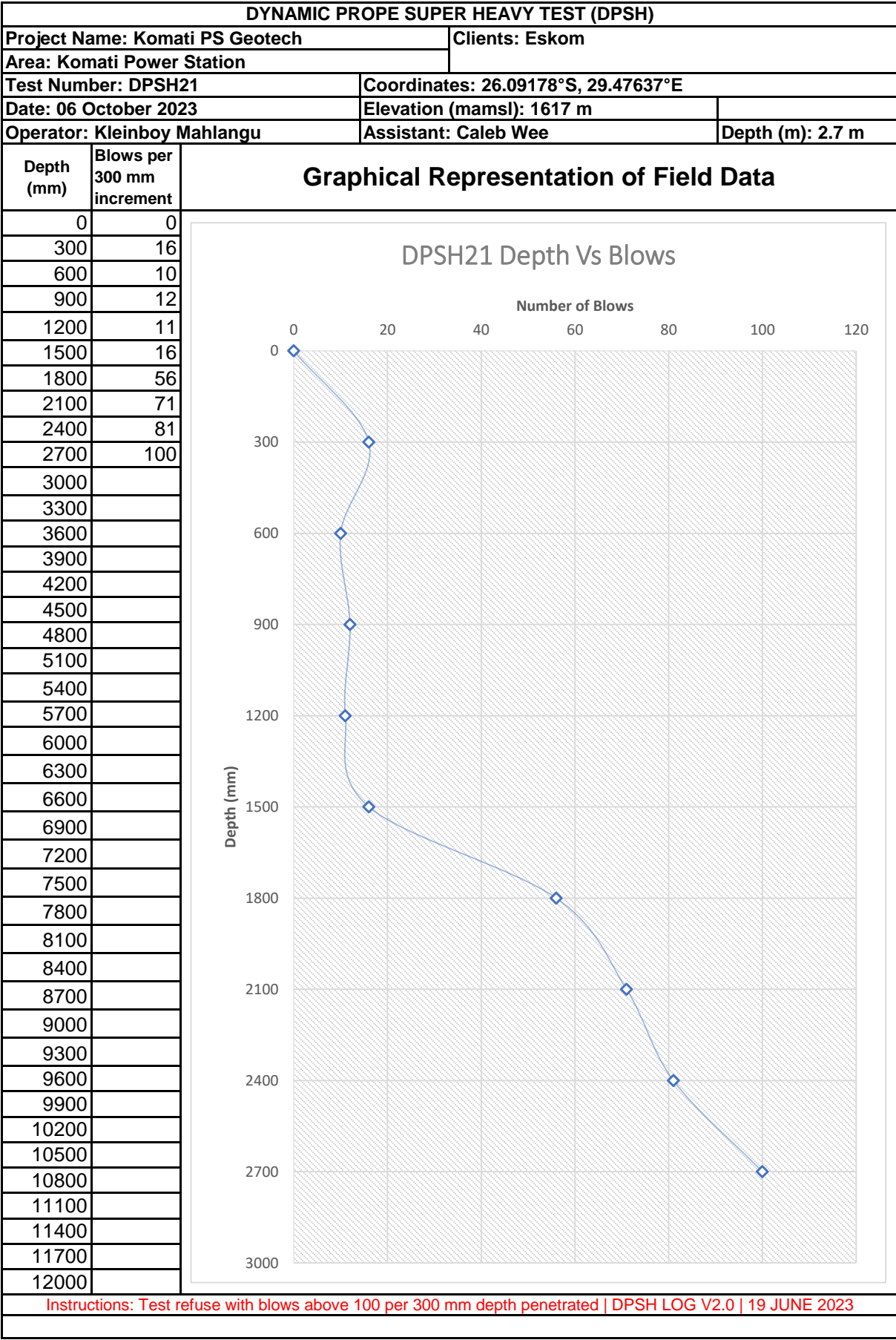


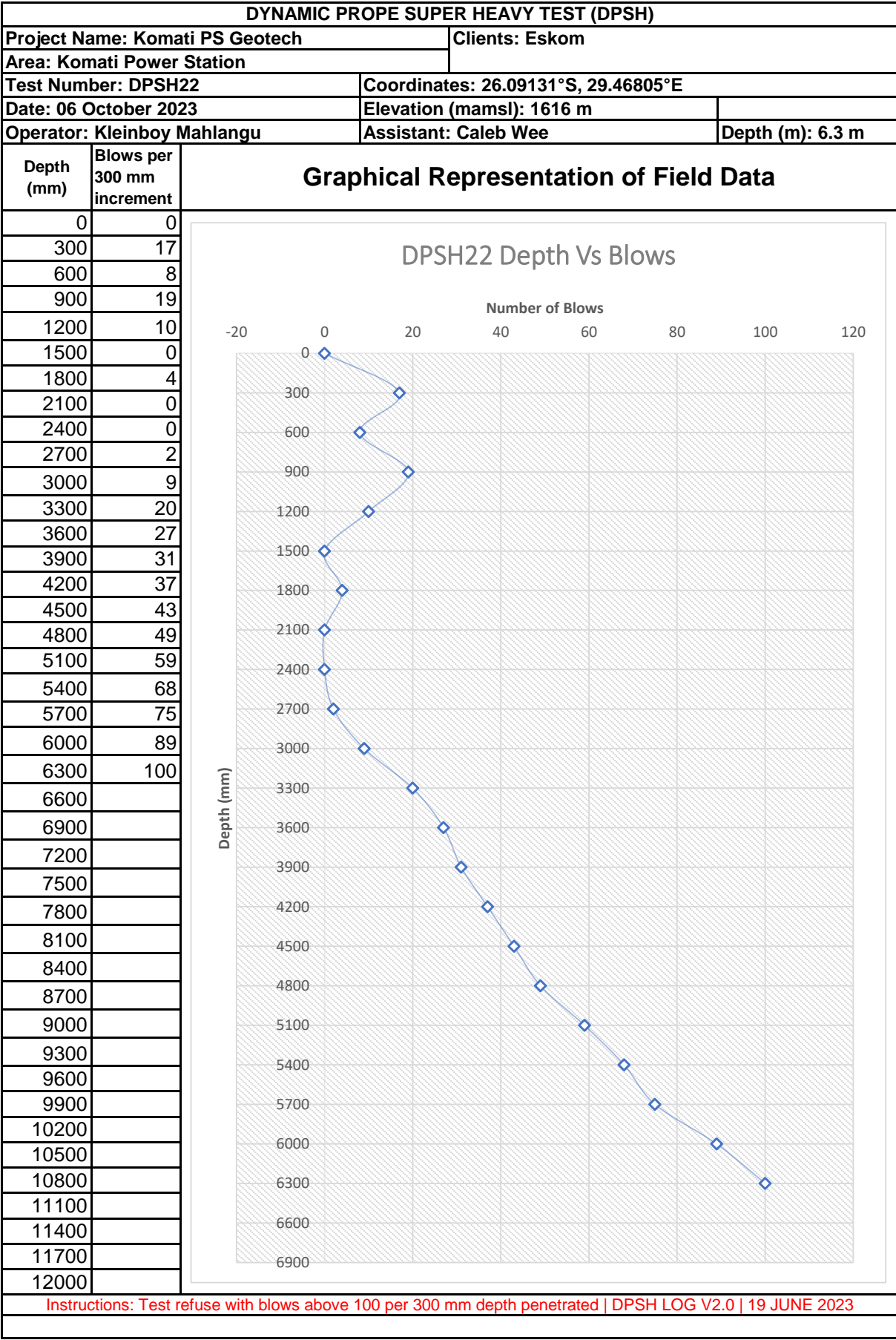


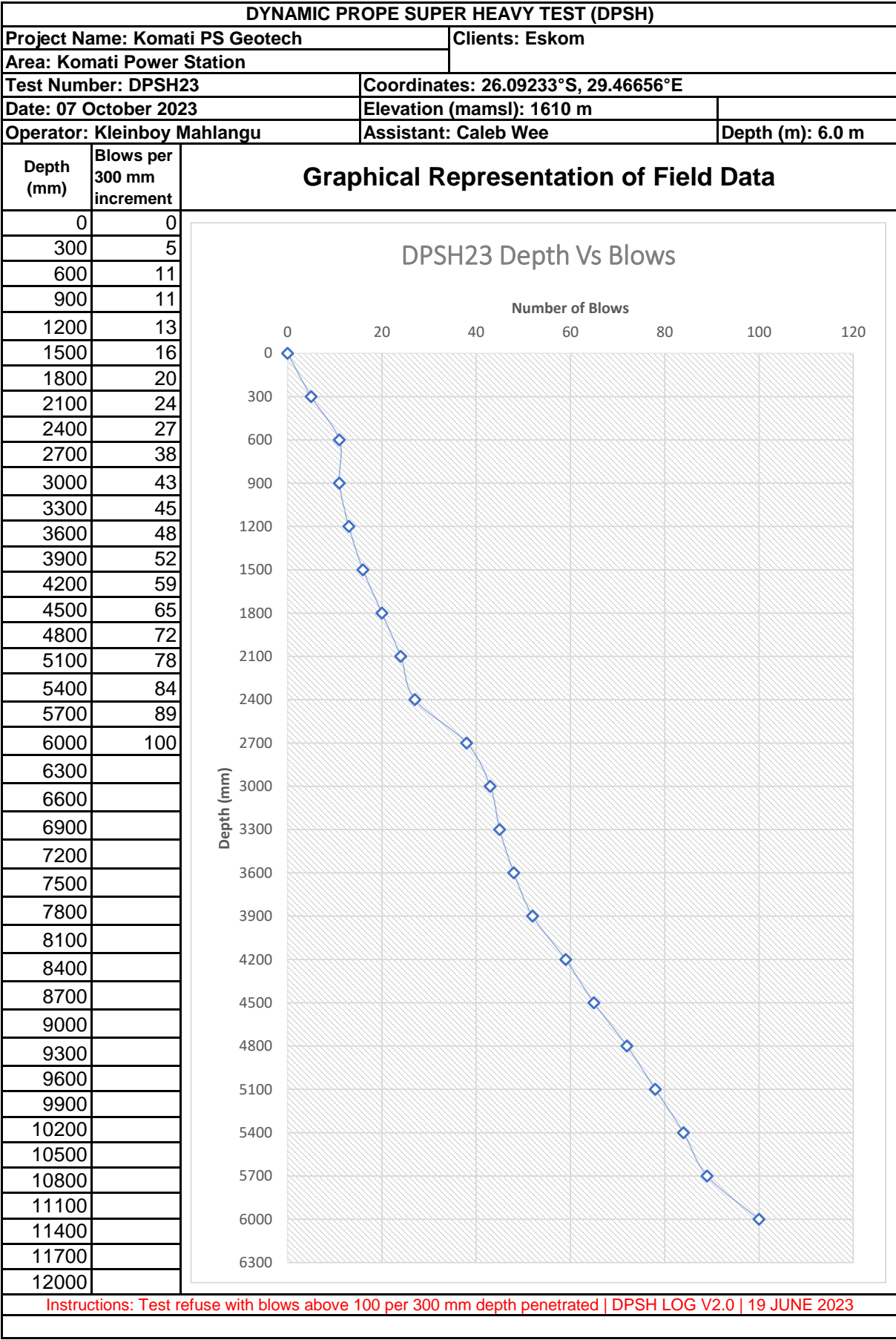




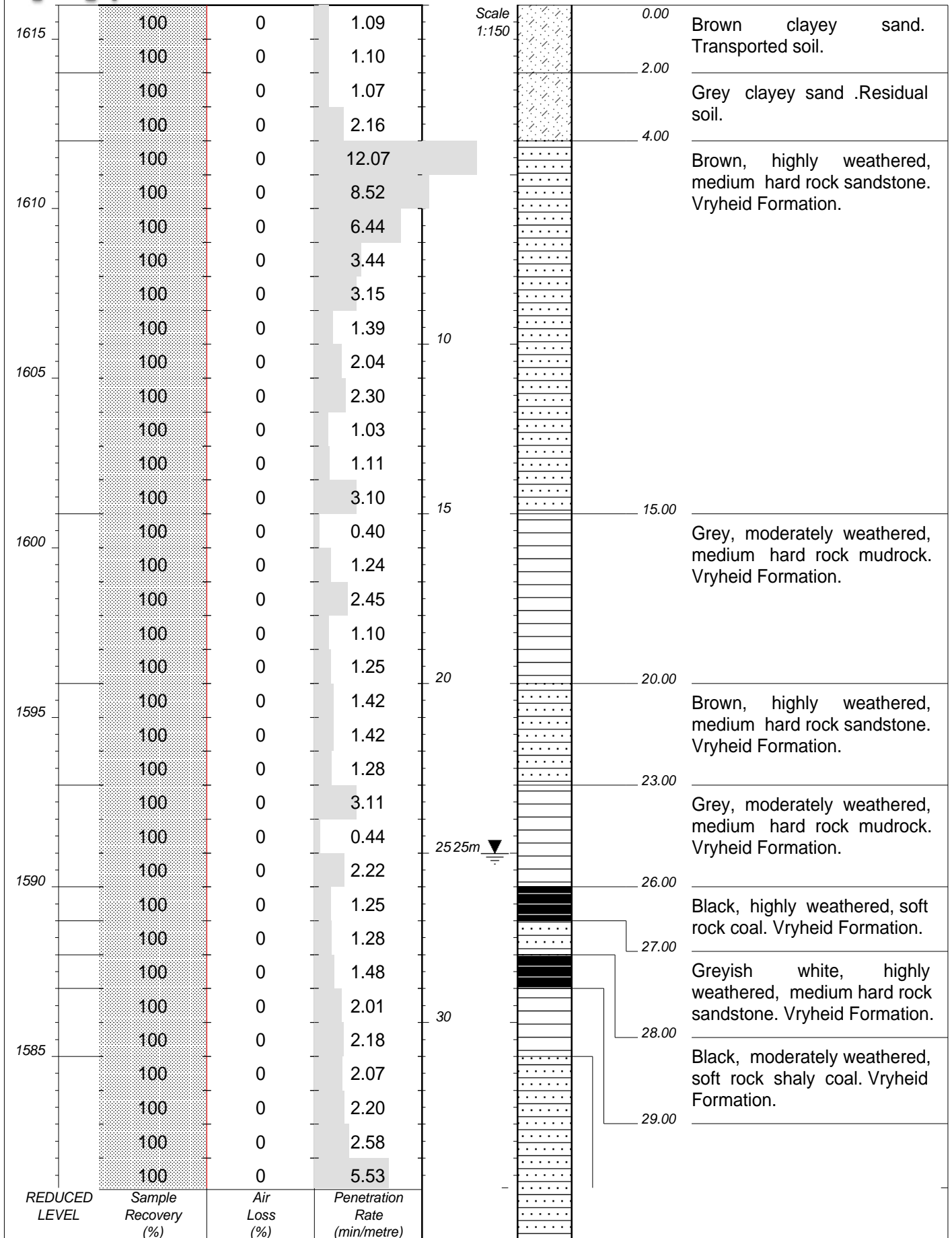


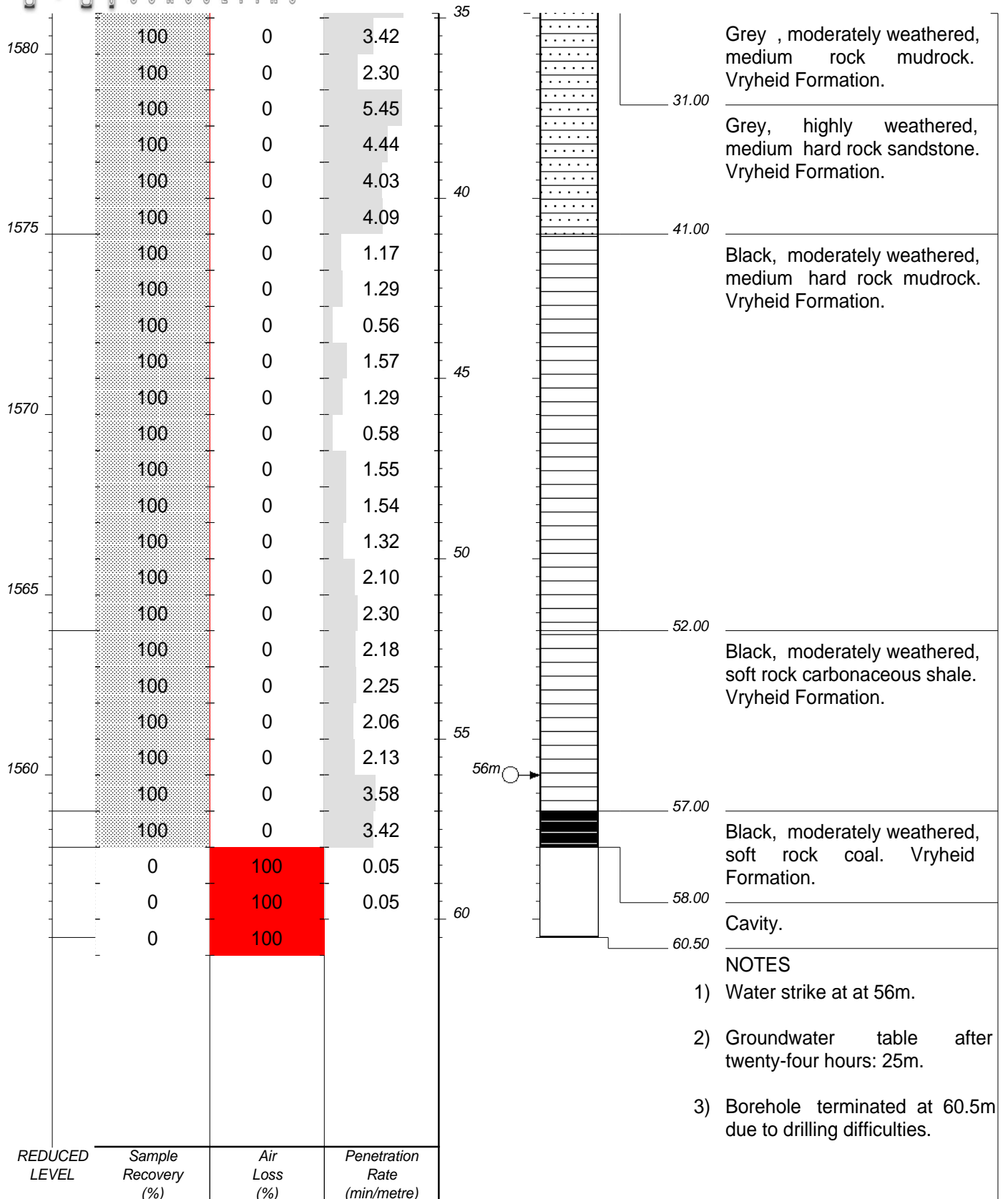






APPENDIX E: PERCUSSION DRILLING LOGS





CONTRACTOR : BAV Consulting
MACHINE : Thor 5000 Drill Rig
DRILLED BY : Zakhele Zaca
PROFIED BY : Andries Vukeya

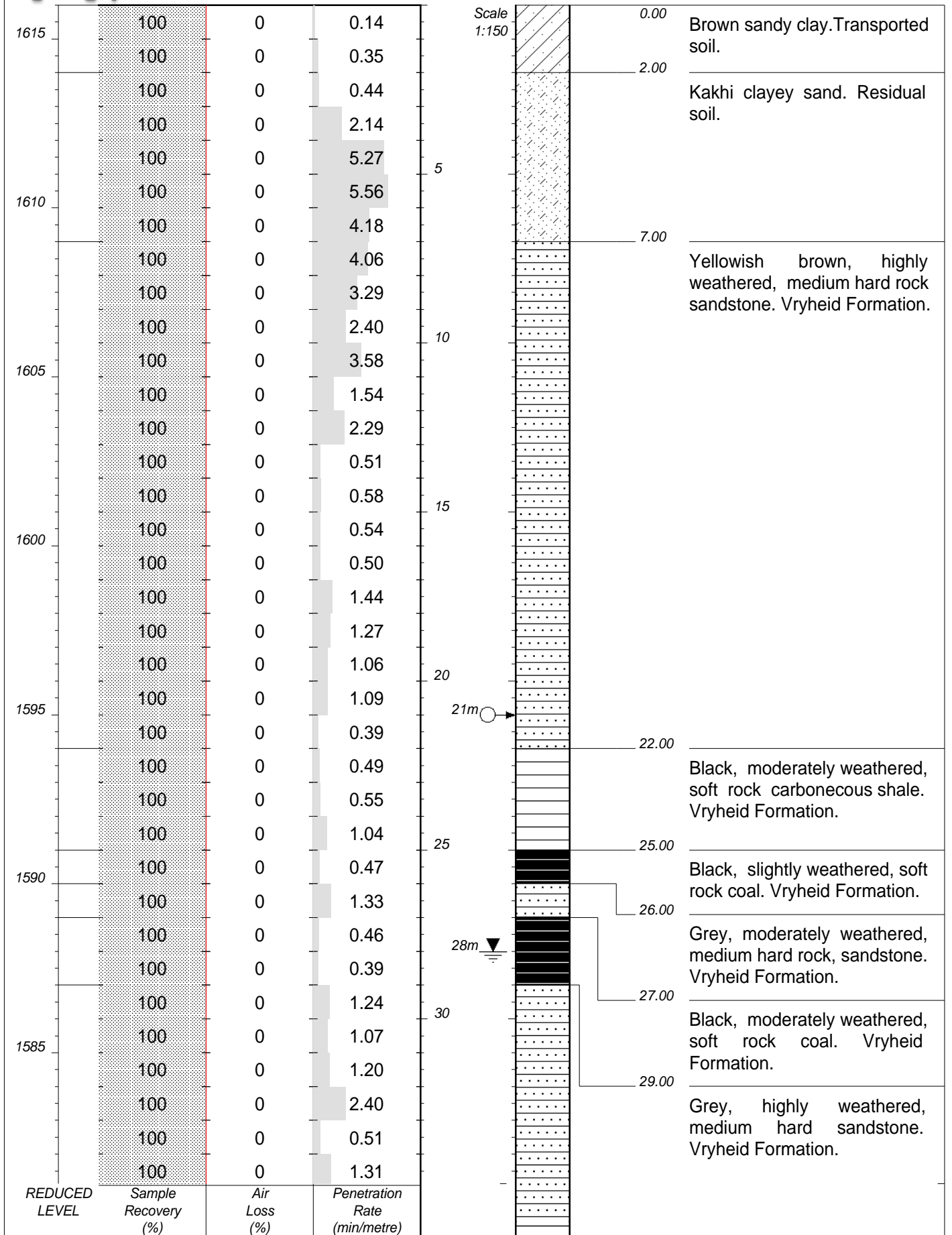
TYPE SET BY : Andries Vukeya
SETUP FILE : STANDARD.SET

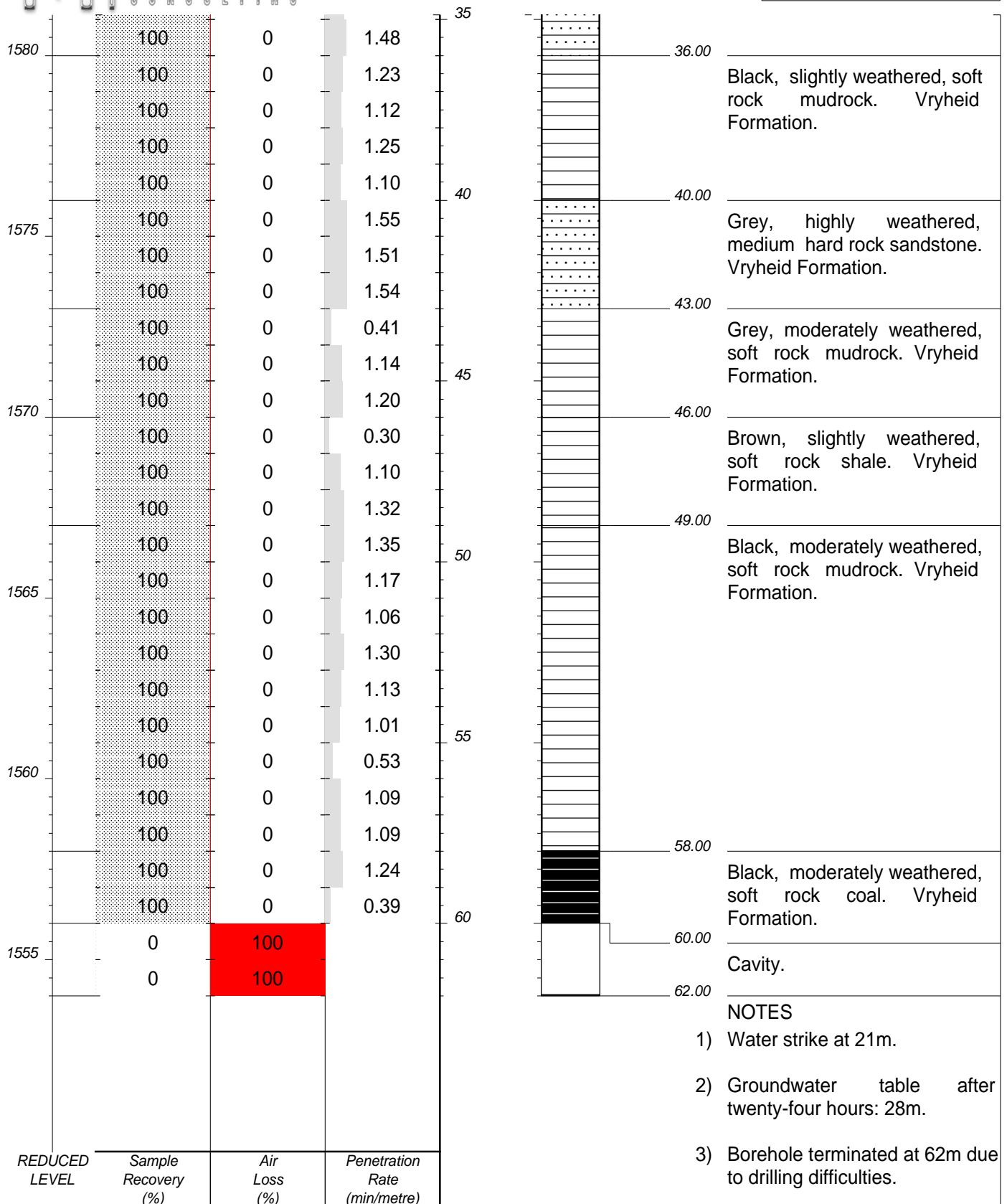
INCLINATION : Vertical
DIAM : 165 mm
DATE : 11/08/2023
DATE : 11/08/2023

DATE : 06/12/2023 13:53
TEXT : ..ussionBoreholesKBH1.TXT

ELEVATION : 1616 m
X-COORD : 29.452354°E
Y-COORD : 26.093280°S

HOLE No: K-BH1





CONTRACTOR : BAV Consulting
MACHINE : Thor 5000 Drill Rig
DRILLED BY : Zakhele Zaca
PROFIED BY : Andries Vukeya

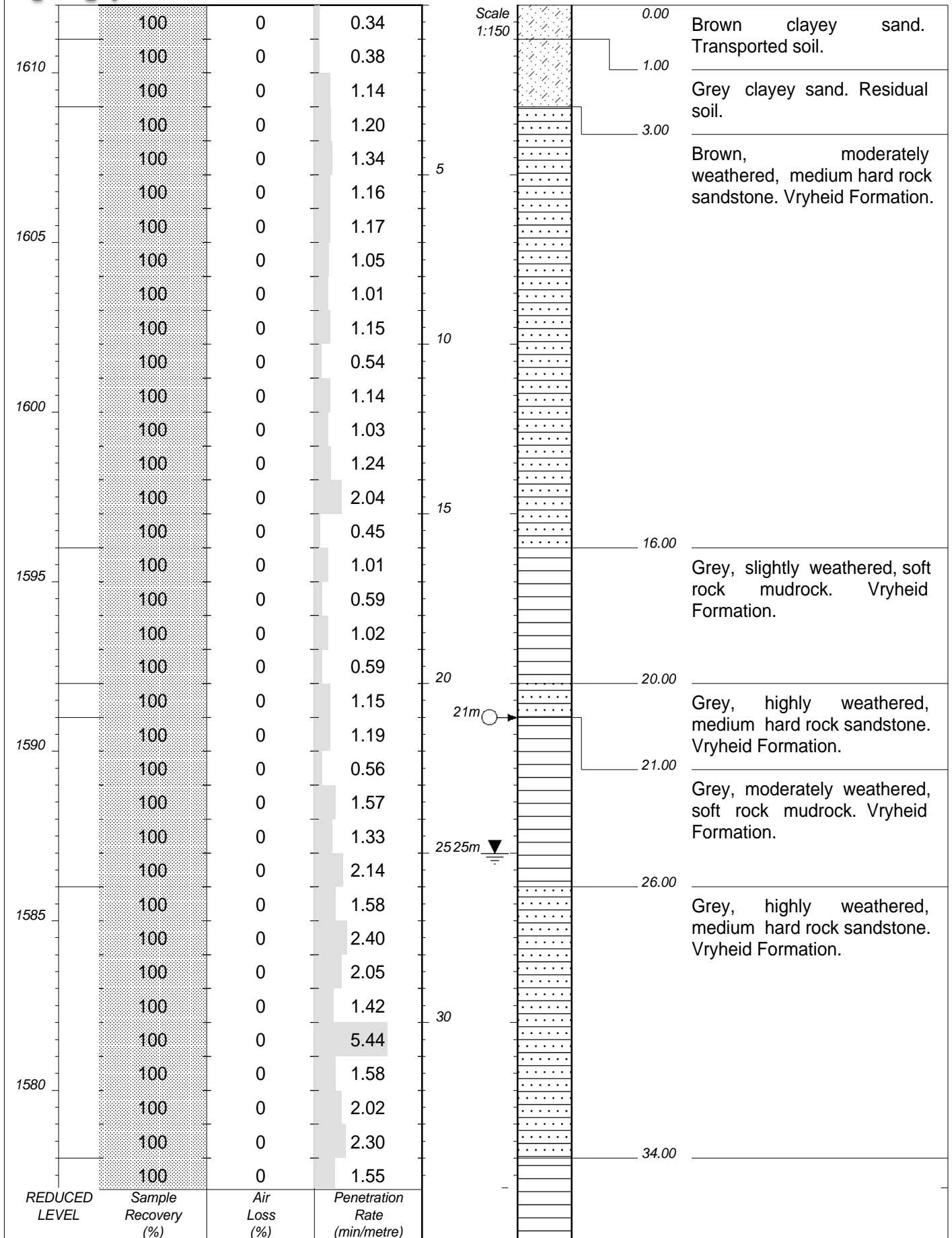
TYPE SET BY : Andries Vukeya
SETUP FILE : STANDARD.SET

INCLINATION : Vertical
DIAM : 165 mm
DATE : 31/08/2023
DATE : 31/08/2023

DATE : 06/12/2023 13:54
TEXT : ..ussionBoreholes\KBH2.TXT

ELEVATION : 1616 m
X-COORD : 29.45026°E
Y-COORD : 26.09327°S

HOLE No: K-BH2





**ESKOM
PRELIMINARY GEOTECHNICAL INVESTIGATION:
KOMATI POWER STATION**

HOLE No: K-BH3
Sheet 2 of 3

JOB NUMBER: MAK152.08.23

	100	0	2.44	35
1575	100	0	4.00	
	100	0	1.20	
	100	0	2.08	
	100	0	2.02	40
	100	0	1.57	
1570	100	0	1.38	
	100	0	1.38	
	100	0	1.47	
	100	0	2.30	45
	100	0	1.59	
1565	100	0	1.29	
	100	0	1.48	
	100	0	2.20	
	100	0	2.17	50
	100	0	1.53	
1560	100	0	1.43	
	100	0	1.55	
	100	0	1.16	
	100	0	1.52	55
	0	100		
1555	0	100		
	0	100		
REDUCED LEVEL	Sample Recovery (%)	Air Loss (%)	Penetration Rate (min/metre)	

Black, slightly weathered, soft
rock mudrock. Vryheid
Formation.

55.00

Cavity.

58.00

NOTES

- 1) Water strike at 21m.
- 2) Groundwater table after twenty-four hours: 25m.
- 3) Borehole terminated at 58m due to drilling difficulties.



ESKOM
PRELIMINARY GEOTECHNICAL INVESTIGATION:
KOMATI POWER STATION

HOLE No: K-BH3
Sheet 3 of 3

JOB NUMBER: MAK152.08.23

REDUCED LEVEL	Sample Recovery (%)	Air Loss (%)	Penetration Rate (min/metre)
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CONTRACTOR : BAV Consulting
MACHINE : Thor 5000 Drill Rig
DRILLED BY : Zakhele Zaca
PROFIED BY : Andries Vukeya

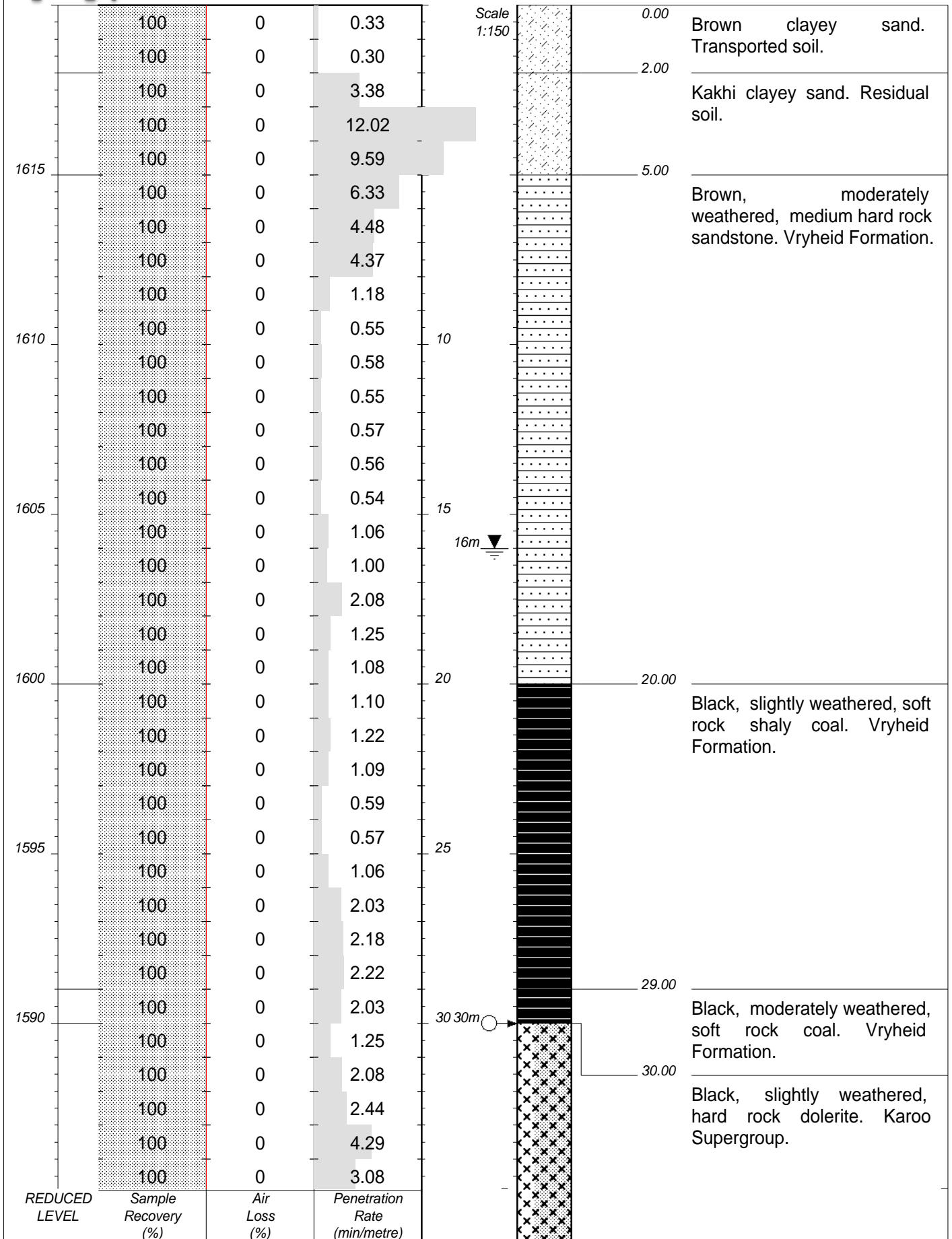
TYPE SET BY : Andries Vukeya
SETUP FILE : STANDARD.SET

INCLINATION : Vertical
DIAM : 165 mm
DATE : 11/08/2023
DATE : 11/08/2023

DATE : 06/12/2023 13:54
TEXT : ..ussionBoreholesKBH3.TXT

ELEVATION : 1612 m
X-COORD : 29.45448°E
Y-COORD : 26.09316°S

HOLE No: K-BH3

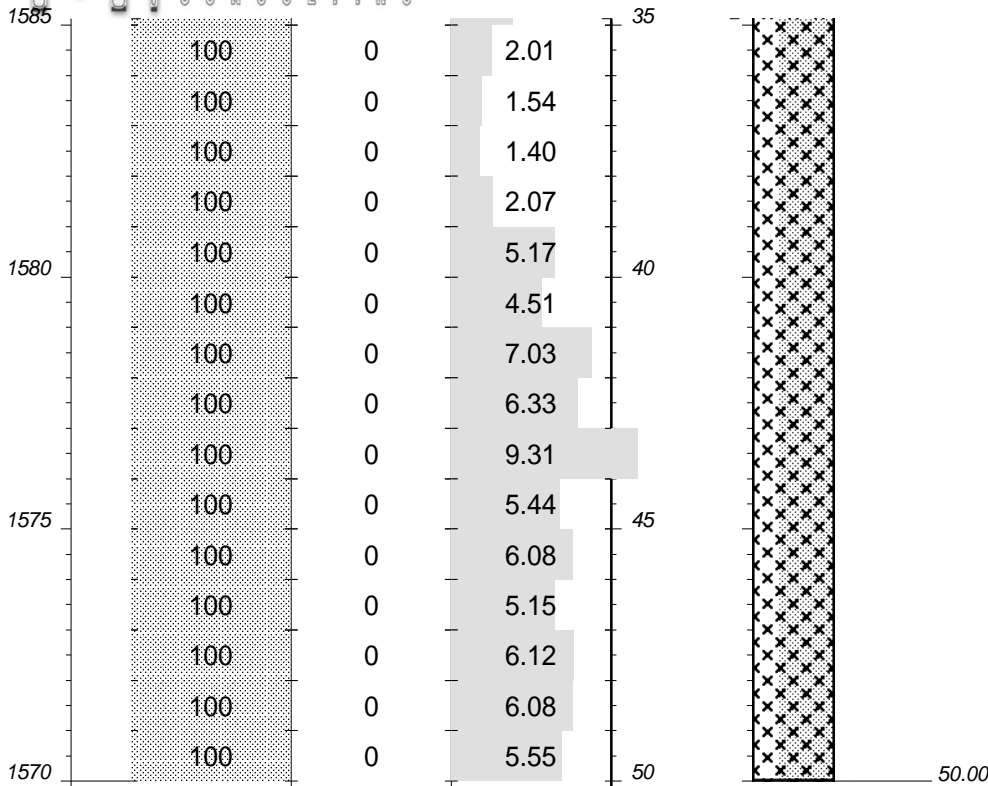




**ESKOM
PRELIMINARY GEOTECHNICAL INVESTIGATION:
KOMATI POWER STATION**

HOLE No: K-BH4
Sheet 2 of 2

JOB NUMBER: MAK152.08.23



NOTES

- 1) Water strike at 30m.
- 2) Groundwater table after twenty-four hours: 16m.
- 3) Borehole terminated at 50m on hard rock dolerite.

REDUCED LEVEL	Sample Recovery (%)	Air Loss (%)	Penetration Rate (min/metre)
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CONTRACTOR : BAV Consulting
MACHINE : Thor 5000 Drill Rig
DRILLED BY : Zakhele Zaca
PROFILED BY : Andries Vukeya

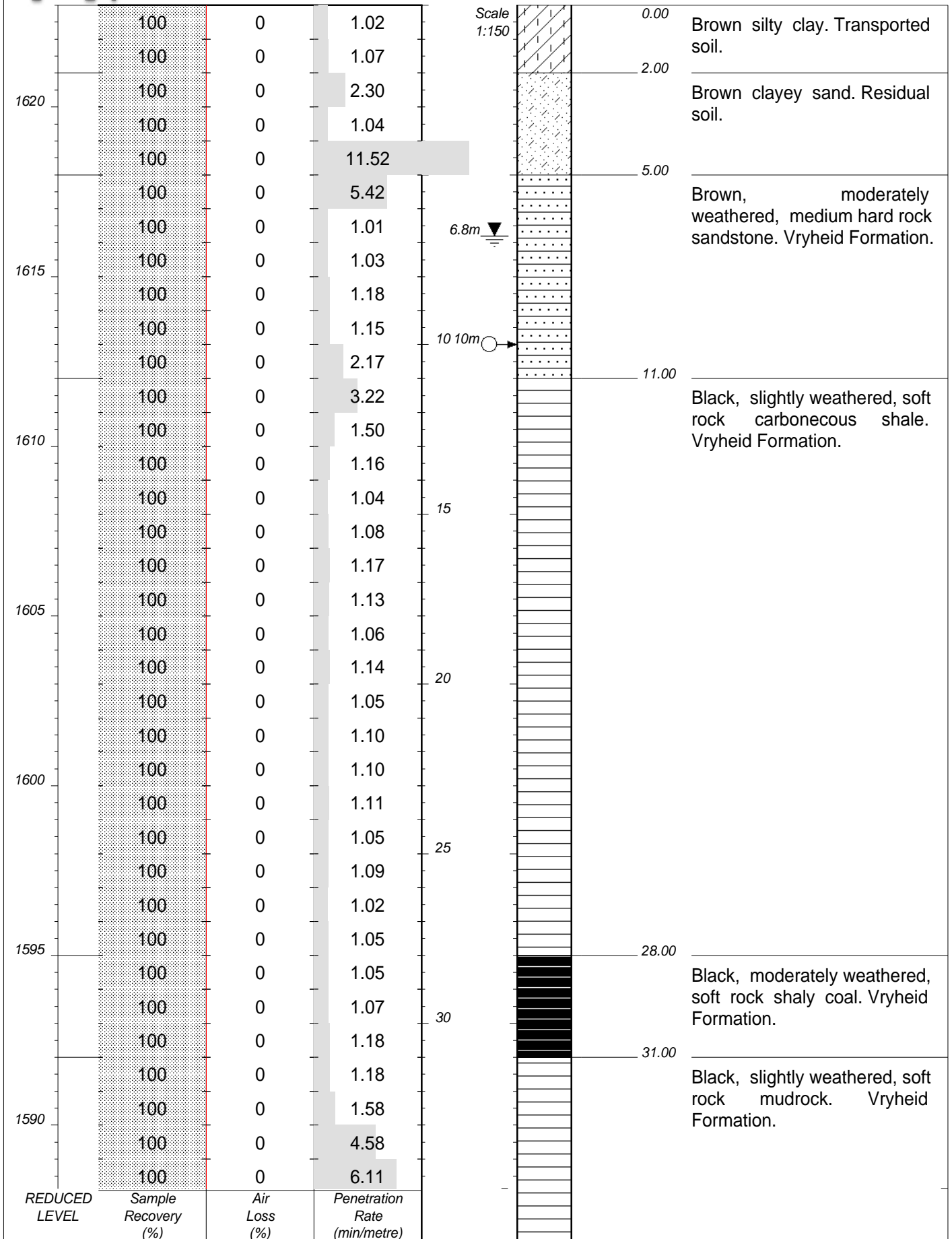
TYPE SET BY : Andries Vukeya
SETUP FILE : STANDARD.SET

INCLINATION : Vertical
DIAM : 165 mm
DATE : 10/08/2023
DATE : 10/08/2023

DATE : 06/12/2023 13:55
TEXT : ..ussionBoreholesKBH4.TXT

ELEVATION : 1620 m
X-COORD : 29.45725°E
Y-COORD : 26.09212°S

HOLE No: K-BH4

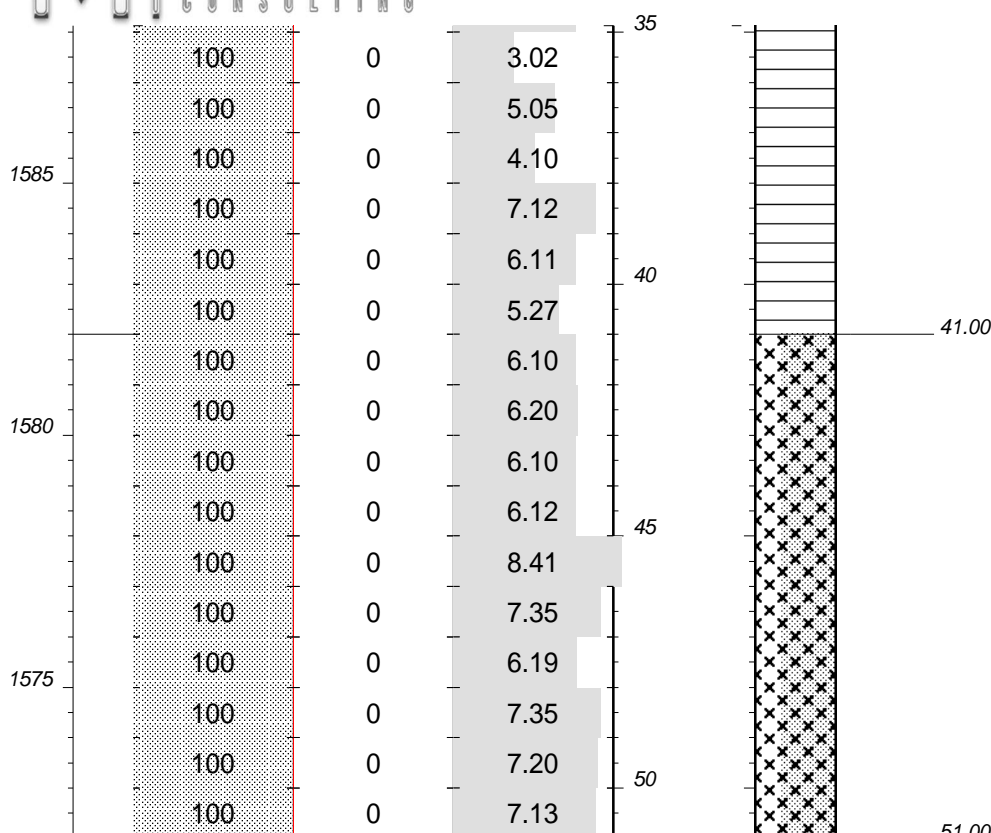




**ESKOM
PRELIMINARY GEOTECHNICAL INVESTIGATION:
KOMATI POWER STATION**

HOLE No: K-BH5
Sheet 2 of 2

JOB NUMBER: MAK152.08.23



Black, slightly weathered, hard rock dolerite.

NOTES

- 1) Water strike at 10m.
- 2) Groundwater table after twenty-four hours: 6.8m.
- 3) Borehole terminated at 51m on hard rock dolerite.

REDUCED LEVEL	Sample Recovery (%)	Air Loss (%)	Penetration Rate (min/metre)
---------------	---------------------	--------------	------------------------------

CONTRACTOR : BAV Consulting
MACHINE : Thor 5000 Drill Rig
DRILLED BY : Zakhele Zaca
PROFIED BY : Andries Vukeya

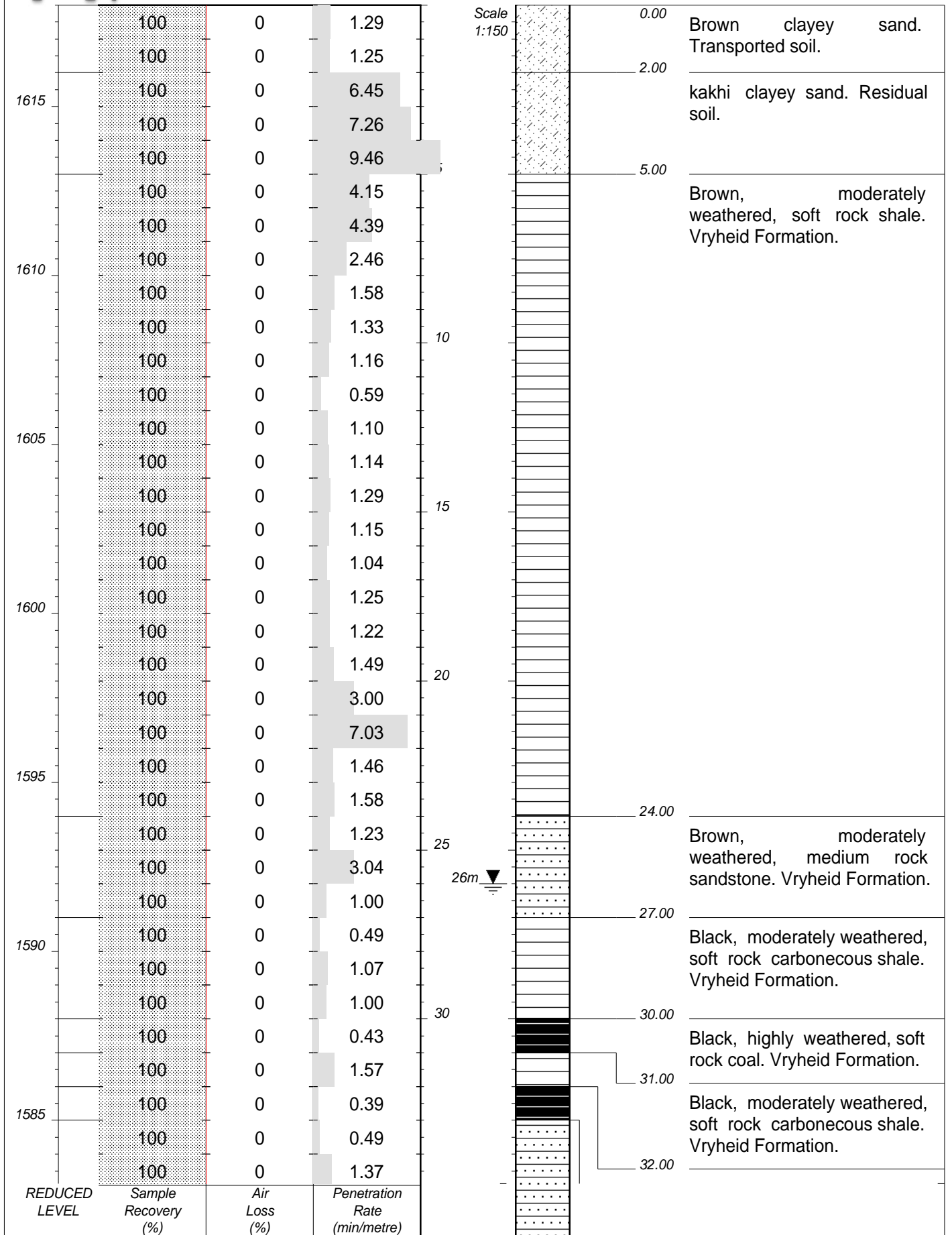
TYPE SET BY : Andries Vukeya
SETUP FILE : STANDARD.SET

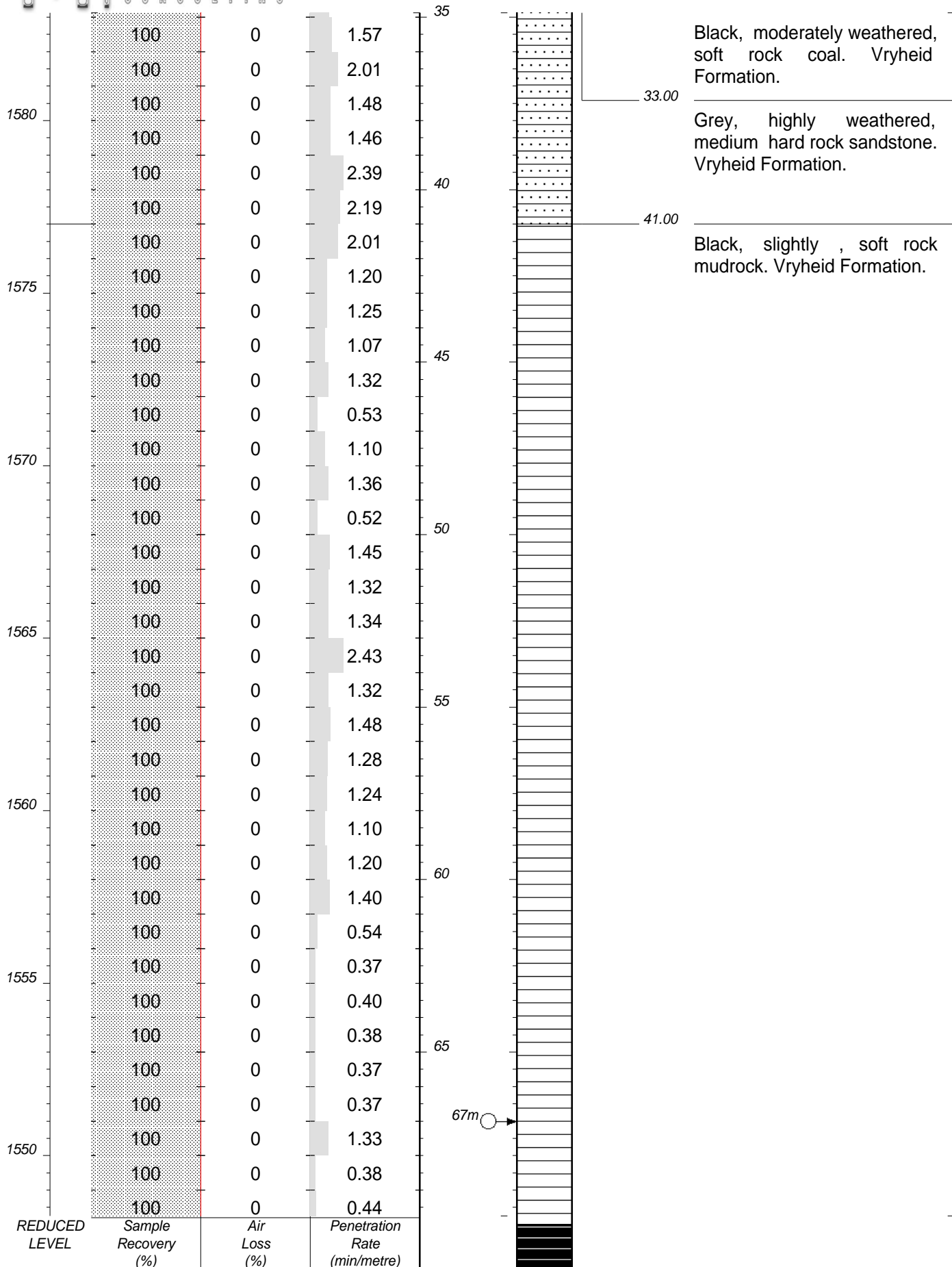
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DIAM : 165 mm
DATE : 09/08/2023
DATE : 09/08/2023

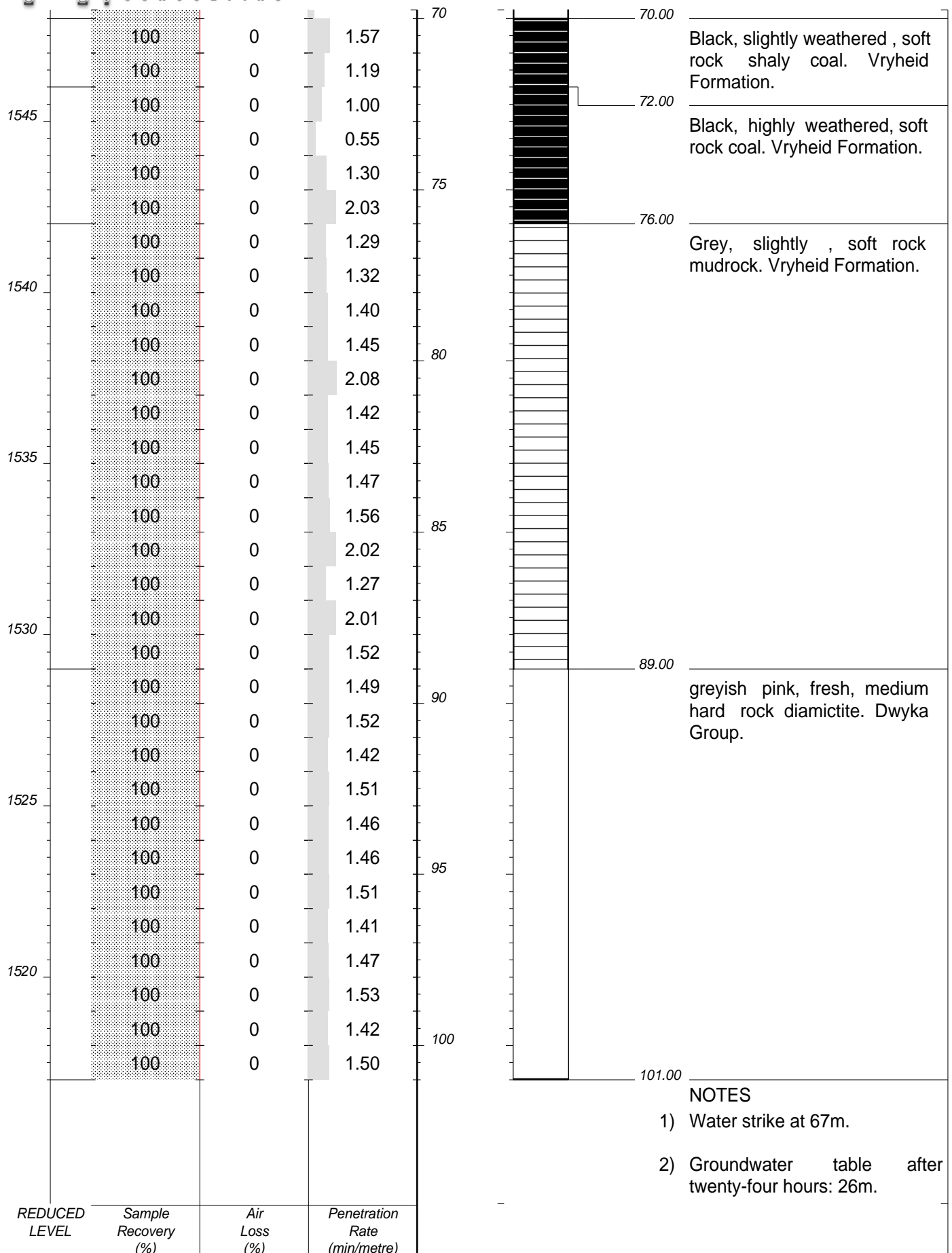
DATE : 06/12/2023 13:56
TEXT : ..ussionBoreholesKBH5.TXT

ELEVATION : 1623 m
X-COORD : 29.46039°E
Y-COORD : 26.09102°S

HOLE No: K-BH5









ESKOM
PRELIMINARY GEOTECHNICAL INVESTIGATION:
KOMATI POWER STATION

HOLE No: K-BH6
Sheet 4 of 4

JOB NUMBER: MAK152.08.23

- 3) Borehole terminated at 101m
due to required depth.

REDUCED LEVEL	Sample Recovery (%)	Air Loss (%)	Penetration Rate (min/metre)
------------------	---------------------------	--------------------	------------------------------------

CONTRACTOR : BAV Consulting
MACHINE : Thor 5000 Drill Rig
DRILLED BY : Zakhele Zaca
PROFIED BY : Andries Vukeya

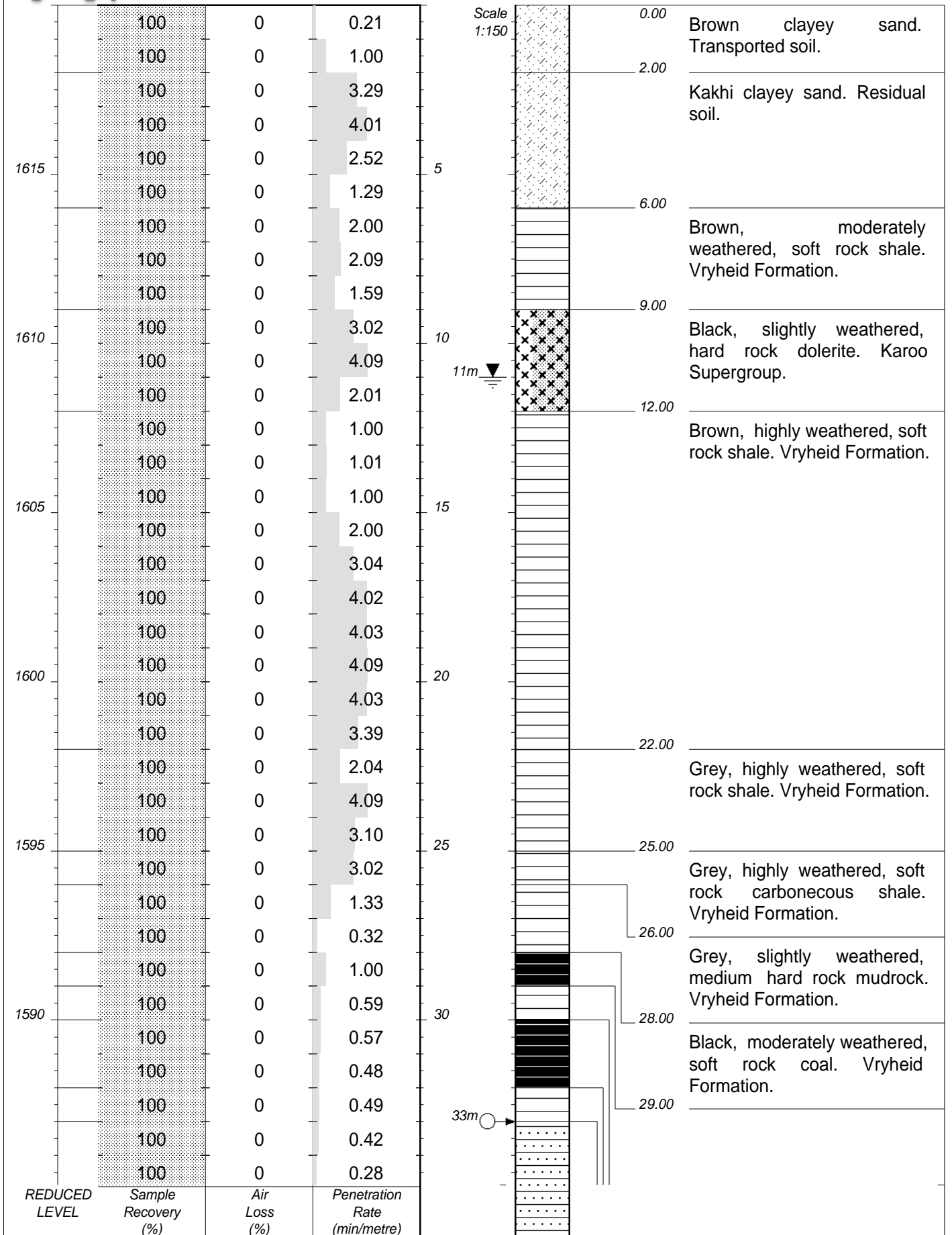
TYPE SET BY : Andries Vukeya
SETUP FILE : STANDARD.SET

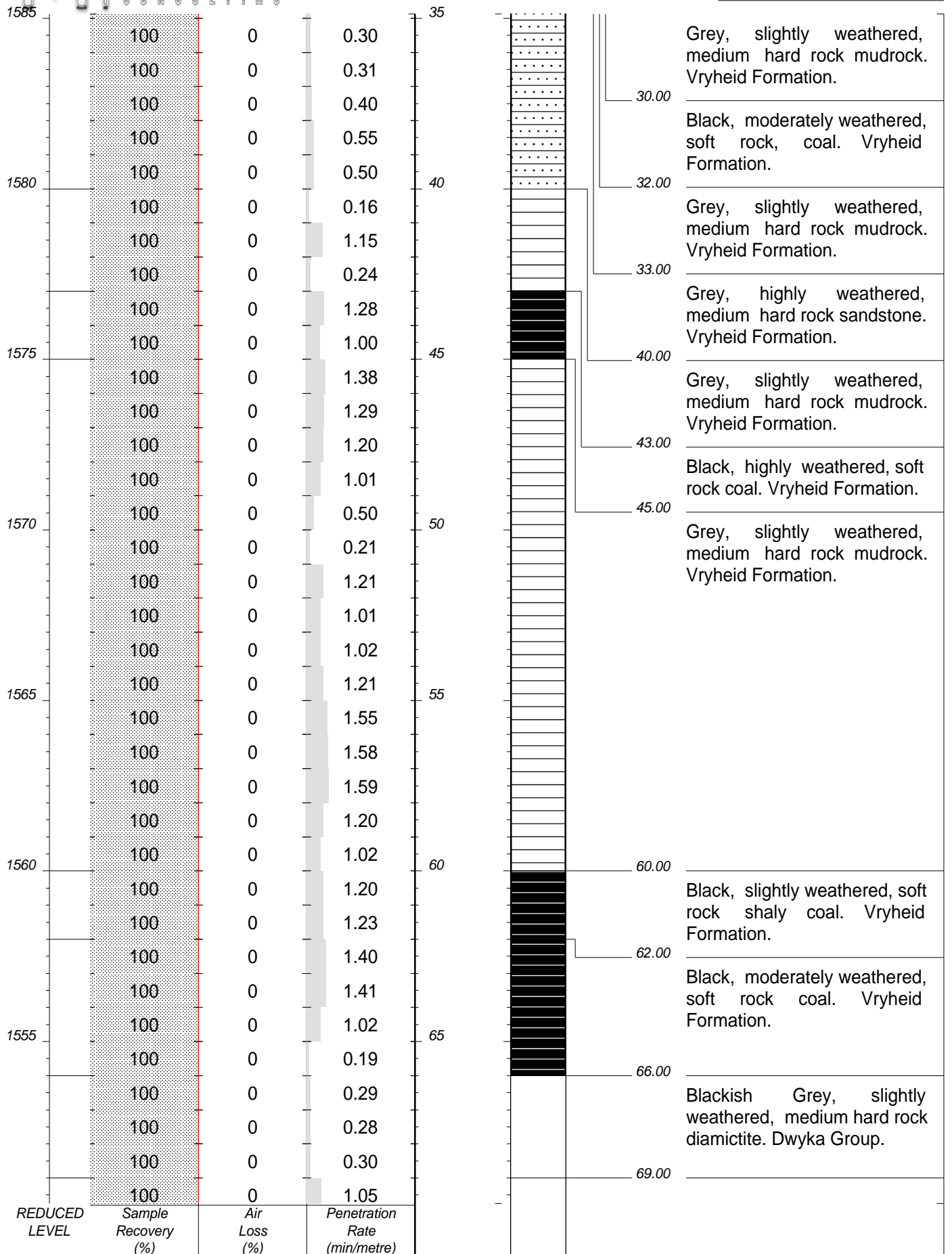
INCLINATION : Vertical
DIAM : 165 mm
DATE : 14/08/2023
DATE : 14/08/2023

DATE : 06/12/2023 13:58
TEXT : ..ussionBoreholes\KBH6.TXT

ELEVATION : 1618 m
X-COORD : 29.45138°E
Y-COORD : 26.09426°S

HOLE No: K-BH6







**ESKOM
PRELIMINARY GEOTECHNICAL INVESTIGATION:
KOMATI POWER STATION**

HOLE No: K-BH7
Sheet 3 of 3

JOB NUMBER: MAK152.08.23

1550	100	0	1.09	70
	100	0	1.00	
	100	0	1.30	
	100	0	1.40	
1545	100	0	1.50	75
	100	0	1.43	
	100	0	1.44	
	100	0	1.09	
	100	0	1.08	
1540	100	0	1.09	80
	100	0	1.20	

Greyish pink, slightly weathered, medium hard rock diamictite. Dwyka Group.

81.00

NOTES

- 1) Water strike at 33m.
- 2) Groundwater table after twenty-four hours: 11m.
- 3) Borehole terminated at 81m due to required depth.

REDUCED LEVEL	Sample Recovery (%)	Air Loss (%)	Penetration Rate (min/metre)
---------------	---------------------	--------------	------------------------------

CONTRACTOR : BAV Consulting
MACHINE : Thor 5000 Drill Rig
DRILLED BY : Zakhele Zaca
PROFIED BY : Andries Vukeya

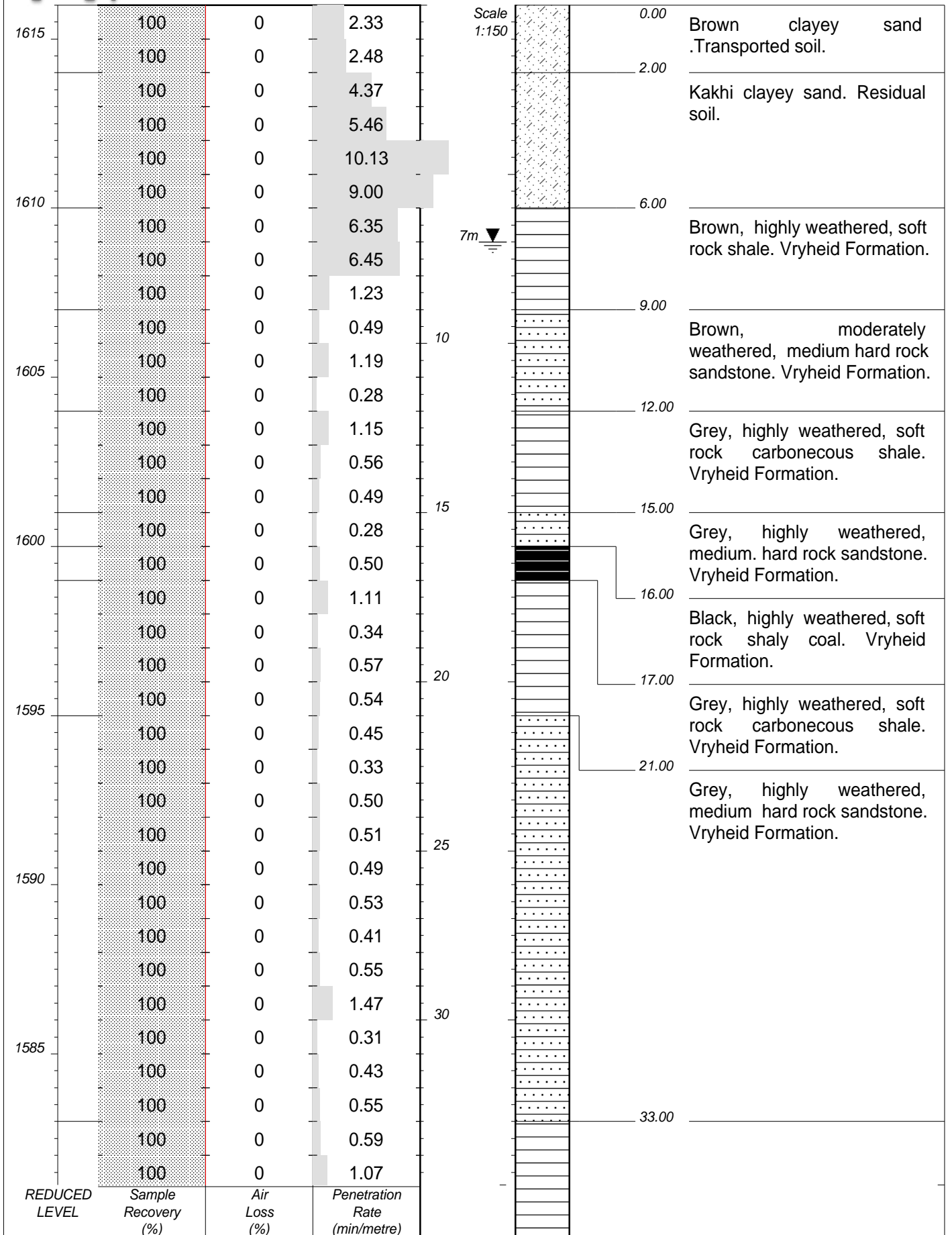
TYPE SET BY : Andries Vukeya
SETUP FILE : STANDARD.SET

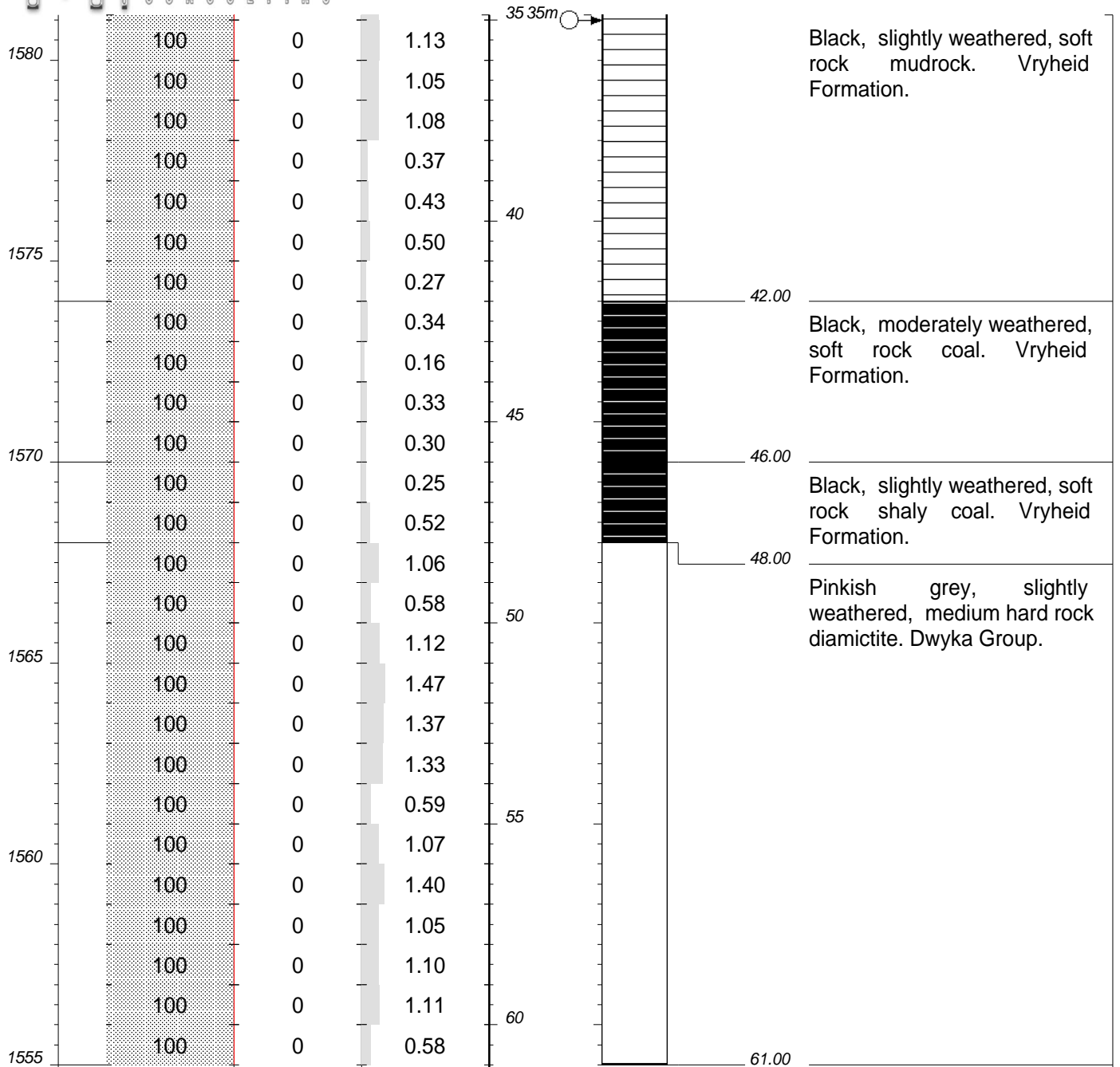
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DIAM : 165 mm
DATE : 02/09/2023
DATE : 02/09/2023

DATE : 06/12/2023 13:58
TEXT : ..ussionBoreholesKBH7.TXT

ELEVATION : 1620 m
X-COORD : 29.45370°E
Y-COORD : 26.09443°S

HOLE No: K-BH7




NOTES

- 1) Water strike at 35m.
- 2) Groundwater table after twenty-four hours: 7m.
- 3) Borehole terminated at 61m due to required depth.

CONTRACTOR : BAV Consulting
MACHINE : Thor 5000 Drill Rig
DRILLED BY : Zakhele Zaca
PROFIED BY : Andries Vukeya

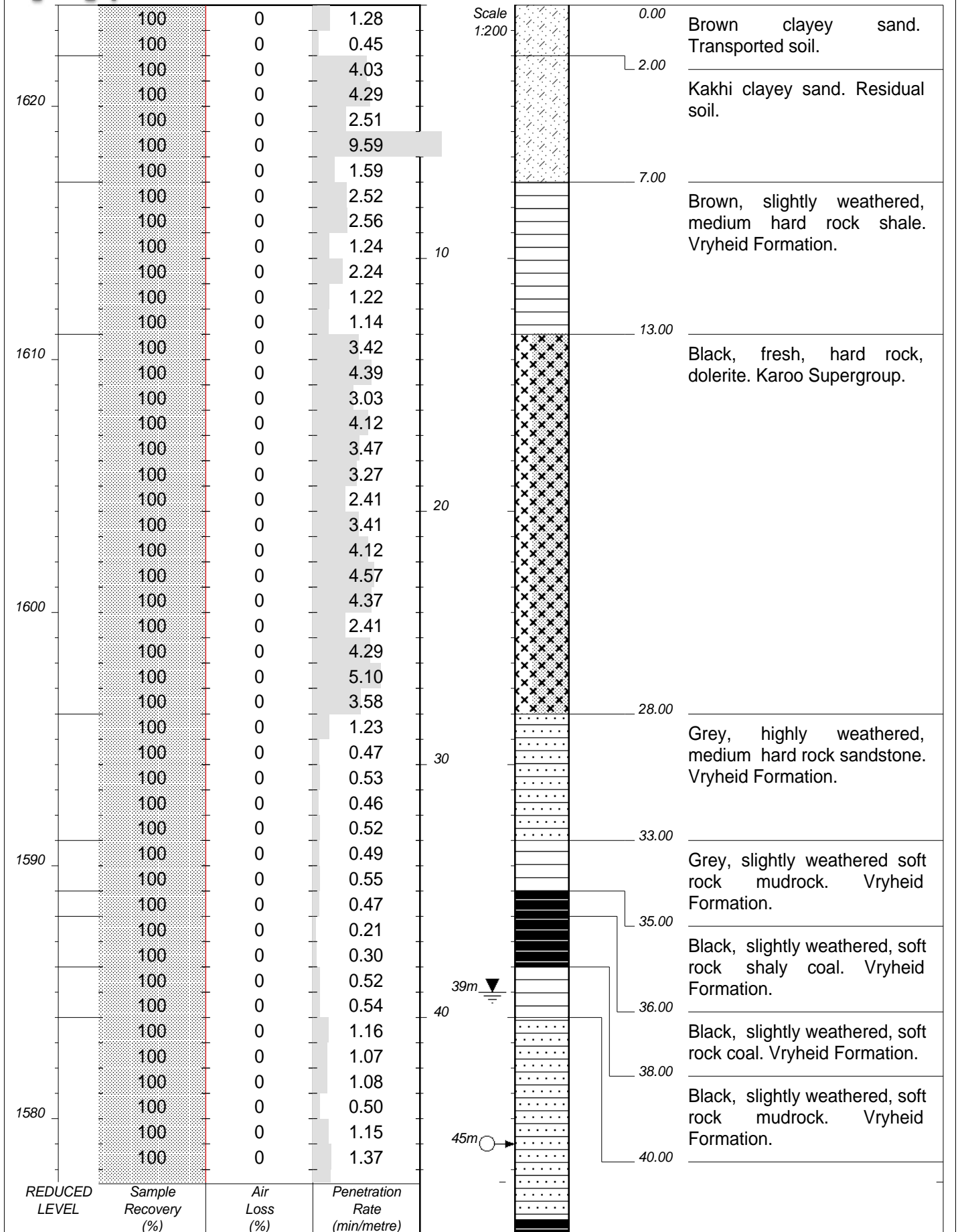
TYPE SET BY : Andries Vukeya
SETUP FILE : STANDARD.SET

INCLINATION : Vertical
DIAM : 165 mm
DATE : 05/09/2023
DATE : 05/09/2023

DATE : 06/12/2023 13:59
TEXT : ..ussionBoreholesKBH8.TXT

ELEVATION : 1616 m
X-COORD : 29.45657°E
Y-COORD : 26.09339°S

HOLE No: K-BH8

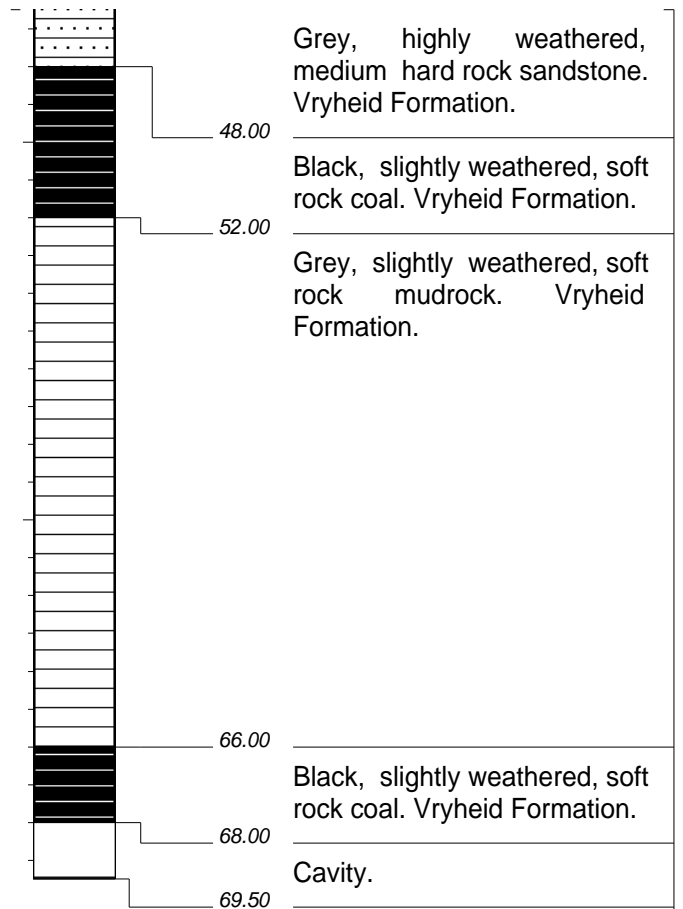
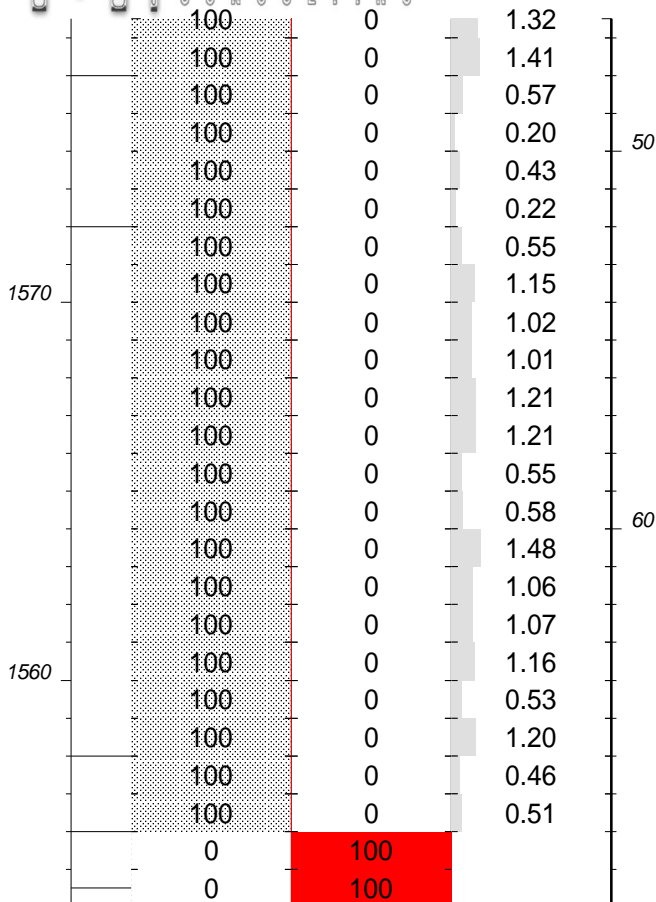




**ESKOM
PRELIMINARY GEOTECHNICAL INVESTIGATION:
KOMATI POWER STATION**

HOLE No: K-BH9
Sheet 2 of 2

JOB NUMBER: MAK152.08.23



NOTES

- 1) Water strike at 45m.
- 2) Groundwater table after twenty-four hours: 39m.
- 3) Borehole terminated at 69.5m due to drilling difficulties.

CONTRACTOR : BAV Consulting
MACHINE : Thor 5000 Drill Rig
DRILLED BY : Zakhele Zaca
PROFIED BY : Andries Vukeya

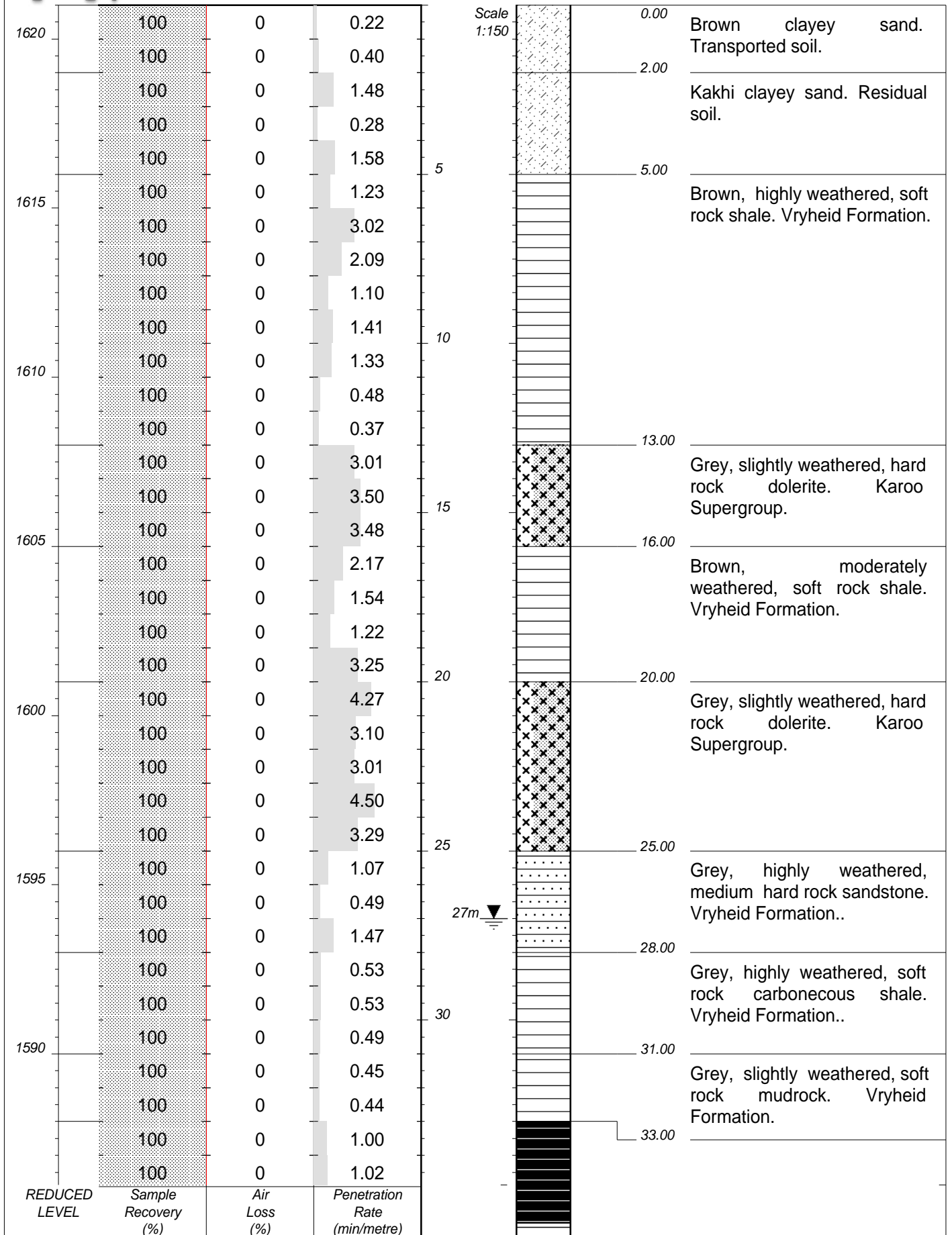
TYPE SET BY : Andries Vukeya
SETUP FILE : STANDARD.SET

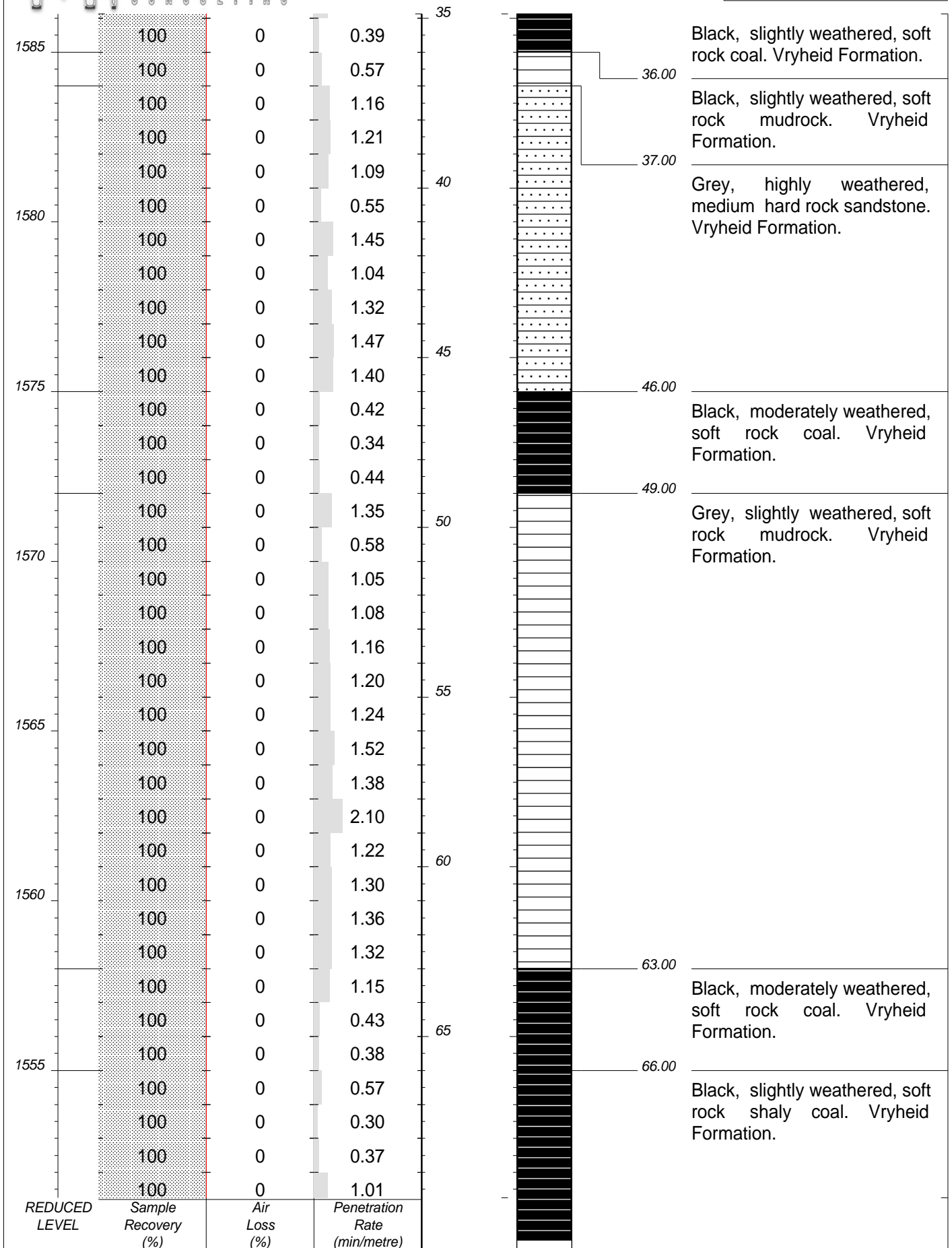
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DIAM : 165 mm
DATE : 02/09/2023
DATE : 02/09/2023

DATE : 06/12/2023 13:59
TEXT : ..ussionBoreholesKBH9.TXT

ELEVATION : 1624 m
X-COORD : 29.45039°E
Y-COORD : 26.09622°S

HOLE No: K-BH9





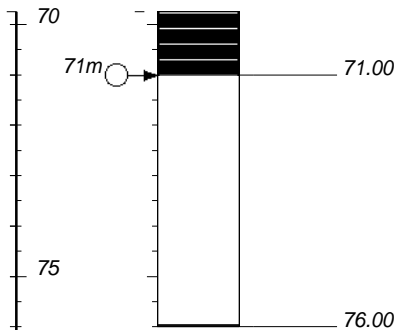


**ESKOM
PRELIMINARY GEOTECHNICAL INVESTIGATION:
KOMATI POWER STATION**

HOLE No: K-BH10
Sheet 3 of 3

JOB NUMBER: MAK152.08.23

1550	100	0	1.35
	100	0	1.18
	100	0	1.19
	100	0	1.27
	100	0	1.08
1545	100	0	1.51



pinkish grey, slightly
weathered, medium hard rock
diamictite. Dwyka Group.

NOTES

- 1) Water strike at 71m.
- 2) Groundwater table after twenty-four hours: 27m.
- 3) Borehole terminated at 76m due to required depth.

REDUCED LEVEL	Sample Recovery (%)	Air Loss (%)	Penetration Rate (min/metre)
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CONTRACTOR : BAV Consulting
MACHINE : Thor 5000 Drill Rig
DRILLED BY : Zakhele Zaca
PROFIED BY : Andries Vukeya

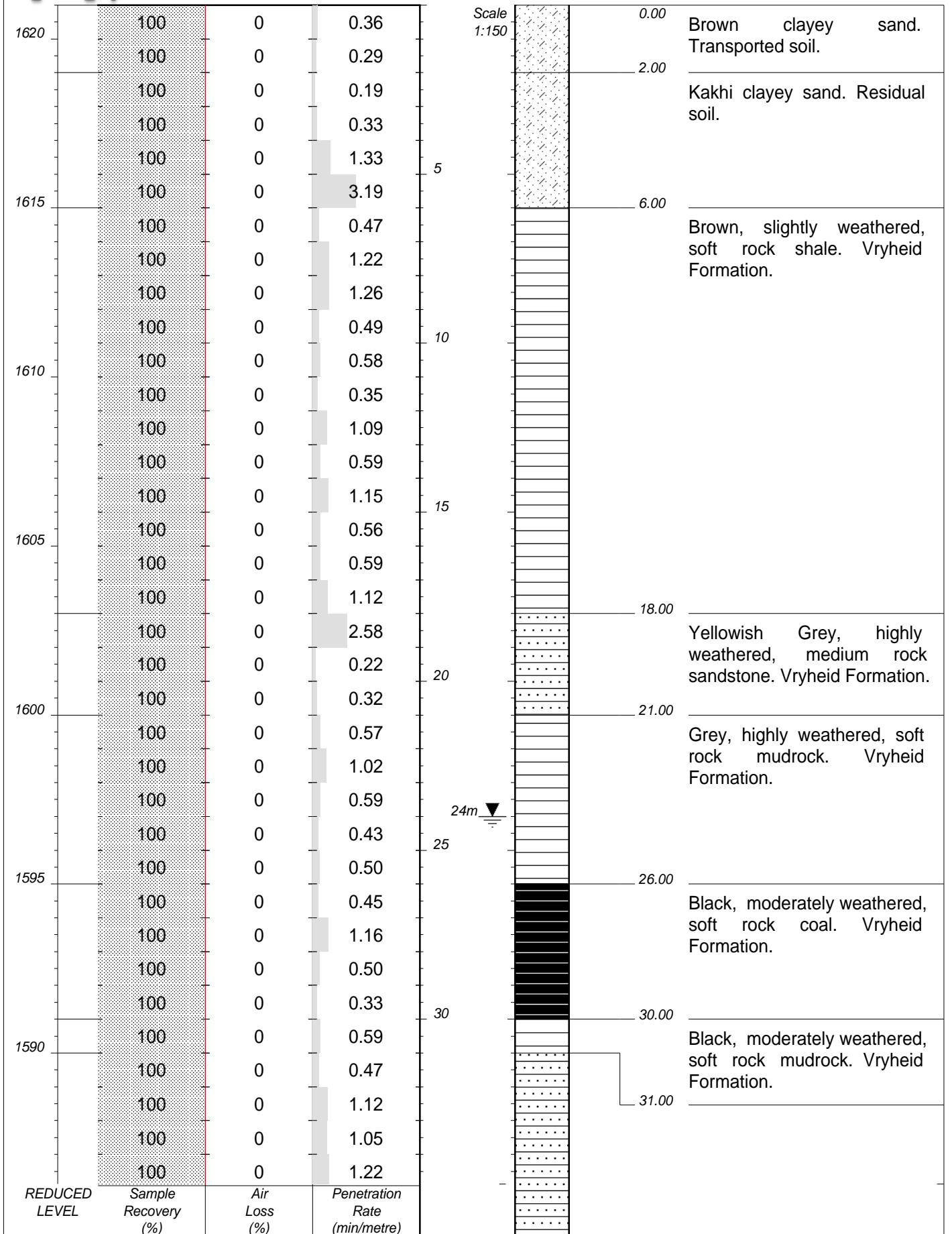
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SETUP FILE : STANDARD.SET

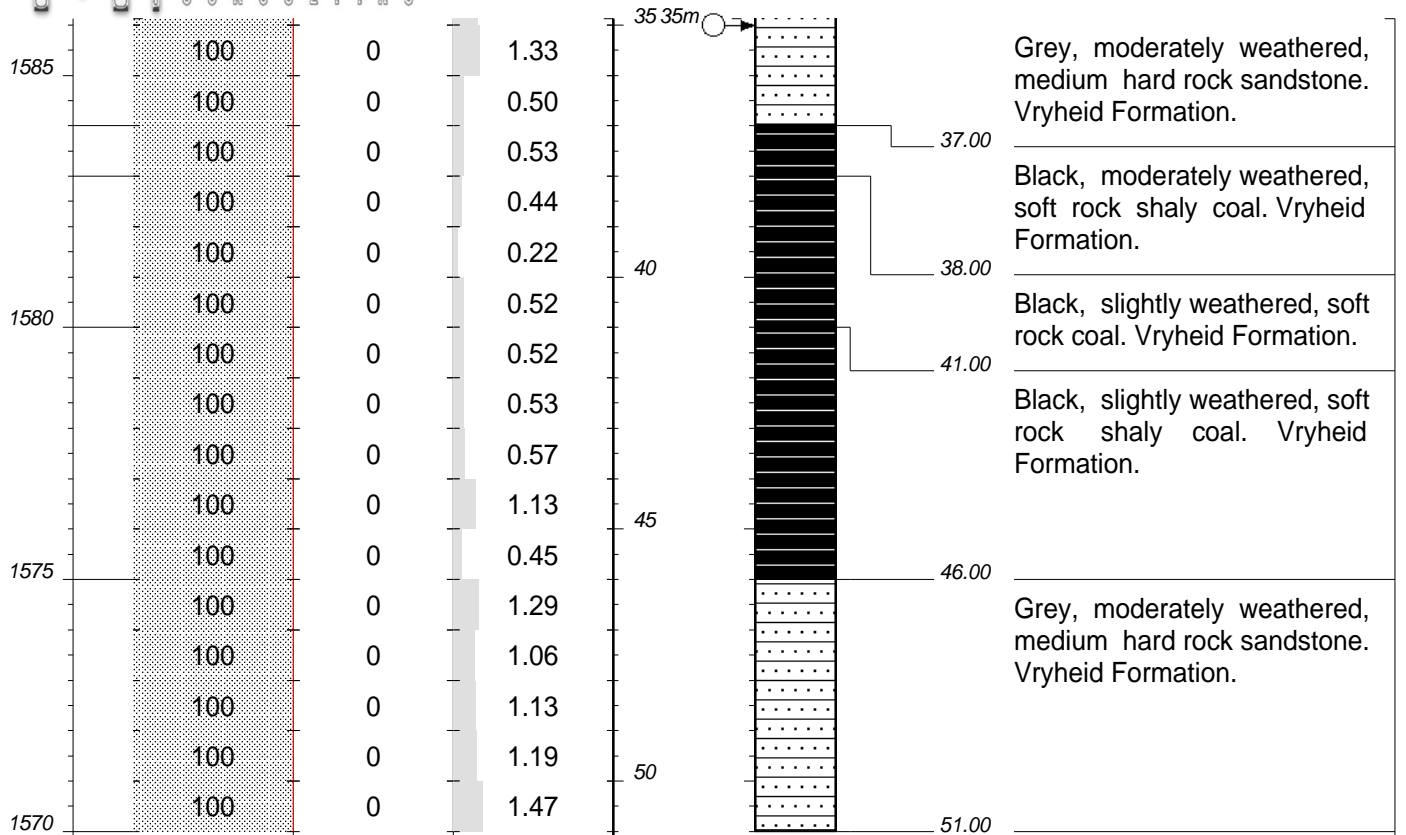
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DIAM : 165 mm
DATE : 07/09/2023
DATE : 07/09/2023

DATE : 06/12/2023 14:00
TEXT : ..ssionBoreholes\KBH10.TXT

ELEVATION : 1621 m
X-COORD : 29.45269°E
Y-COORD : 26.09587°S

HOLE No: K-BH10





NOTES

- 1) Water strike at 35m.
- 2) Groundwater table after twenty-four hours: 24m.
- 3) Borehole terminated at 51m due to drilling difficulties.

REDUCED LEVEL Sample Recovery (%) Air Loss (%) Penetration Rate (min/metre)

CONTRACTOR : BAV Consulting
MACHINE : Thor 5000 Drill Rig
DRILLED BY : Zakhele Zaca
PROFILED BY : Andries Vukeya

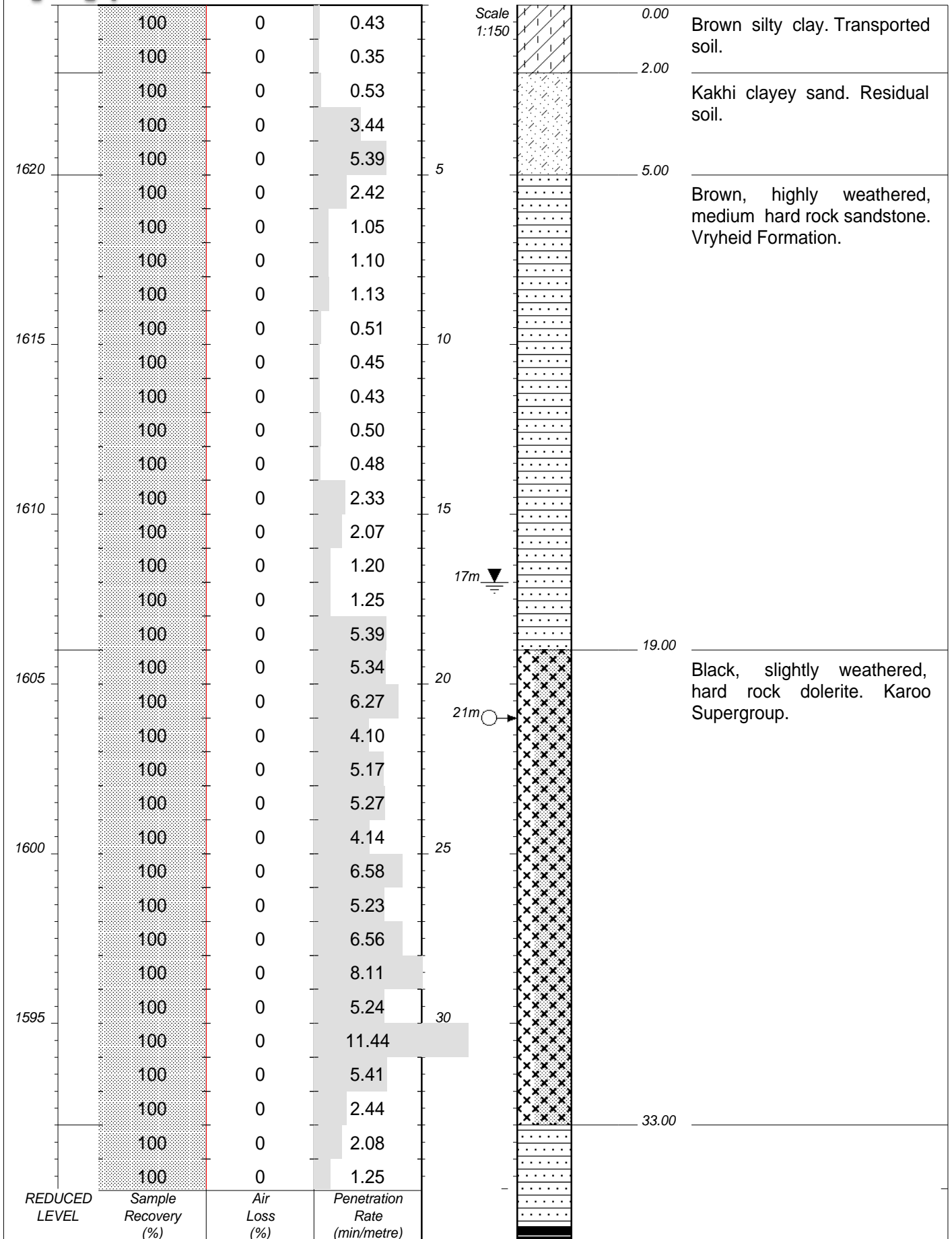
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SETUP FILE : STANDARD.SET

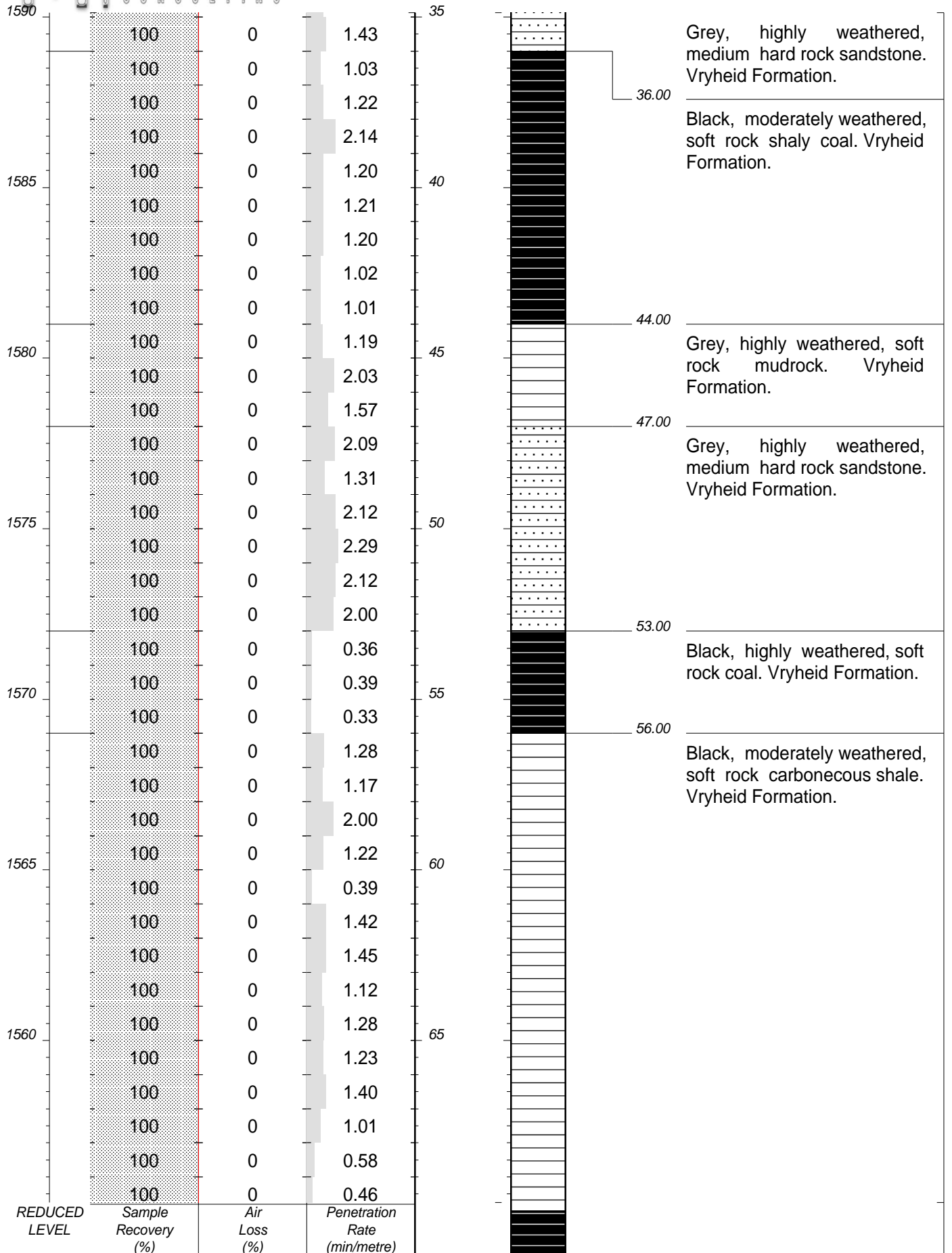
INCLINATION : Vertical
DIAM : 165 mm
DATE : 05/08/2023
DATE : 05/08/2023

DATE : 06/12/2023 14:01
TEXT : ..ssionBoreholes\KBH11.TXT

ELEVATION : 1621 m
X-COORD : 29.45511 °E
Y-COORD : 26.09521 °S

HOLE No: K-BH11





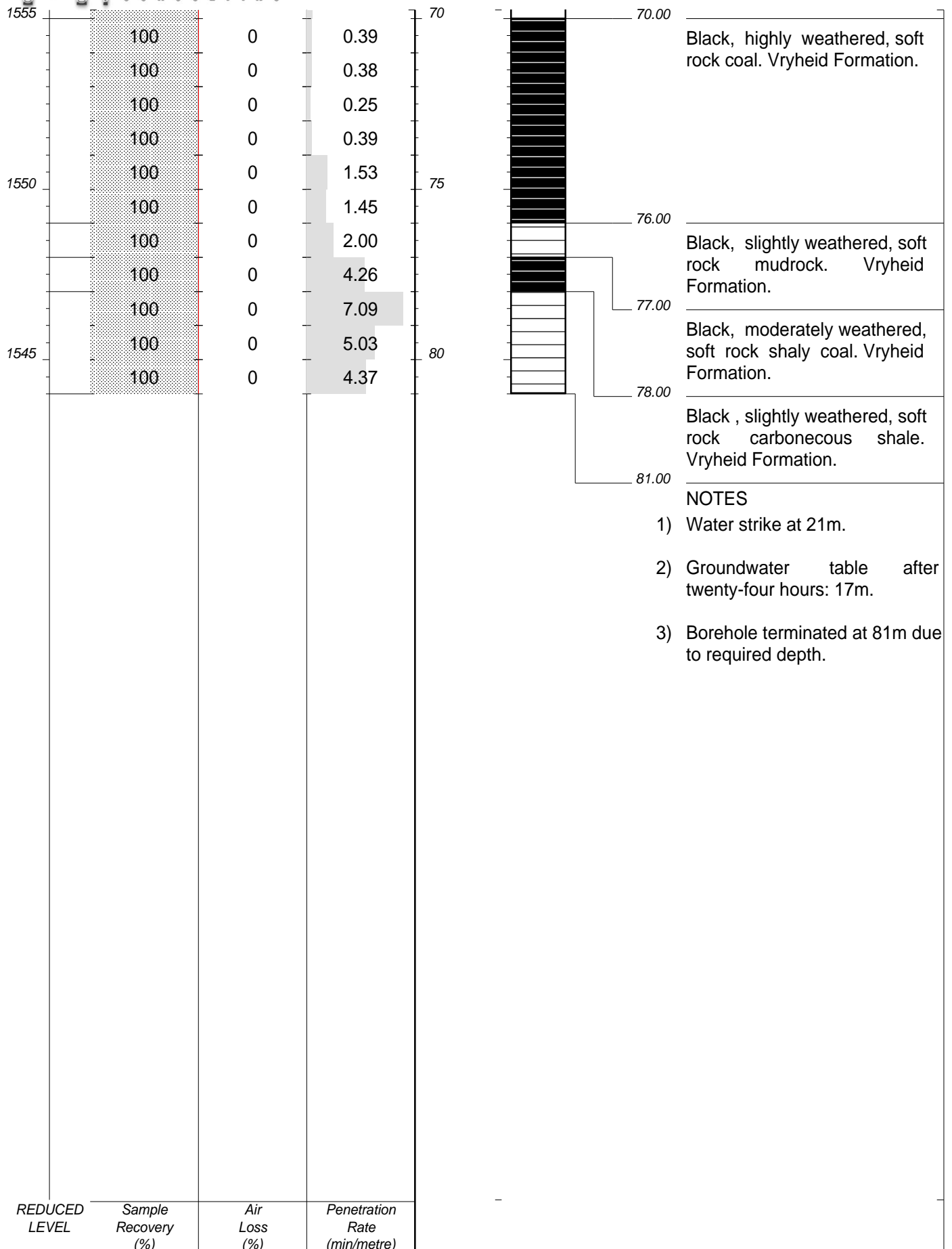


**ESKOM
PRELIMINARY GEOTECHNICAL INVESTIGATION:
KOMATI POWER STATION**

HOLE No: K-BH12

Sheet 3 of 4

JOB NUMBER: MAK152.08.23





ESKOM
PRELIMINARY GEOTECHNICAL INVESTIGATION:
KOMATI POWER STATION

HOLE No: K-BH12
Sheet 4 of 4

JOB NUMBER: MAK152.08.23

REDUCED LEVEL	Sample Recovery (%)	Air Loss (%)	Penetration Rate (min/metre)
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CONTRACTOR : BAV Consulting
MACHINE : Thor 5000 Drill Rig
DRILLED BY : Zakhele Zaca
PROFIED BY : Andries Vukeya

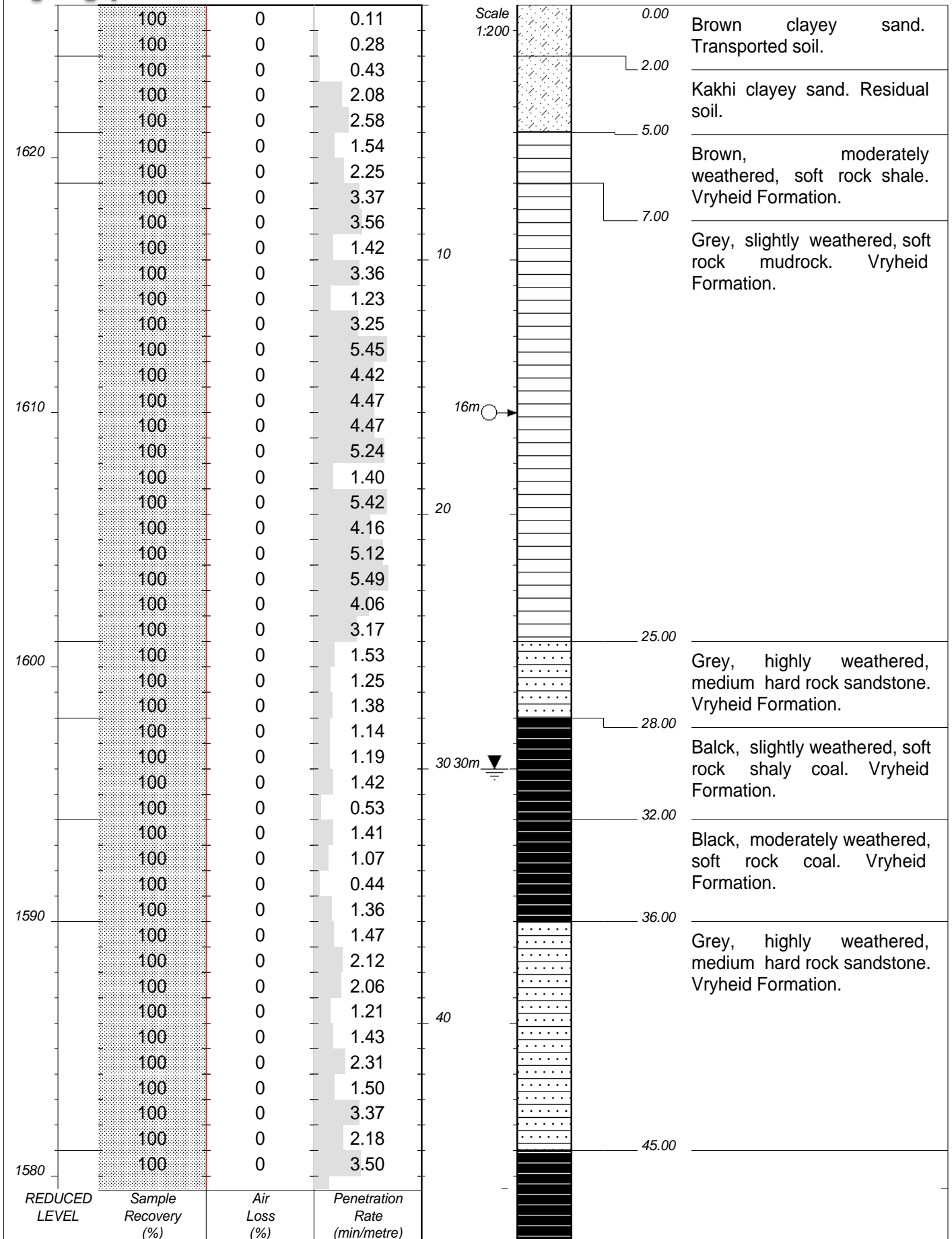
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SETUP FILE : STANDARD.SET

INCLINATION : Vertical
DIAM : 165 mm
DATE : 10/09/2023
DATE : 10/09/2023

DATE : 06/12/2023 14:01
TEXT : ..ssionBoreholes\KBH12.TXT

ELEVATION : 1625 m
X-COORD : 29.44029°E
Y-COORD : 26.09774°S

HOLE No: K-BH12

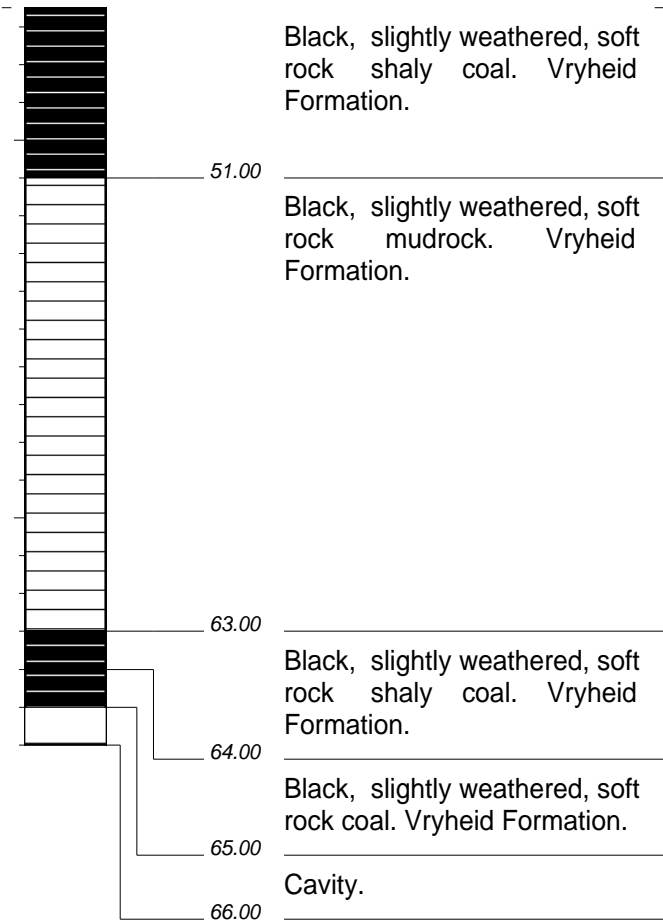
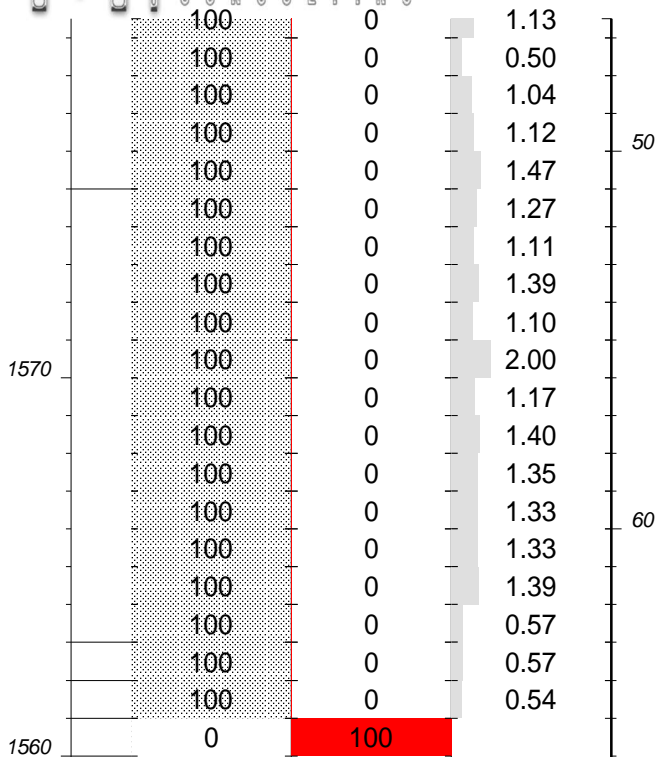




**ESKOM
PRELIMINARY GEOTECHNICAL INVESTIGATION:
KOMATI POWER STATION**

HOLE No: K-BH13
Sheet 2 of 2

JOB NUMBER: MAK152.08.23



NOTES

- 1) Water strike at 16m.
- 2) Groundwater table after twenty-four hours: 30m.
- 3) Borehole terminated at 66m due to drilling difficulties.

REDUCED LEVEL	Sample Recovery (%)	Air Loss (%)	Penetration Rate (min/metre)
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CONTRACTOR : BAV Consulting
MACHINE : Thor 5000 Drill Rig
DRILLED BY : Zakhele Zaca
PROFIED BY : Andries Vukeya

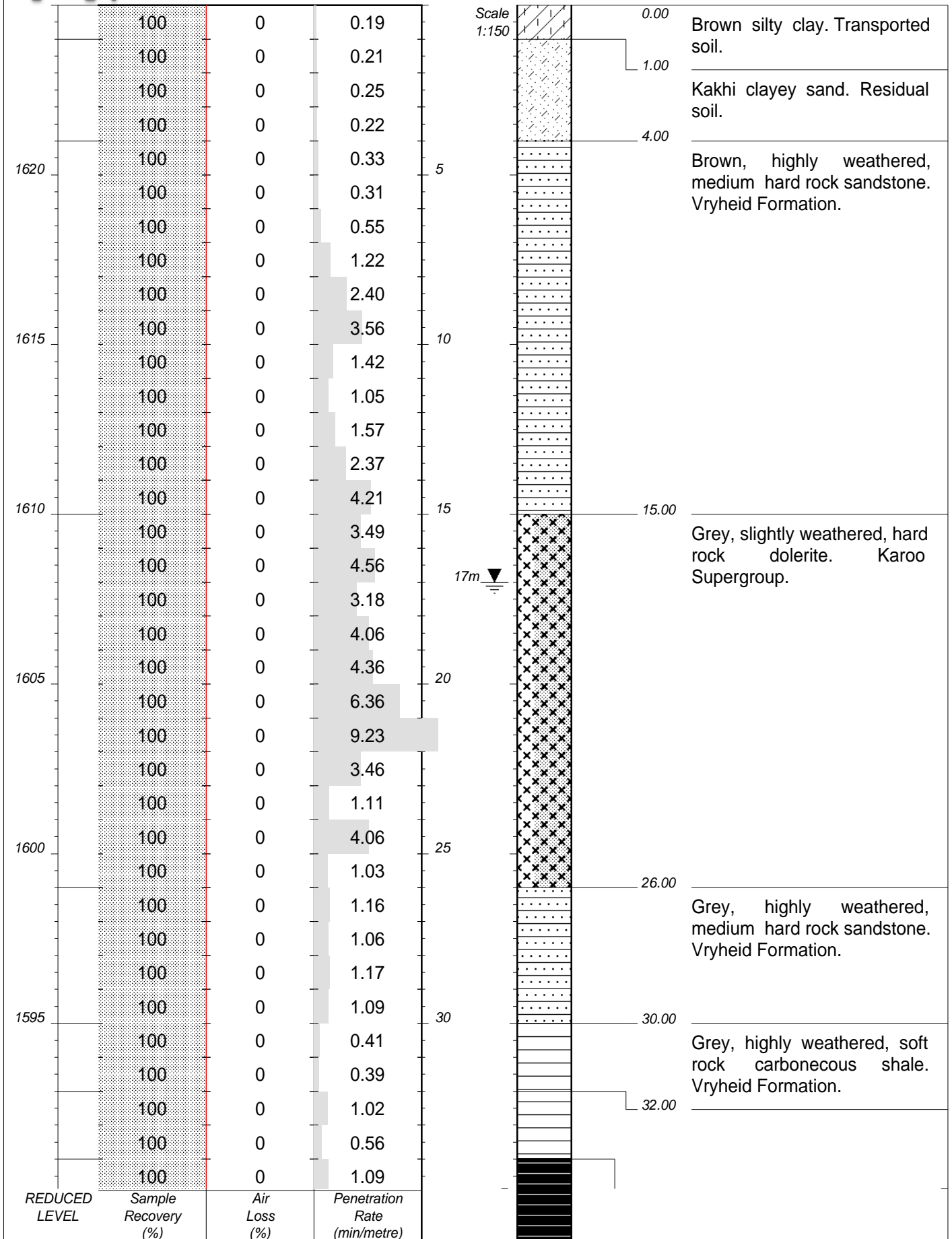
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SETUP FILE : STANDARD.SET

INCLINATION : Vertical
DIAM : 165 mm
DATE : 04/09/2023
DATE : 04/09/2023

DATE : 06/12/2023 14:02
TEXT : ..ssionBoreholes\KBH13.TXT

ELEVATION : 1626 m
X-COORD : 29.45139°E
Y-COORD : 26.09559°S

HOLE No: K-BH13



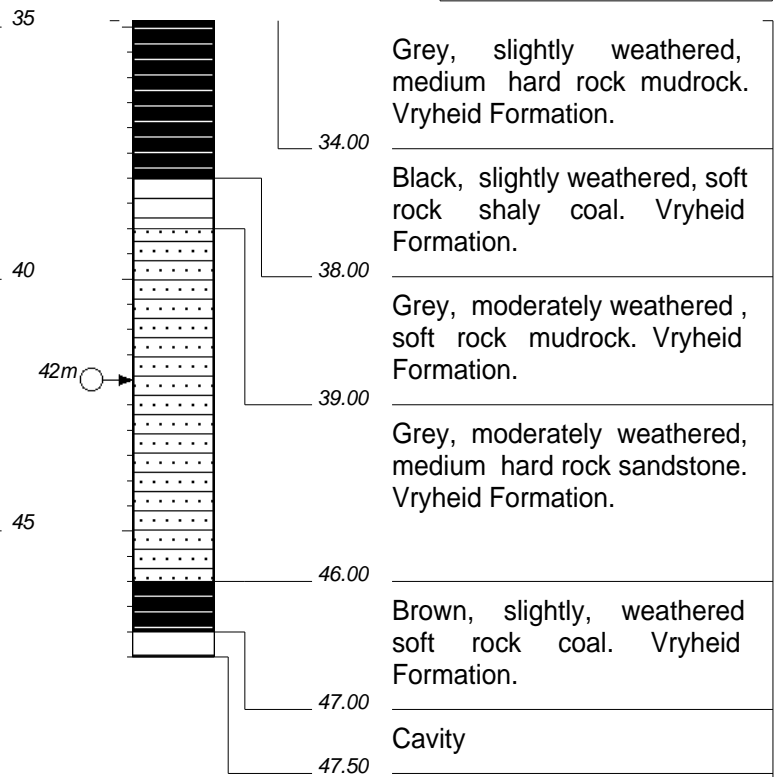


**ESKOM
PRELIMINARY GEOTECHNICAL INVESTIGATION:
KOMATI POWER STATION**

HOLE No: K-BH14
Sheet 2 of 2

JOB NUMBER: MAK152.08.23

1590	100	0	1.25
	100	0	0.48
	100	0	0.45
	100	0	1.04
1585	100	0	1.17
	100	0	1.42
	100	0	2.19
	100	0	4.39
	100	0	2.59
1580	100	0	4.14
	100	0	4.14
	100	0	1.05
	0	100	0.12



NOTES

- 1) Water strike at 42m.
- 2) Groundwater table after twenty-four hours: 17m.
- 3) Borehole terminated at 47.5m due to drilling difficulties.

REDUCED LEVEL	Sample Recovery (%)	Air Loss (%)	Penetration Rate (min/metre)
---------------	---------------------	--------------	------------------------------

CONTRACTOR : BAV Consulting
MACHINE : Thor 5000 Drill Rig
DRILLED BY : Zakhele Zaca
PROFILED BY : Andries Vukeya

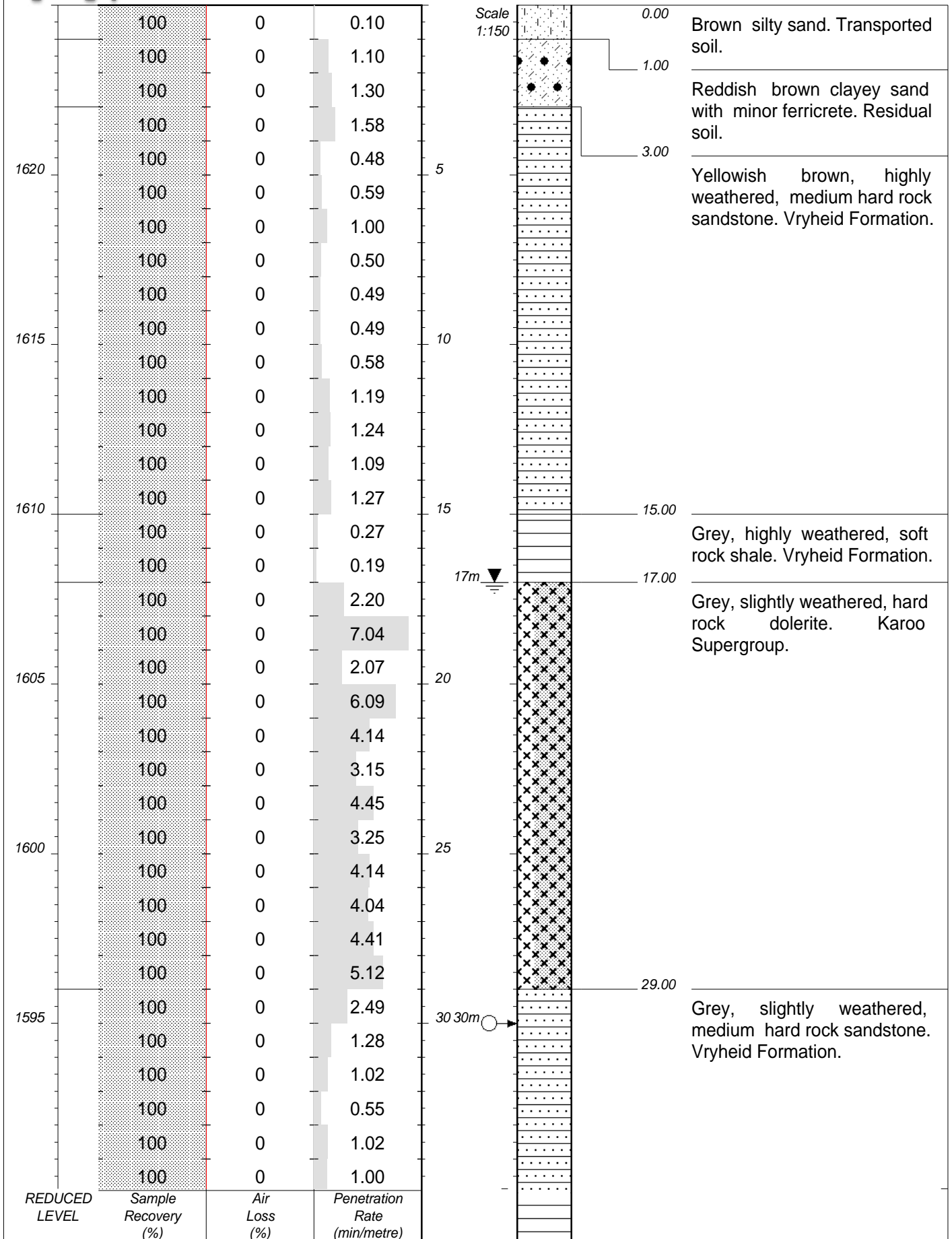
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SETUP FILE : STANDARD.SET

INCLINATION : Vertical
DIAM : 165 mm
DATE : 08/09/2023
DATE : 08/09/2023

DATE : 06/12/2023 14:03
TEXT : ..ssionBoreholes\KBH14.TXT

ELEVATION : 1625 m
X-COORD : 29.45394°E
Y-COORD : 26.09684°S

HOLE No: K-BH14



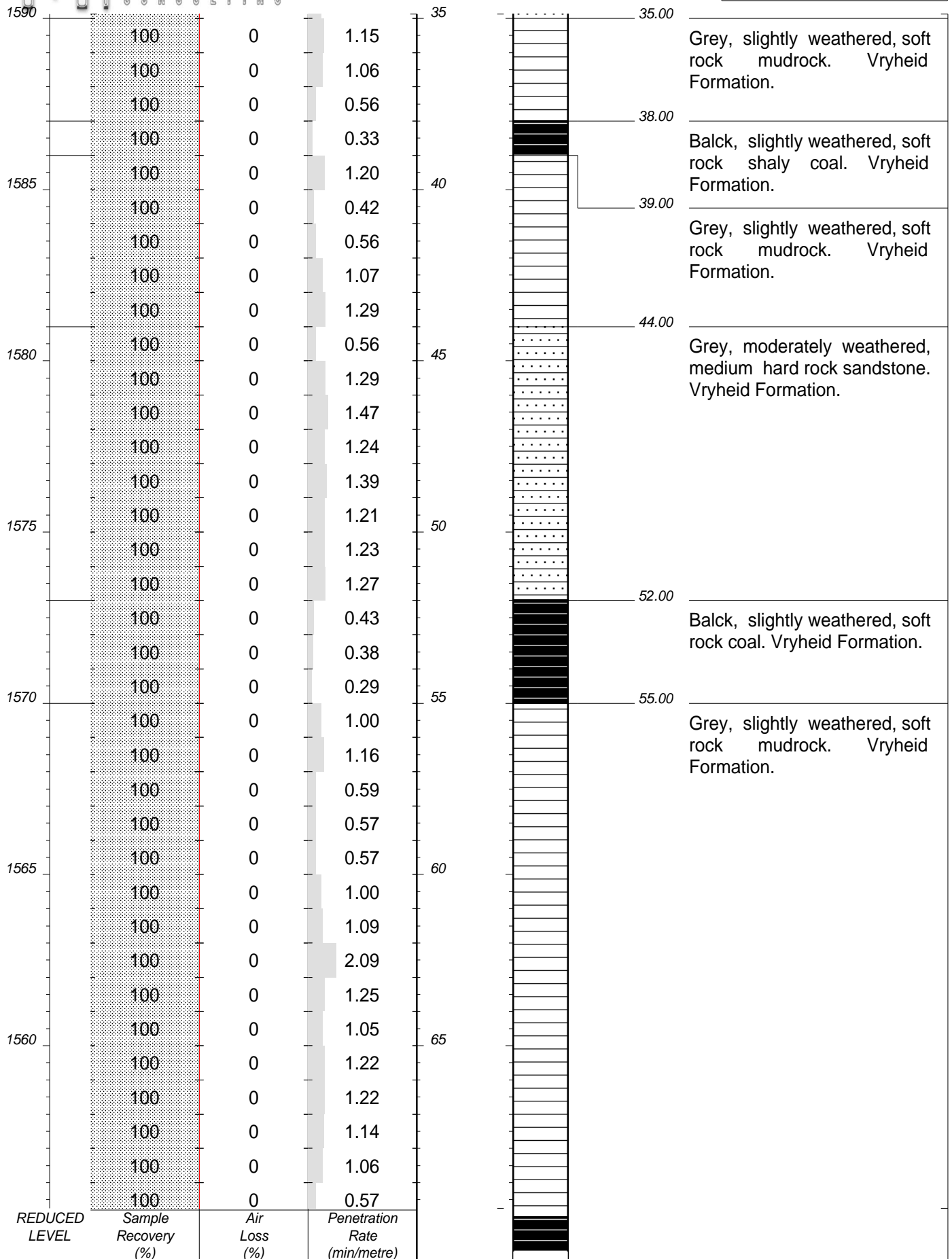


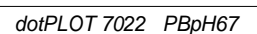
**ESKOM
PRELIMINARY GEOTECHNICAL INVESTIGATION:
KOMATI POWER STATION**

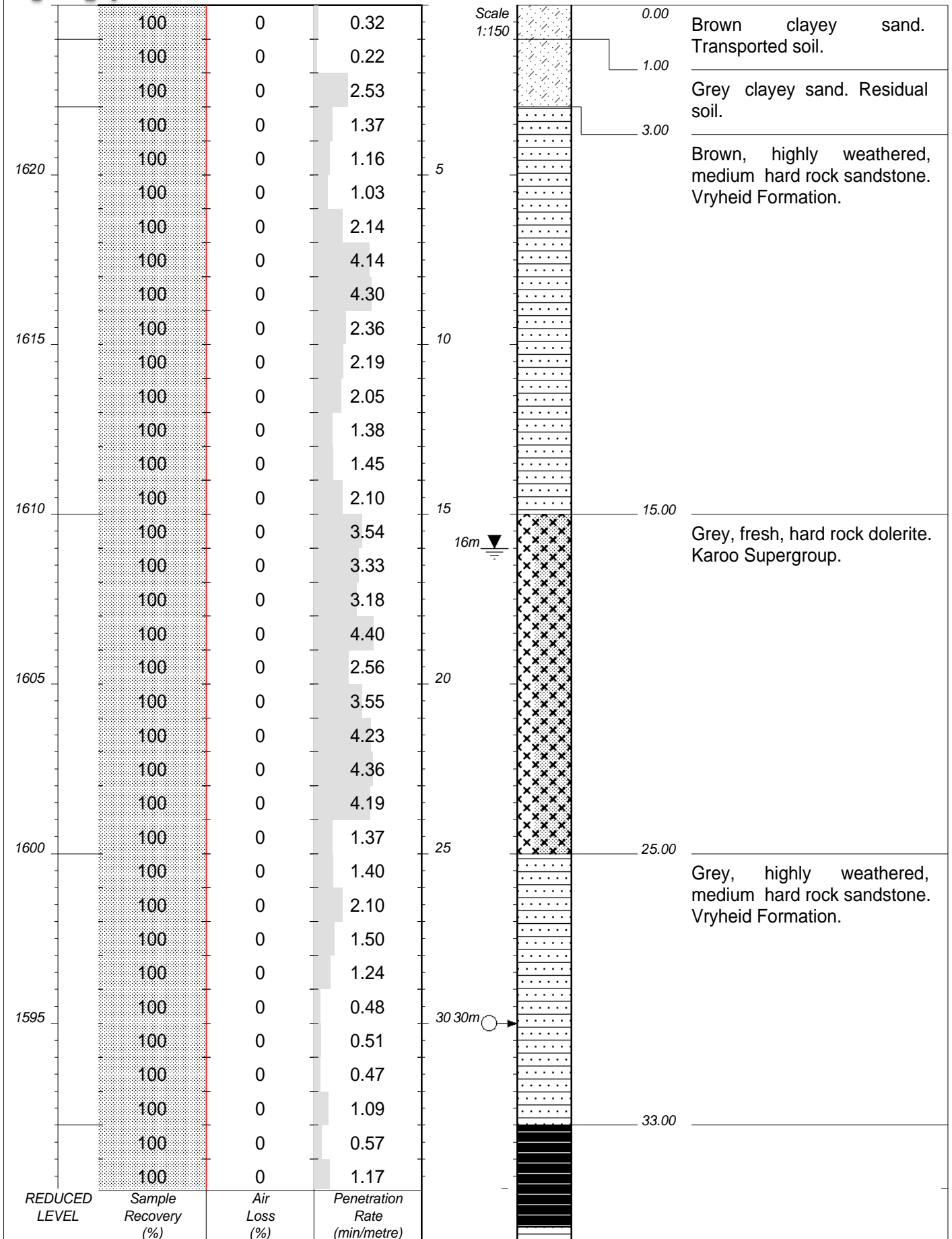
HOLE No: K-BH15

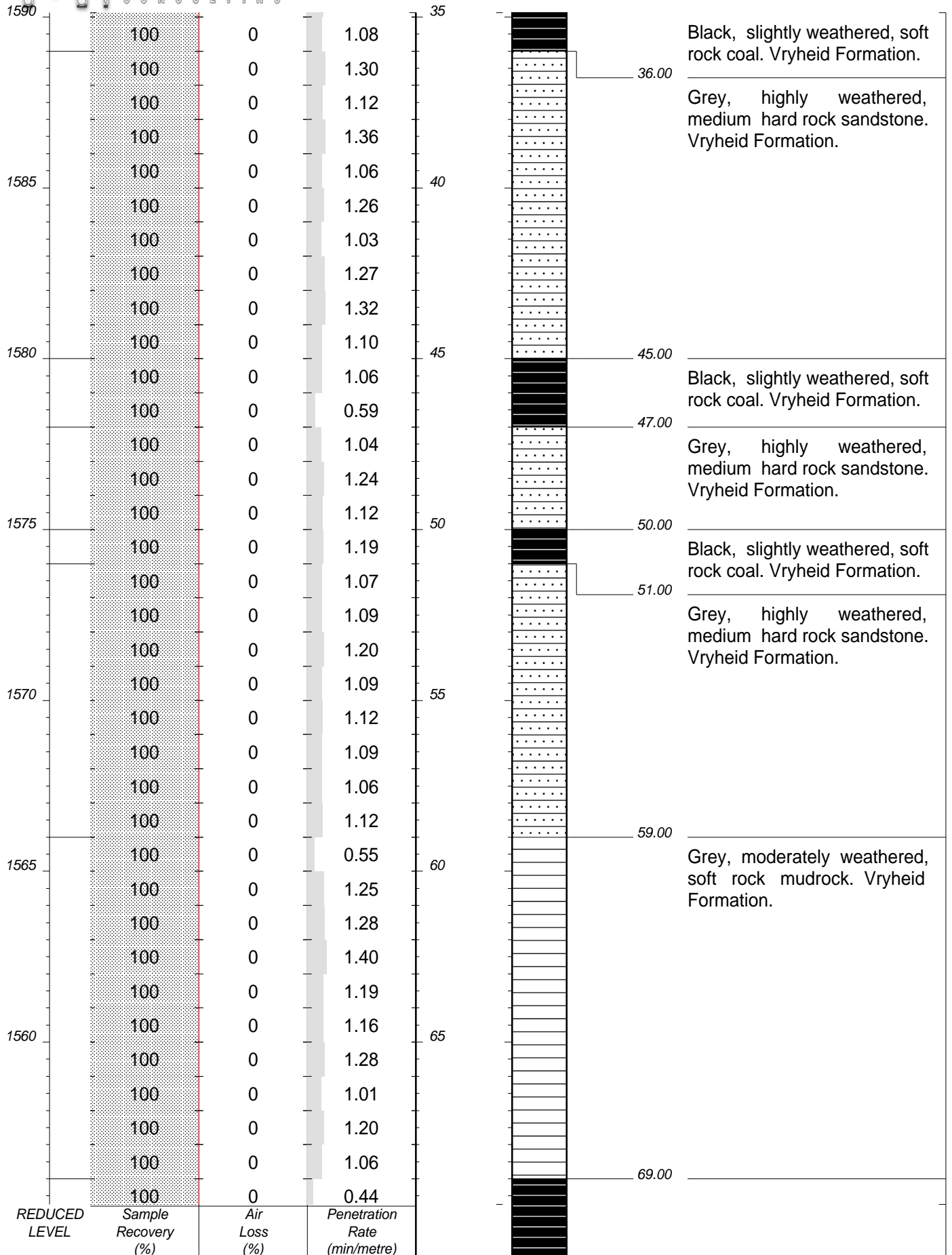
Sheet 2 of 3

JOB NUMBER: MAK152.08.23







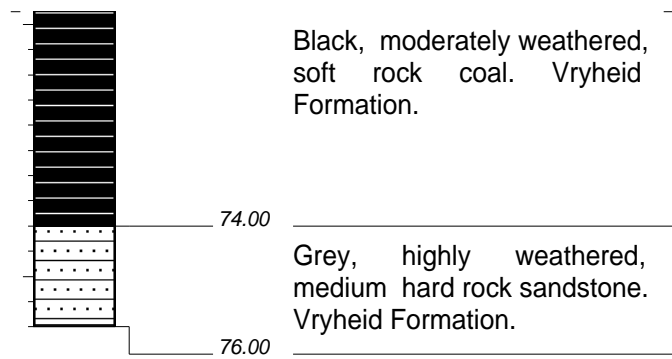
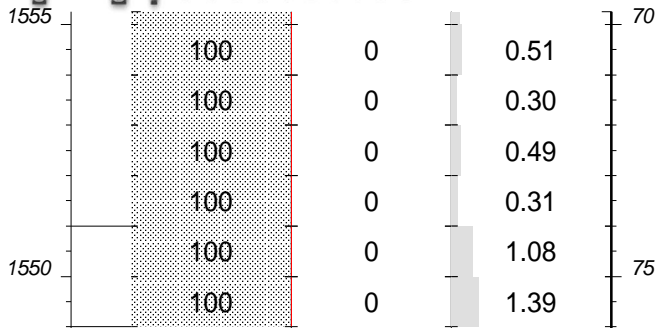




**ESKOM
PRELIMINARY GEOTECHNICAL INVESTIGATION:
KOMATI POWER STATION**

HOLE No: K-BH16
Sheet 3 of 3

JOB NUMBER: MAK152.08.23



NOTES

- 1) Water strike at 30m.
- 2) Groundwater table after twenty-four hours: 16m.
- 3) Borehole terminated at 76m due to required depth.

REDUCED LEVEL	Sample Recovery (%)	Air Loss (%)	Penetration Rate (min/metre)
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CONTRACTOR : BAV Consulting
MACHINE : Thor 5000 Drill Rig
DRILLED BY : Zakhele Zaca
PROFILED BY : Andries Vukeya

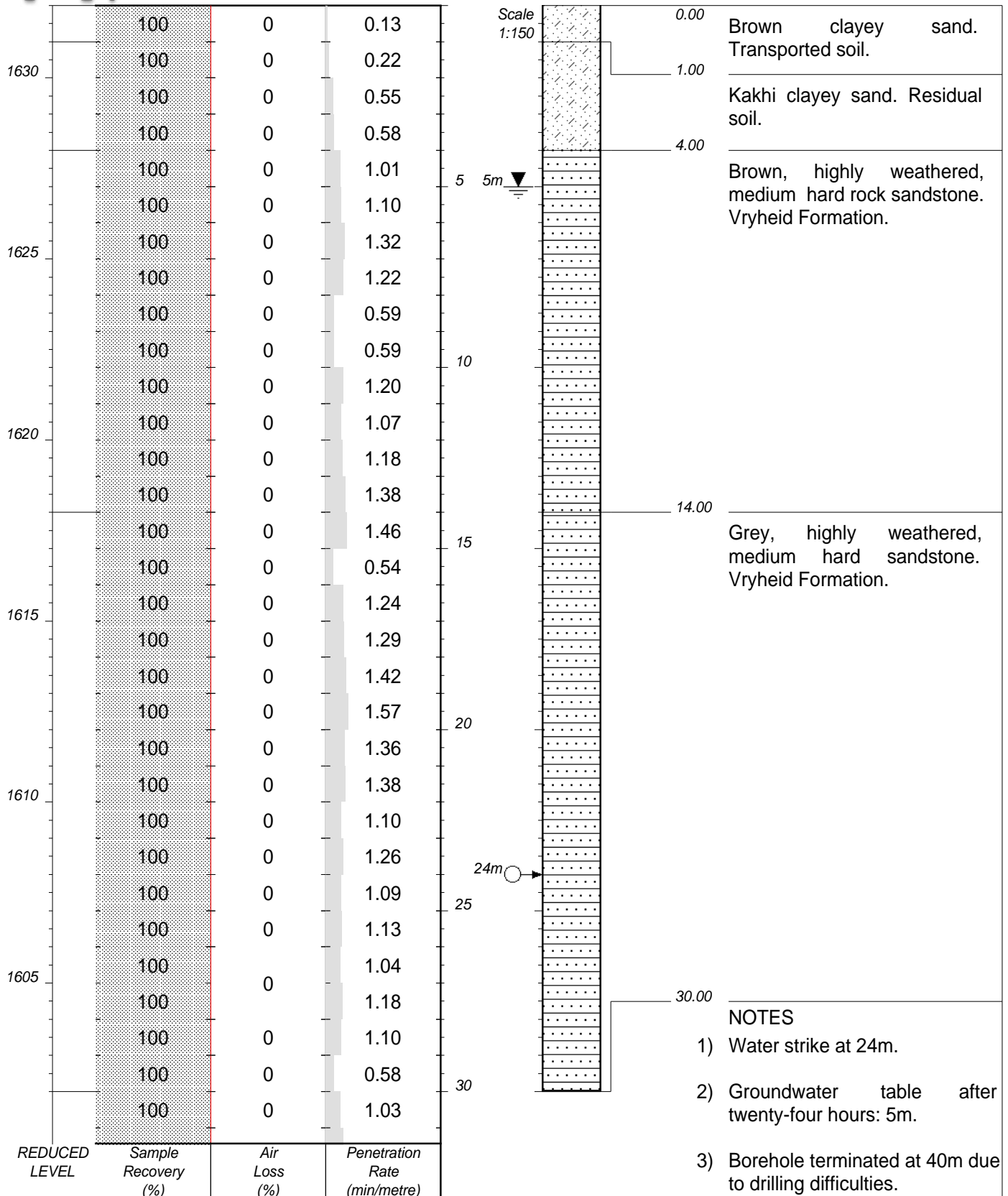
TYPE SET BY : Andries Vukeya
SETUP FILE : STANDARD.SET

INCLINATION : Vertical
DIAM : 165 mm
DATE : 08/09/2023
DATE : 08/09/2023

DATE : 06/12/2023 14:04
TEXT : ..ssionBoreholes\KBH16.TXT

ELEVATION : 1625 m
X-COORD : 29.45489°E
Y-COORD : 26.09797°S

HOLE No: K-BH16



CONTRACTOR : BAV Consulting
MACHINE : Thor 5000 Drill Rig
DRILLED BY : Zakhele Zaca
PROFIED BY : Andries Vukeya

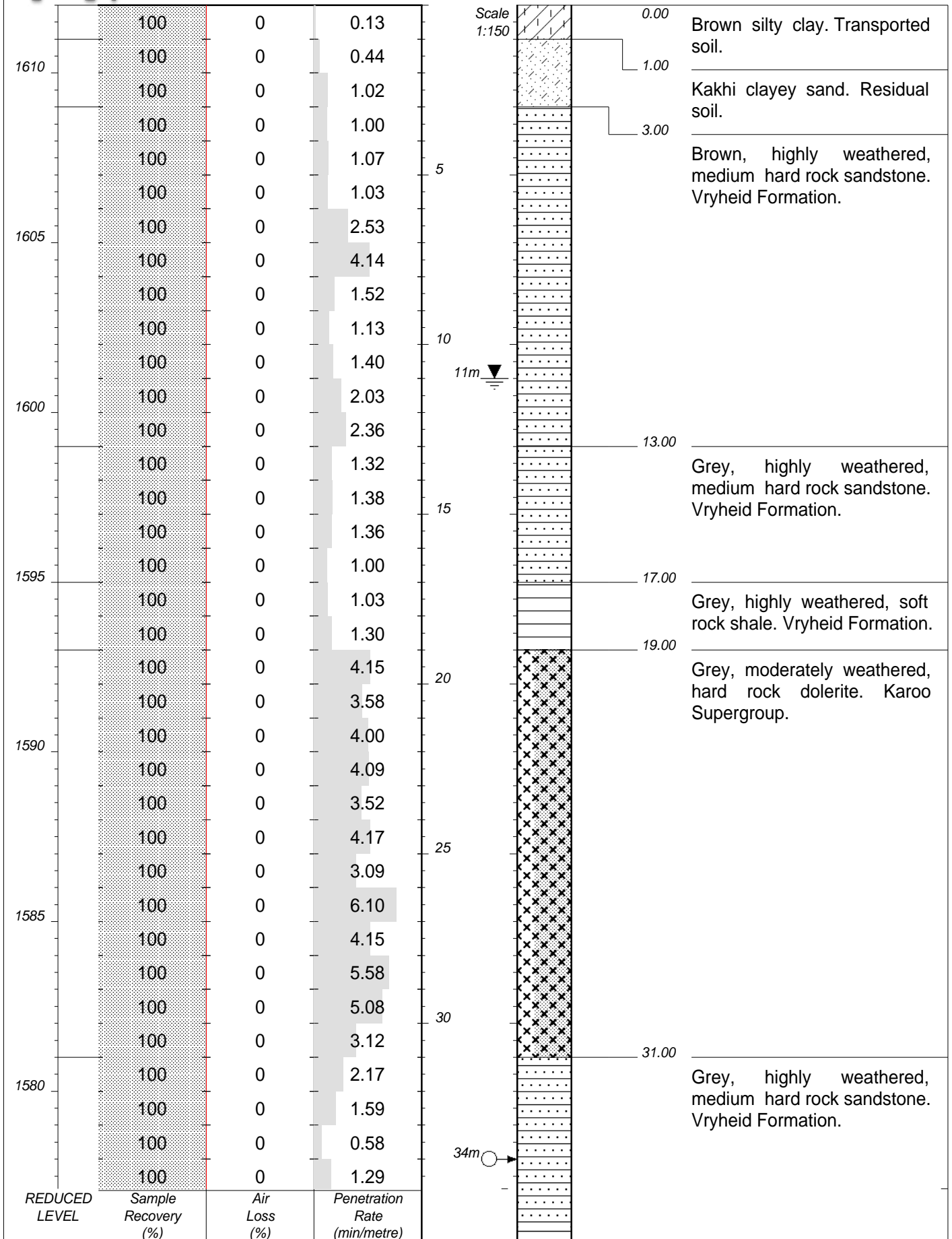
TYPE SET BY : Andries Vukeya
SETUP FILE : STANDARD.SET

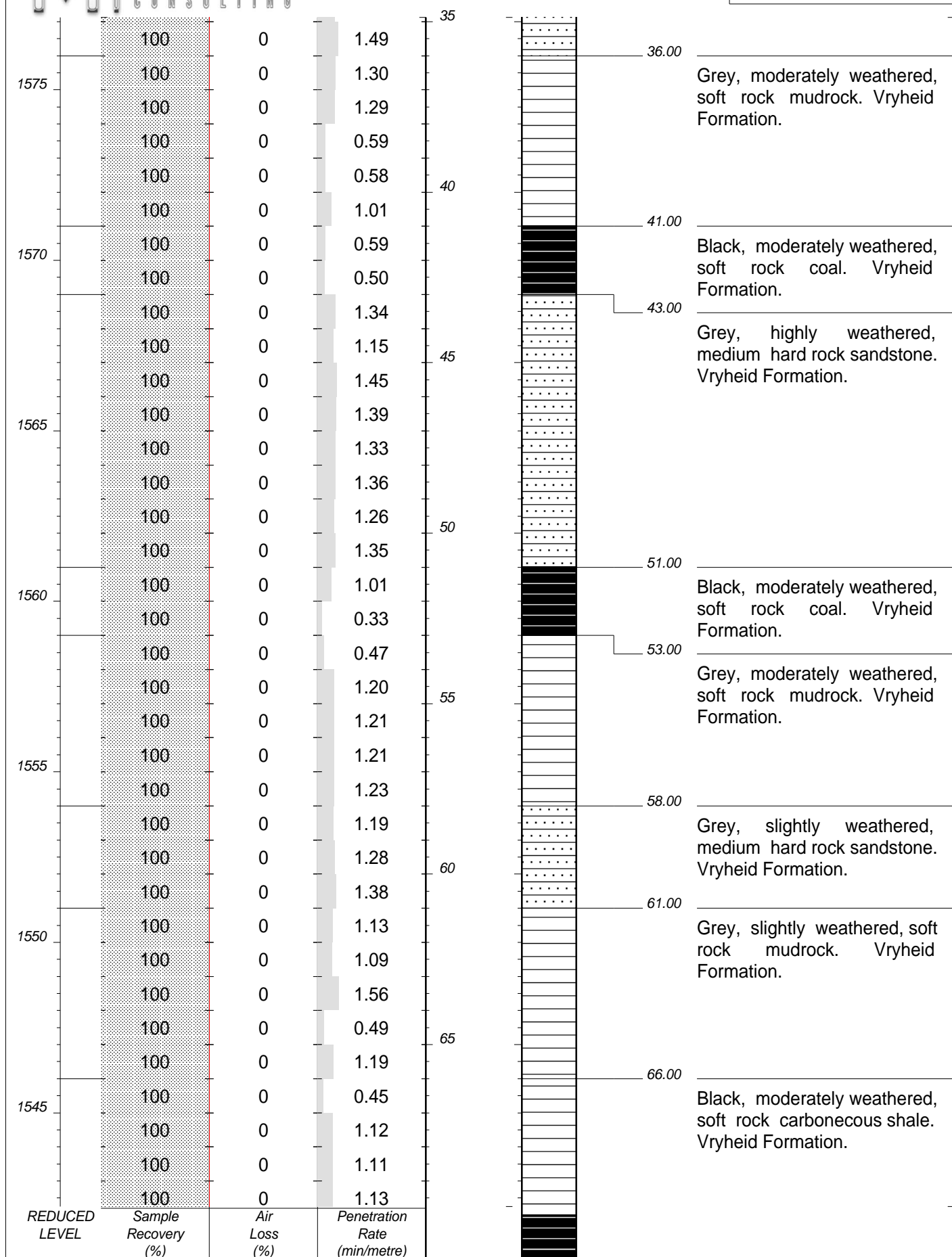
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DIAM : 165 mm
DATE : 11/09/2023
DATE : 11/09/2023

DATE : 06/12/2023 14:05
TEXT : ..ssionBoreholes\KBH17.TXT

ELEVATION : 1632 m
X-COORD : 29.45122°E
Y-COORD : 26.10133°S

HOLE No: K-BH17



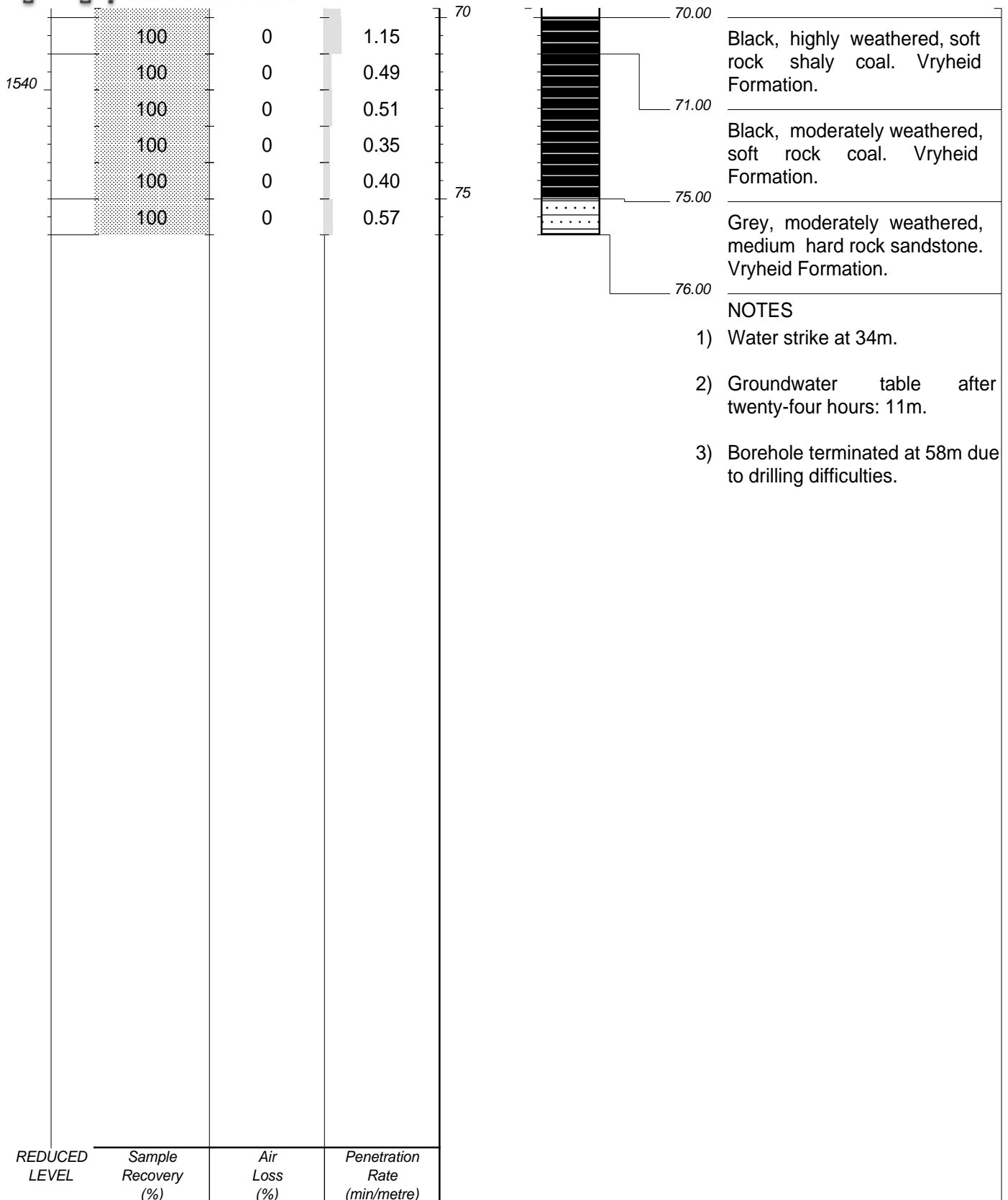




**ESKOM
PRELIMINARY GEOTECHNICAL INVESTIGATION:
KOMATI POWER STATION**

HOLE No: K-BH18
Sheet 3 of 3

JOB NUMBER: MAK152.08.23



CONTRACTOR : BAV Consulting
MACHINE : Thor 5000 Drill Rig
DRILLED BY : Zakhele Zaca
PROFILED BY : Andries Vukeya

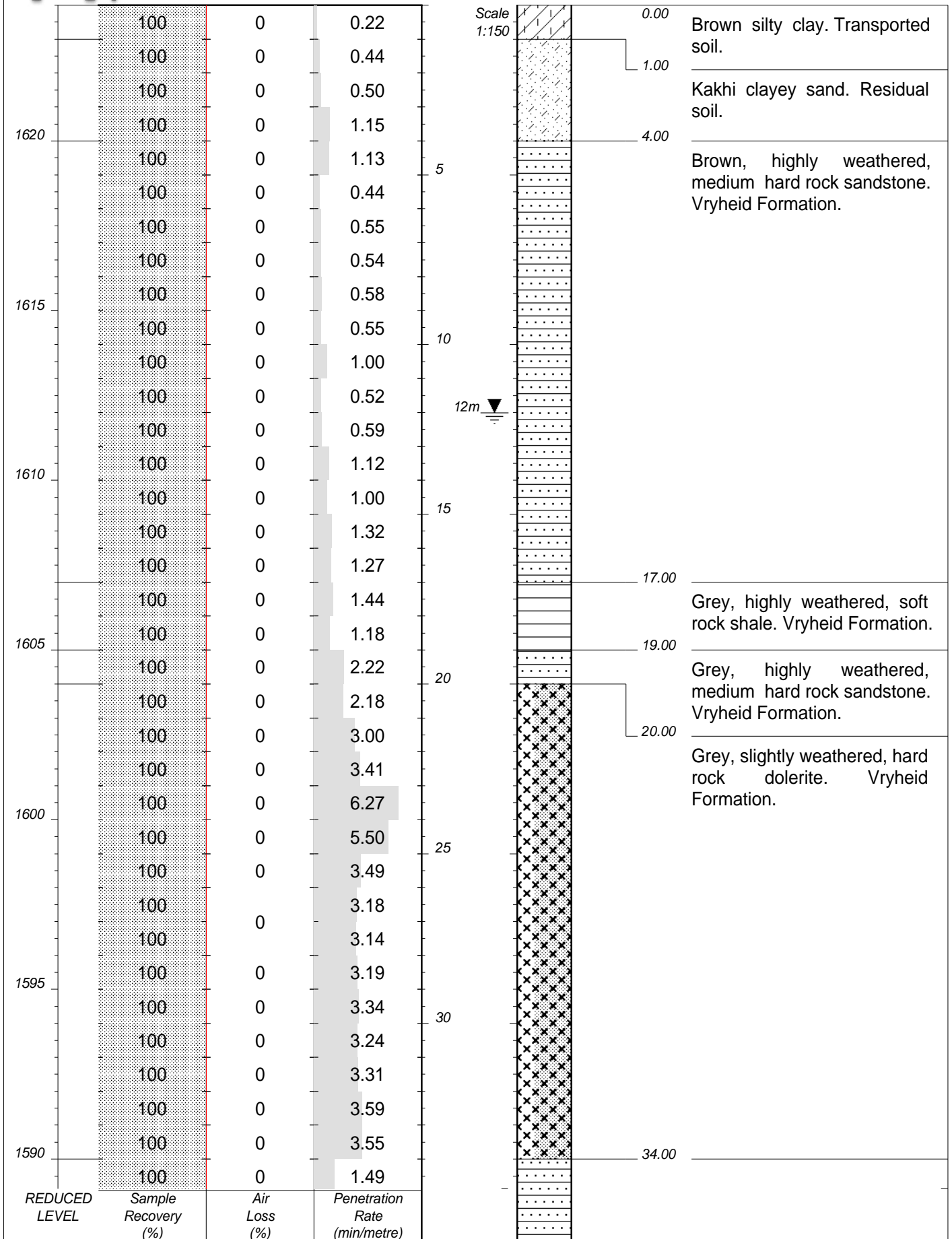
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DATE : 12/09/2023
DATE : 12/09/2023

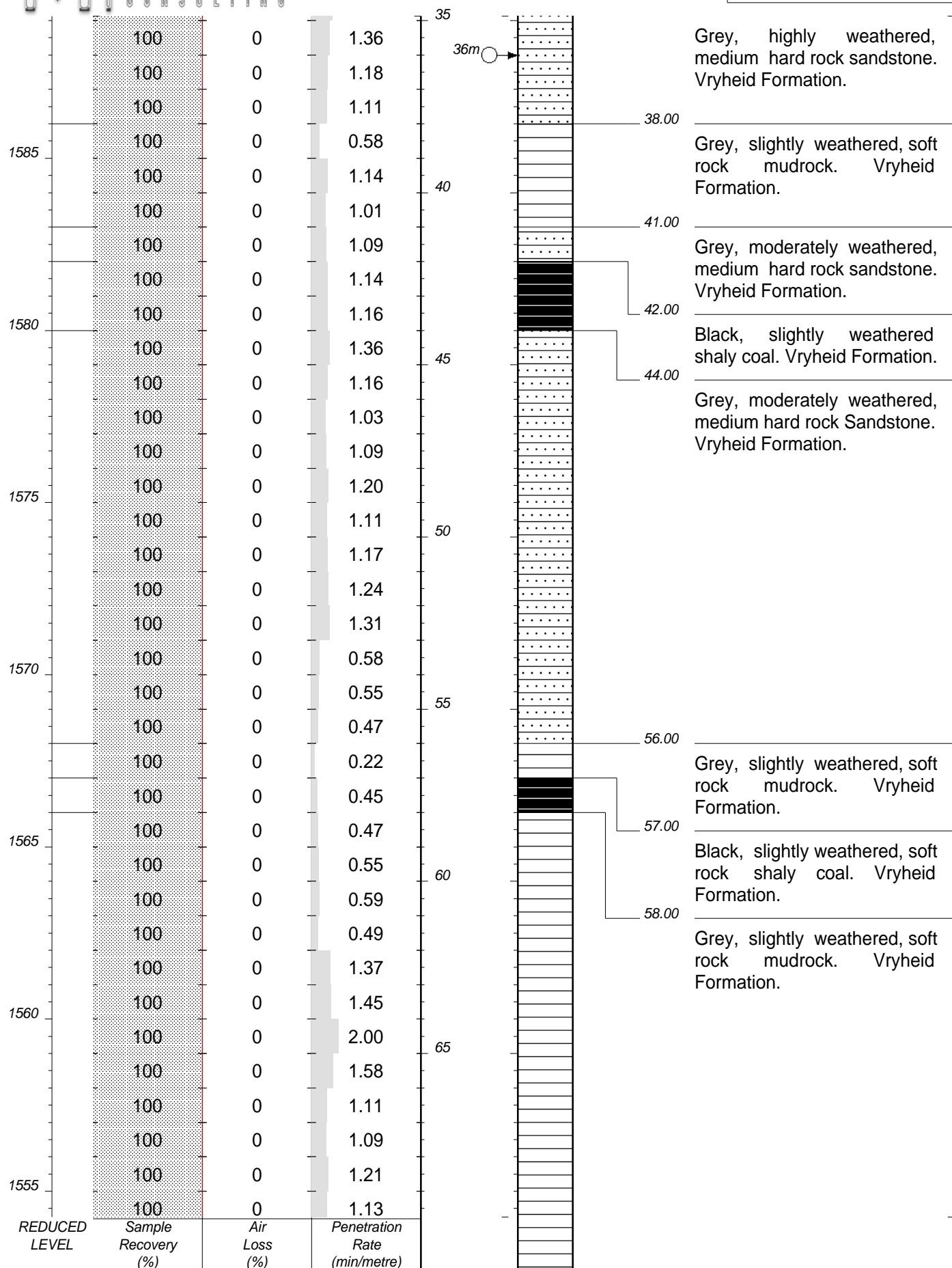
ELEVATION : 1612 m
X-COORD : 29.45352°E
Y-COORD : 26.09799°S

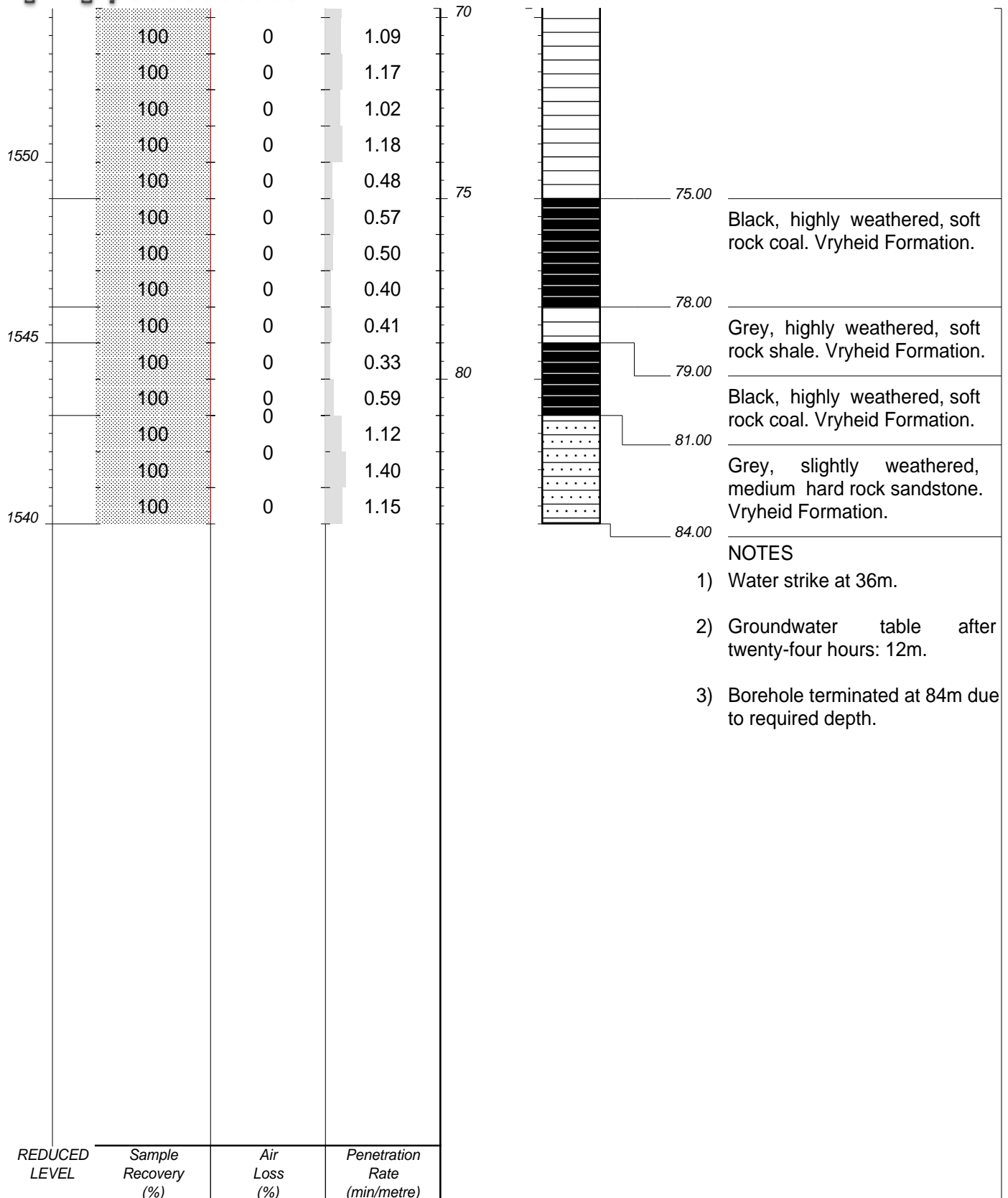
TYPE SET BY : Andries Vukeya
SETUP FILE : STANDARD.SET

DATE : 06/12/2023 14:06
TEXT : ..ssionBoreholes\KBH18.TXT

HOLE No: K-BH18







CONTRACTOR : BAV Consulting
MACHINE : Thor 5000 Drill Rig
DRILLED BY : Zakhele Zaca
PROFIED BY : Andries Vukeya

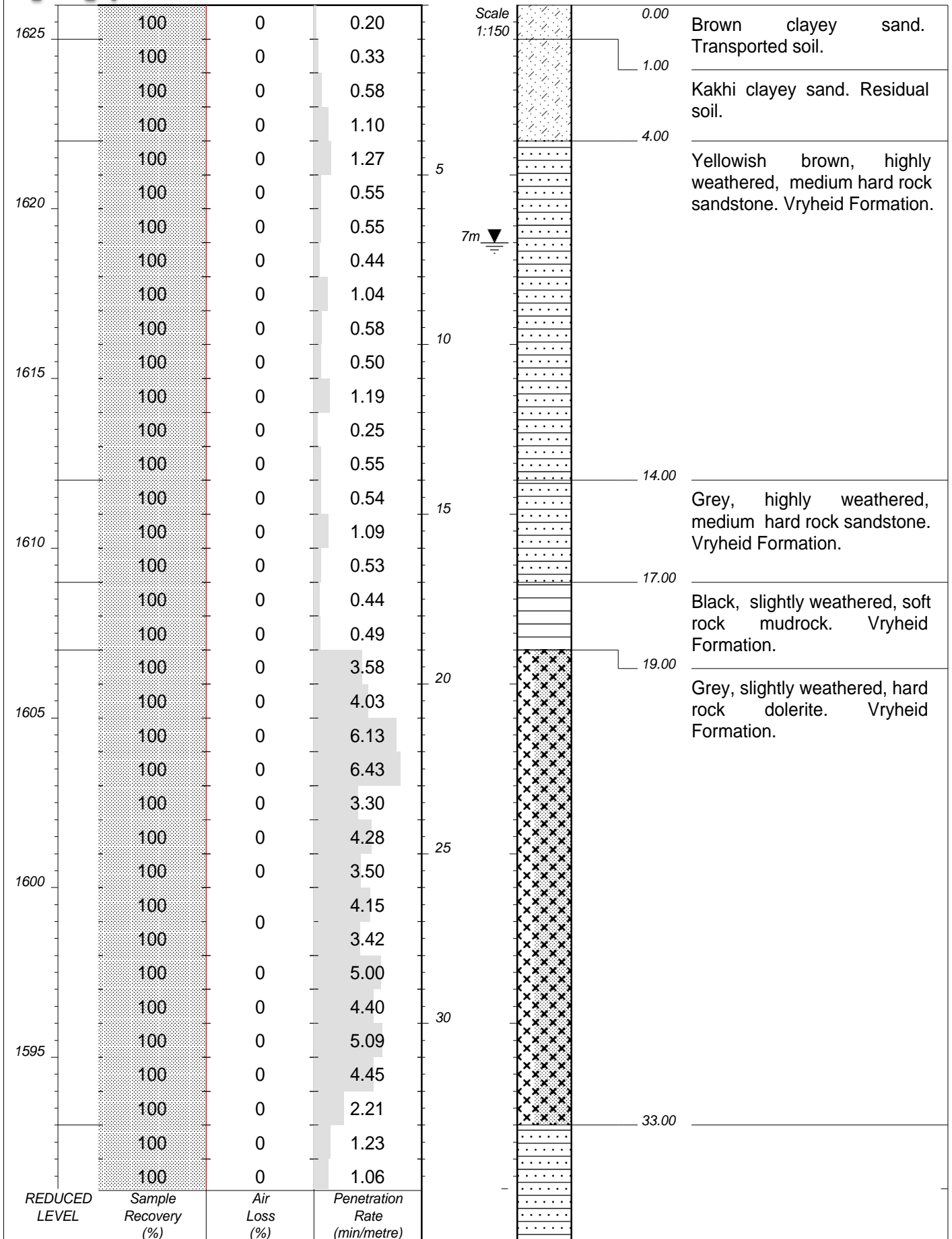
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SETUP FILE : STANDARD.SET

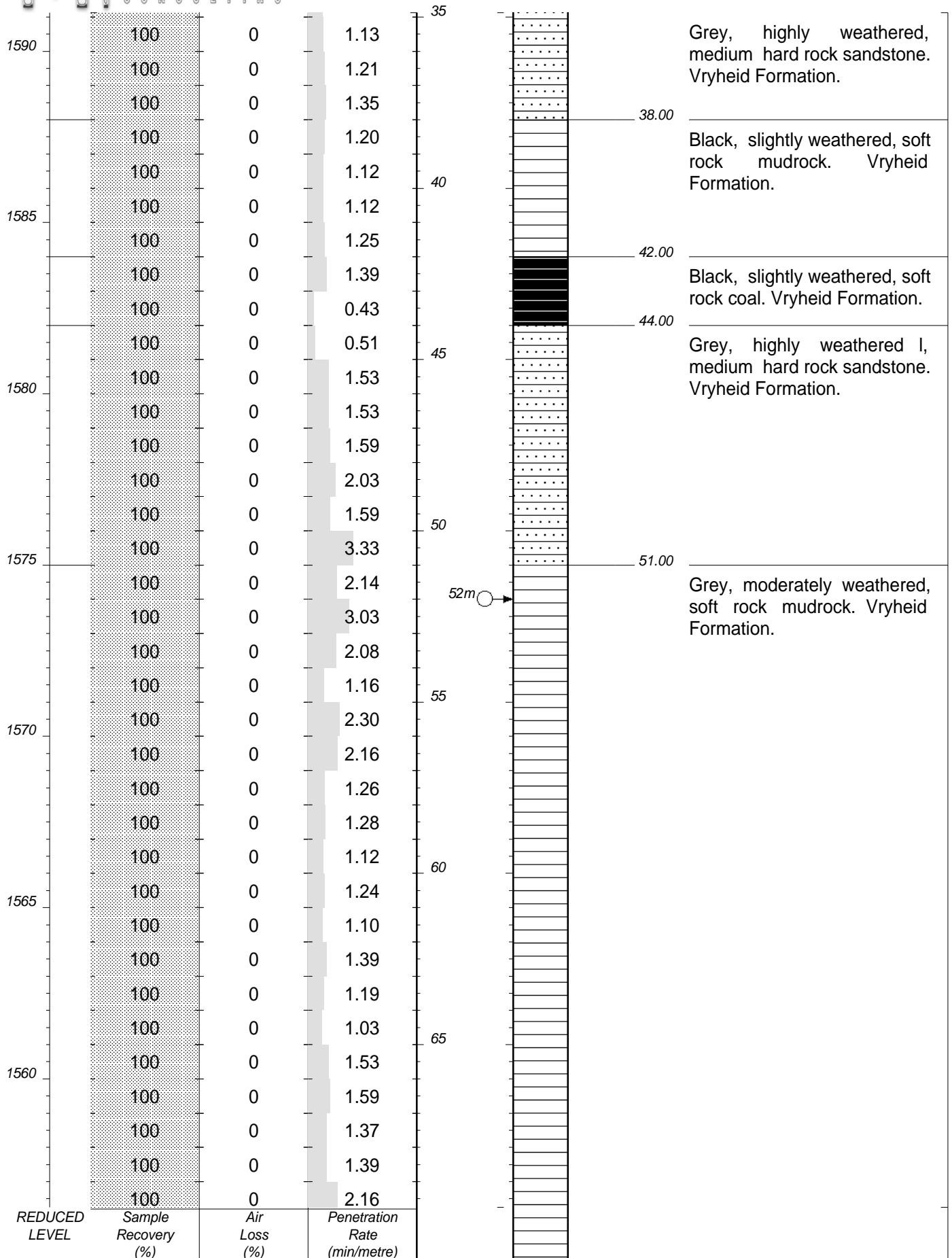
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DATE : 12/09/2023

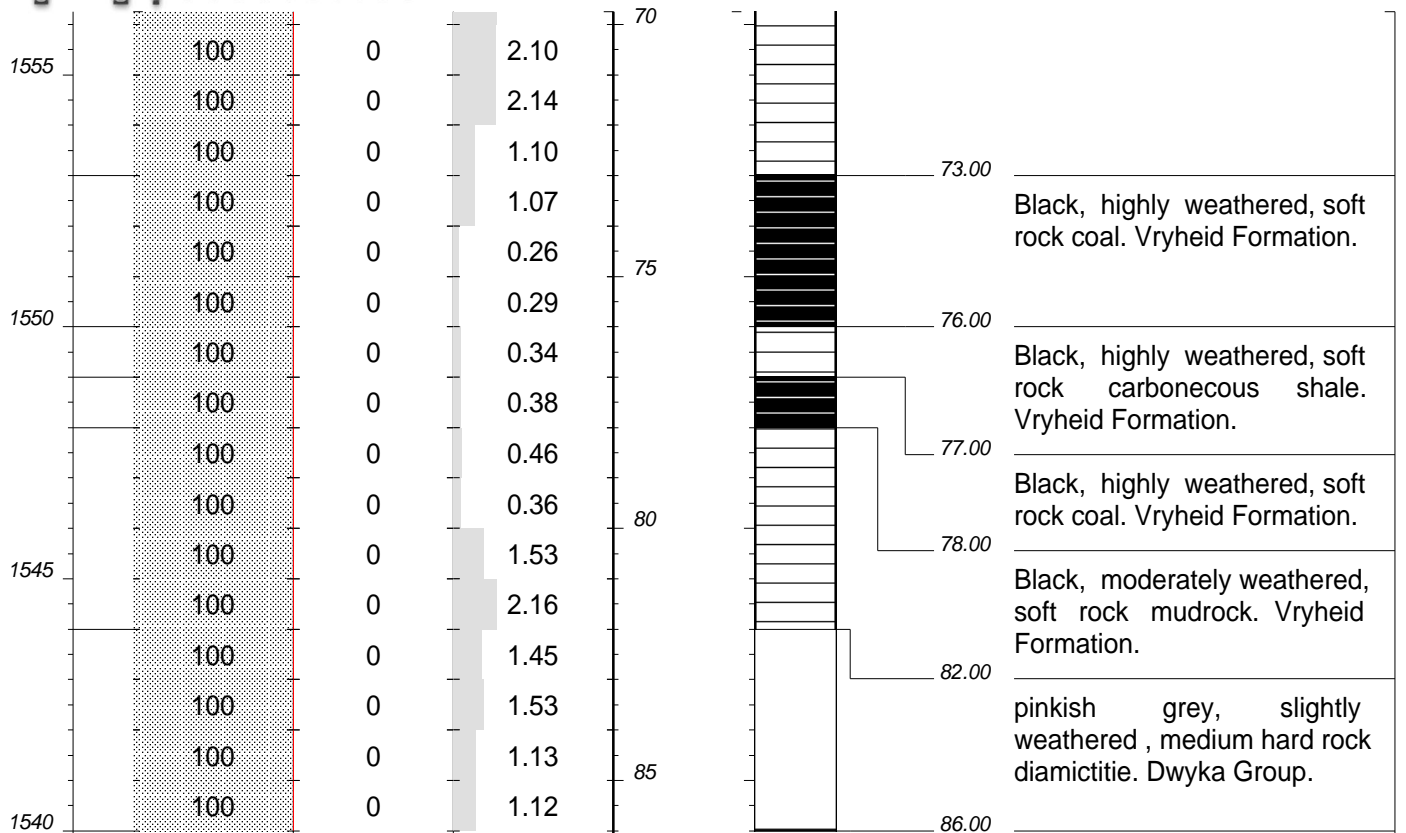
DATE : 06/12/2023 14:07
TEXT : ..ssionBoreholes\KBH19.TXT

ELEVATION : 1624 m
X-COORD : 29.45159°E
Y-COORD : 26.09822°S

HOLE No: K-BH19






NOTES

- 1) Water strike at 52m.
- 2) Groundwater table after twenty-four hours: 7m.
- 3) Borehole terminated at 86m due to required depth.

REDUCED LEVEL	Sample Recovery (%)	Air Loss (%)	Penetration Rate (min/metre)
---------------	---------------------	--------------	------------------------------

CONTRACTOR : BAV Consulting
MACHINE : Thor 5000 Drill Rig
DRILLED BY : Zakhele Zaca
PROFILED BY : Andries Vukeya

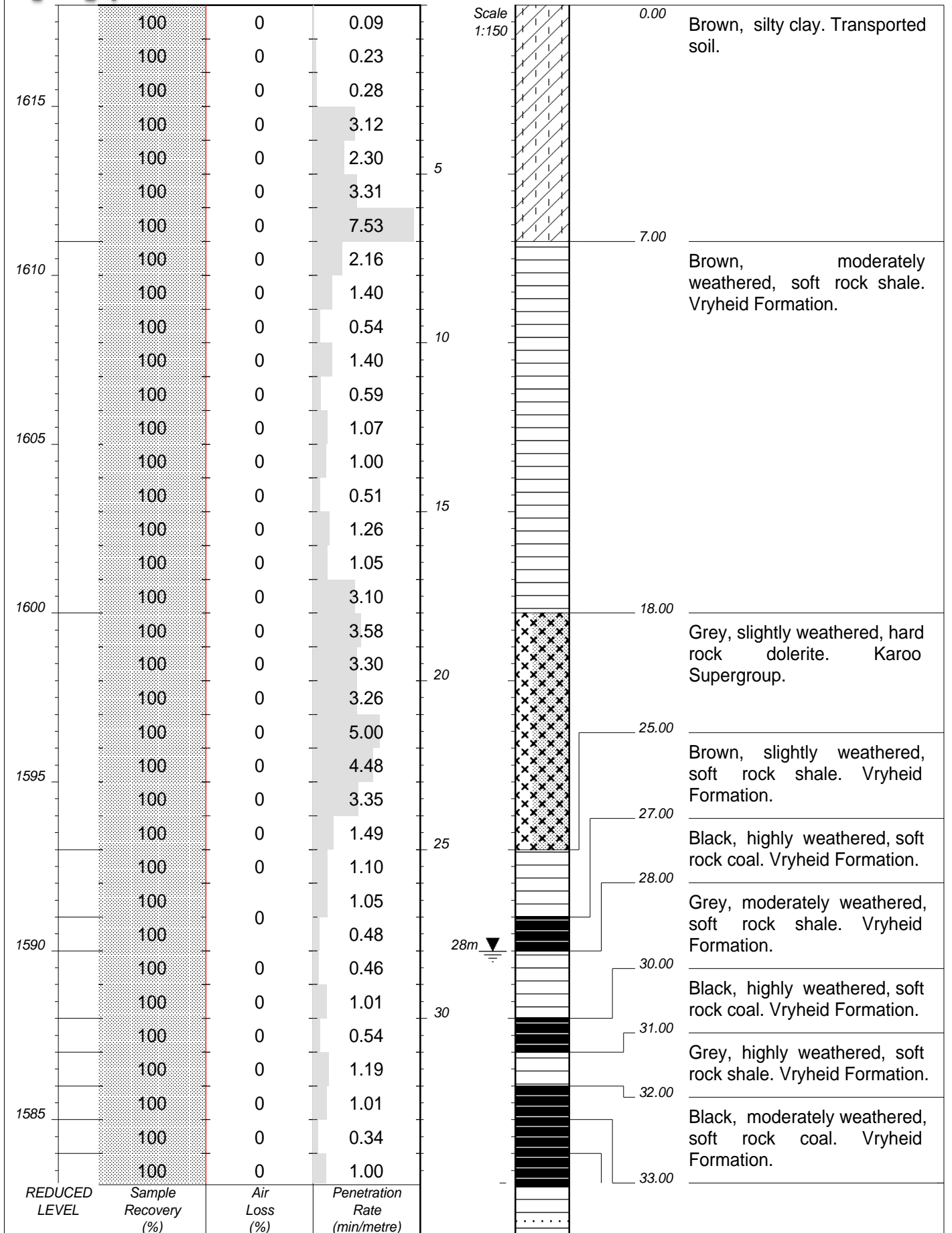
TYPE SET BY : Andries Vukeya
SETUP FILE : STANDARD.SET

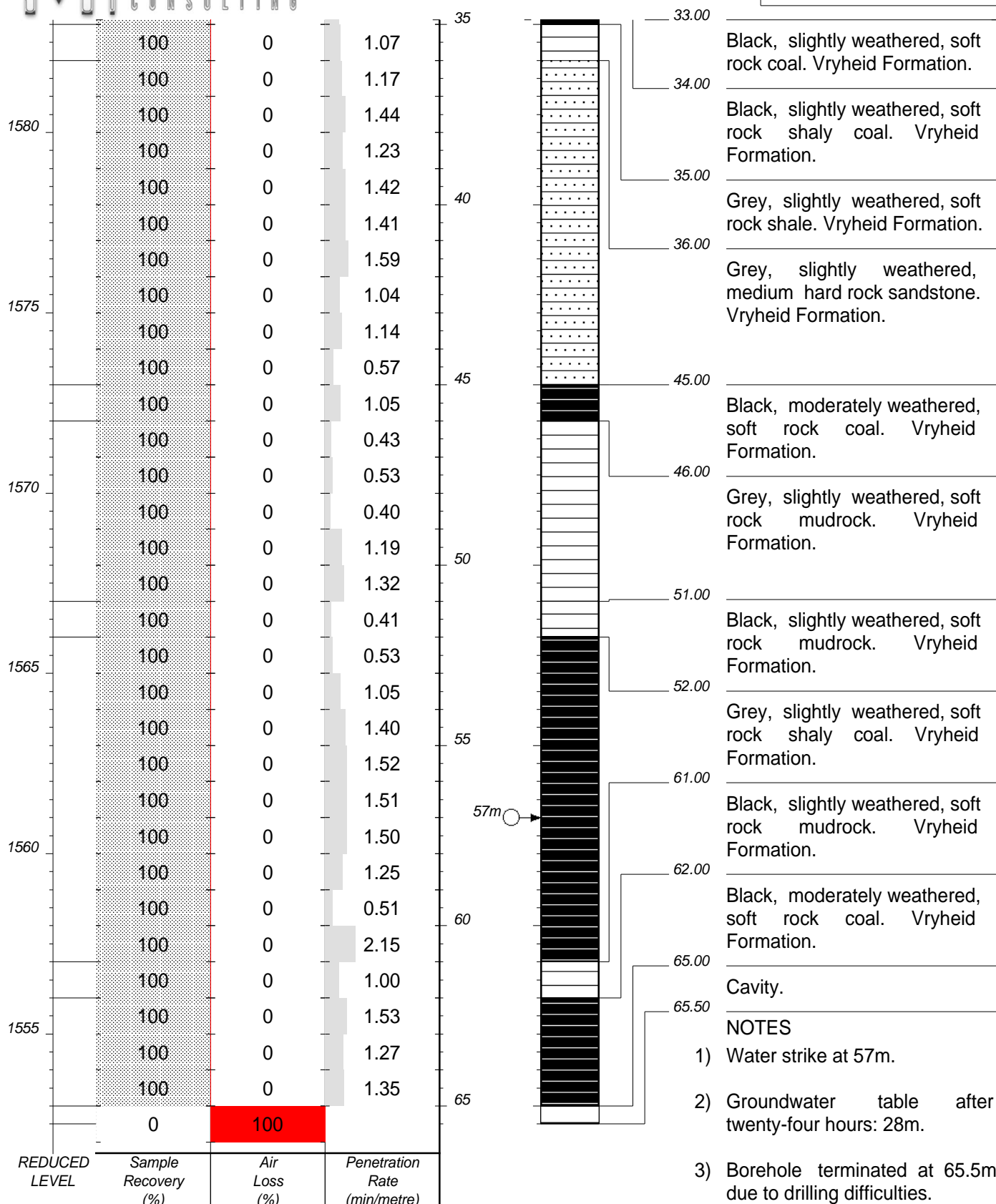
INCLINATION : Vertical
DIAM : 165 mm
DATE : 11/09/2023
DATE : 11/09/2023

DATE : 06/12/2023 14:07
TEXT : ..ssionBoreholes\KBH20.TXT

ELEVATION : 1626 m
X-COORD : 29.45272°E
Y-COORD : 26.09837°S

HOLE No: K-BH20





CONTRACTOR : BAV Consulting
MACHINE : Thor 5000 Drill Rig
DRILLED BY : Zakhele Zaca
PROFIED BY : Andries Vukeya

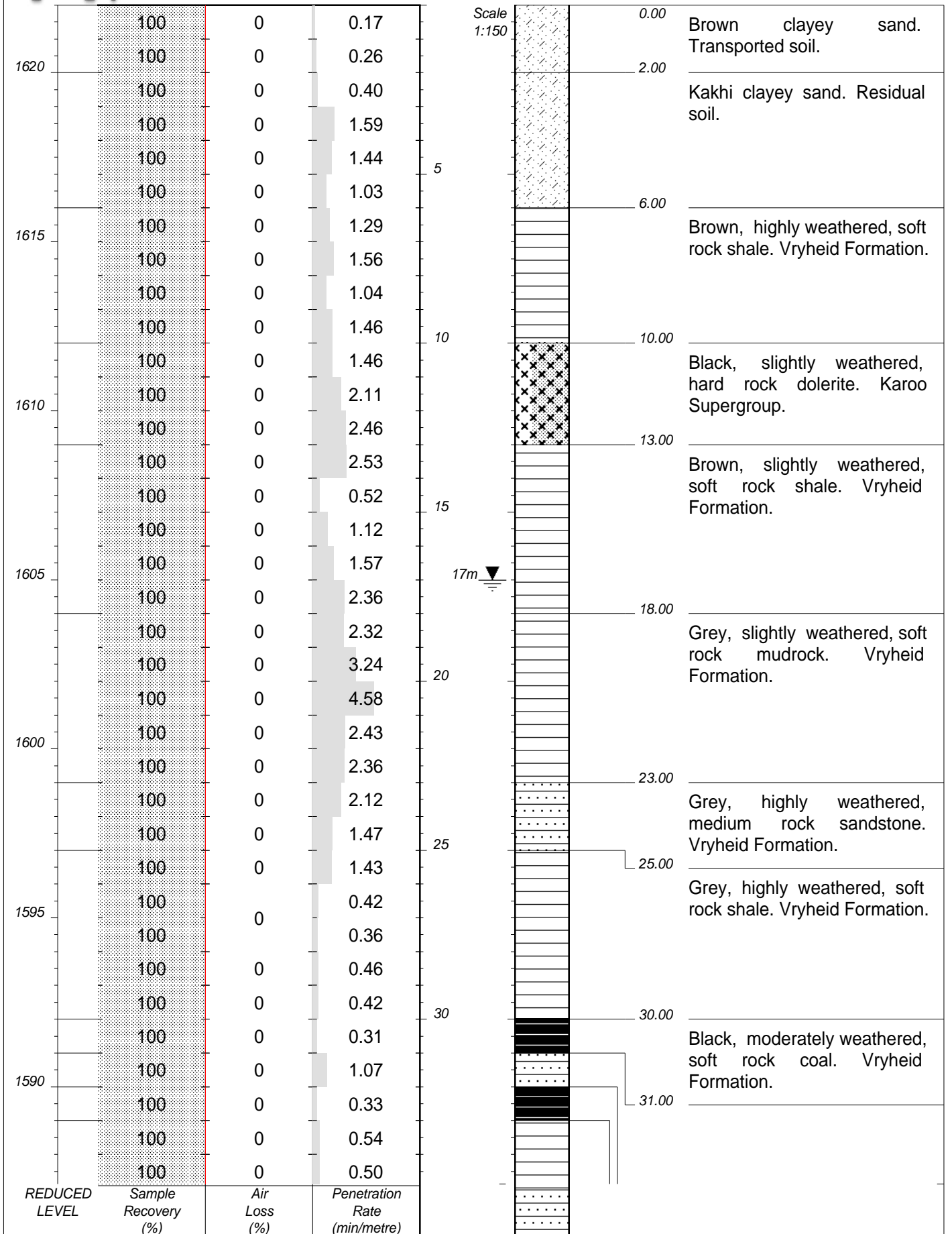
TYPE SET BY : Andries Vukeya
SETUP FILE : STANDARD.SET

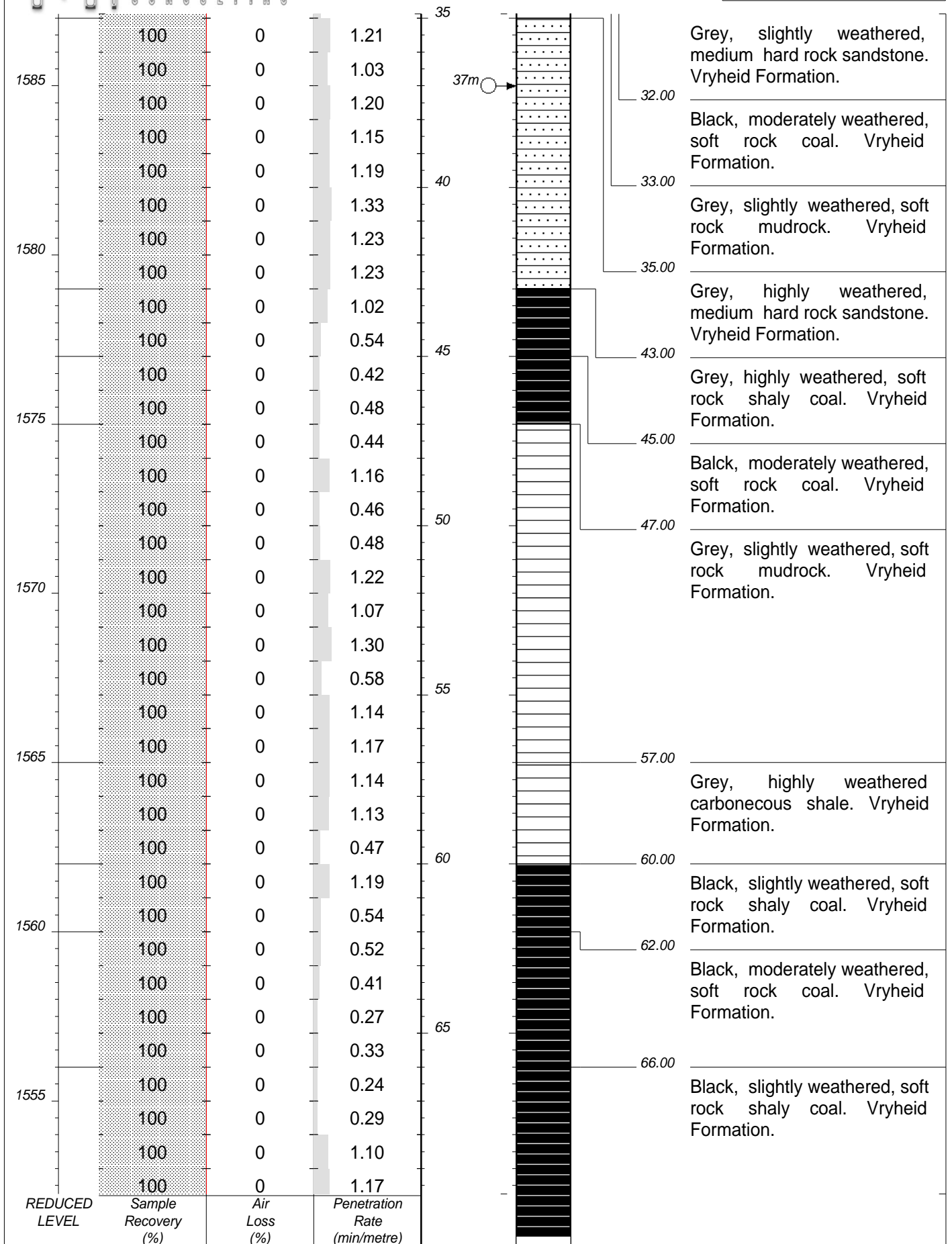
INCLINATION : Vertical
DIAM : 165 mm
DATE : 01/09/2023
DATE : 01/09/2023

DATE : 06/12/2023 14:08
TEXT : ..ssionBoreholes\KBH21.TXT

ELEVATION : 1618 m
X-COORD : 29.45028°E
Y-COORD : 26.09487°S

HOLE No: K-BH21



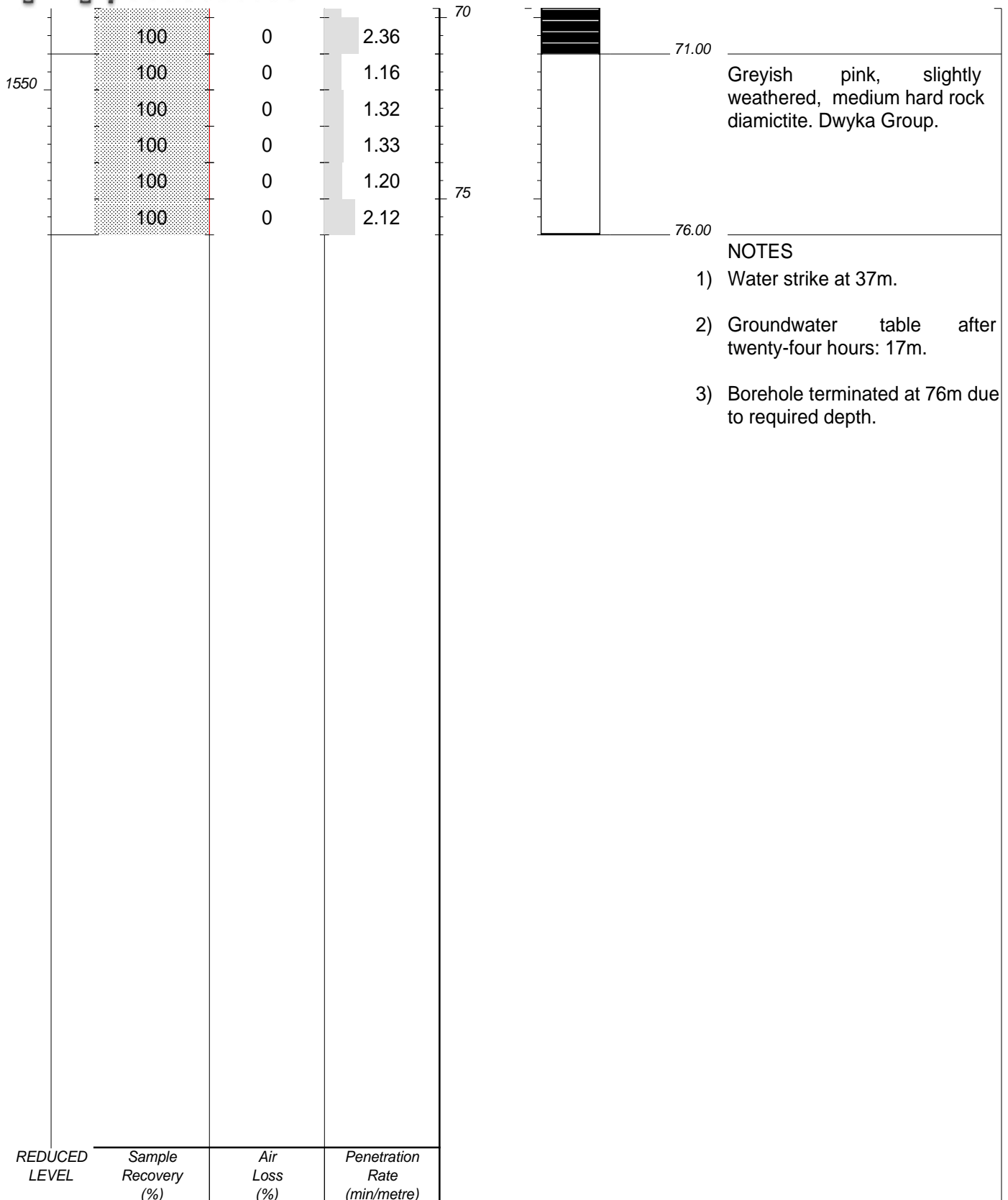




**ESKOM
PRELIMINARY GEOTECHNICAL INVESTIGATION:
KOMATI POWER STATION**

HOLE No: K-BH22
Sheet 3 of 3

JOB NUMBER: MAK152.08.23



CONTRACTOR : BAV Consulting
MACHINE : Thor 5000 Drill Rig
DRILLED BY : Zakhele Zaca
PROFIED BY : Andries Vukeya

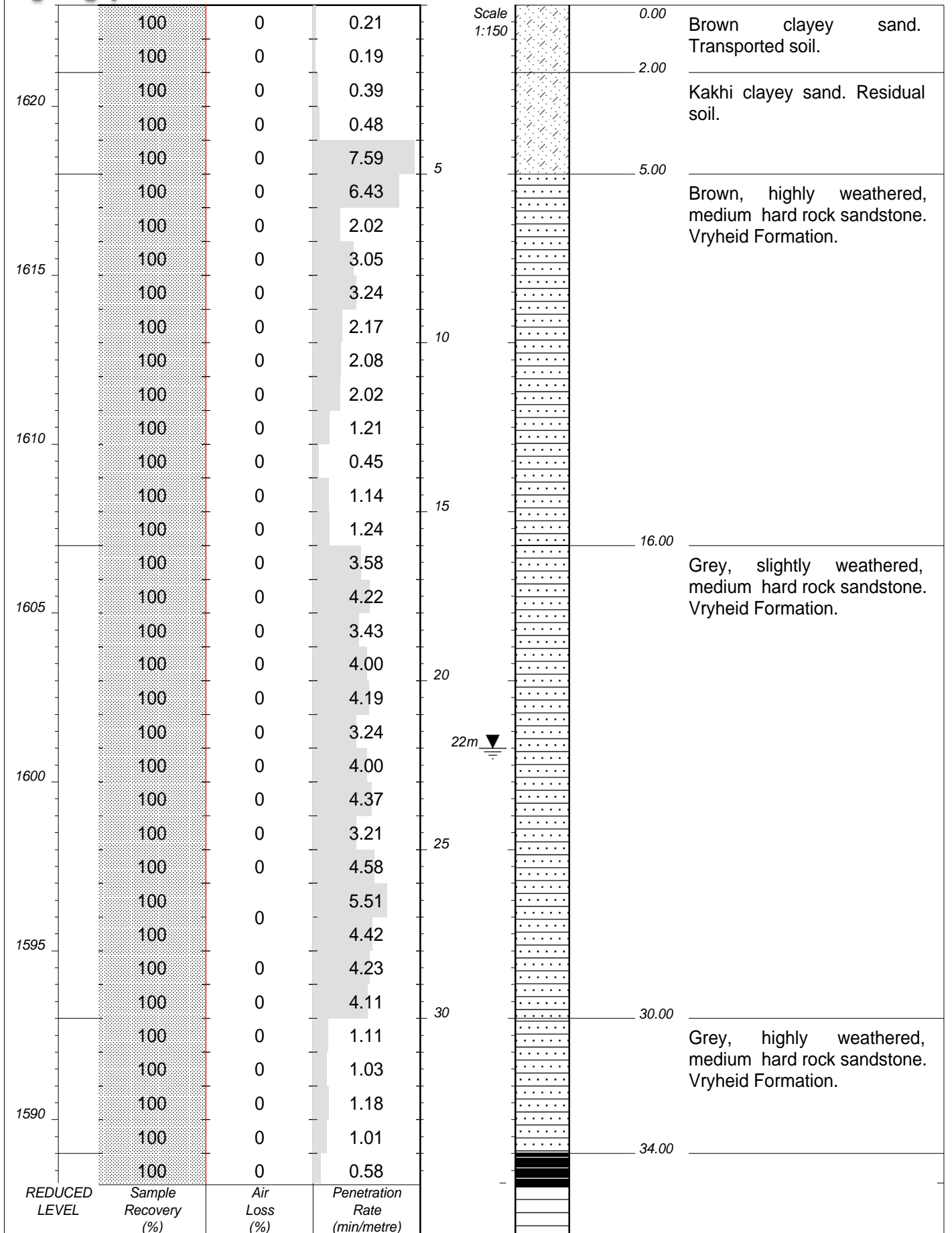
INCLINATION : Vertical
DIAM : 165 mm
DATE : 04/09/2023
DATE : 04/09/2023

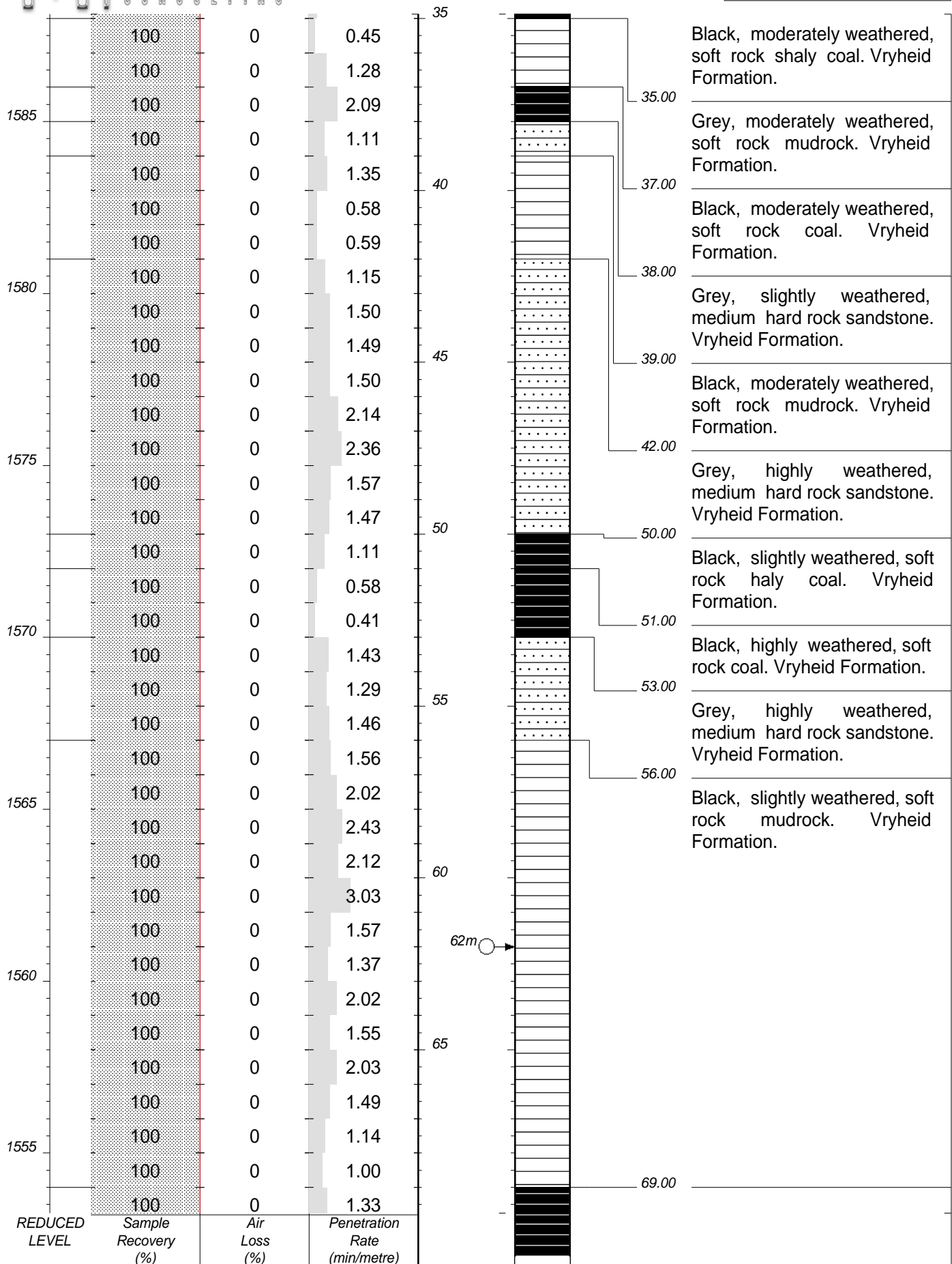
ELEVATION : 1622 m
X-COORD : 29.45245°E
Y-COORD : 26.09507°S

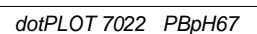
TYPE SET BY : Andries Vukeya
SETUP FILE : STANDARD.SET

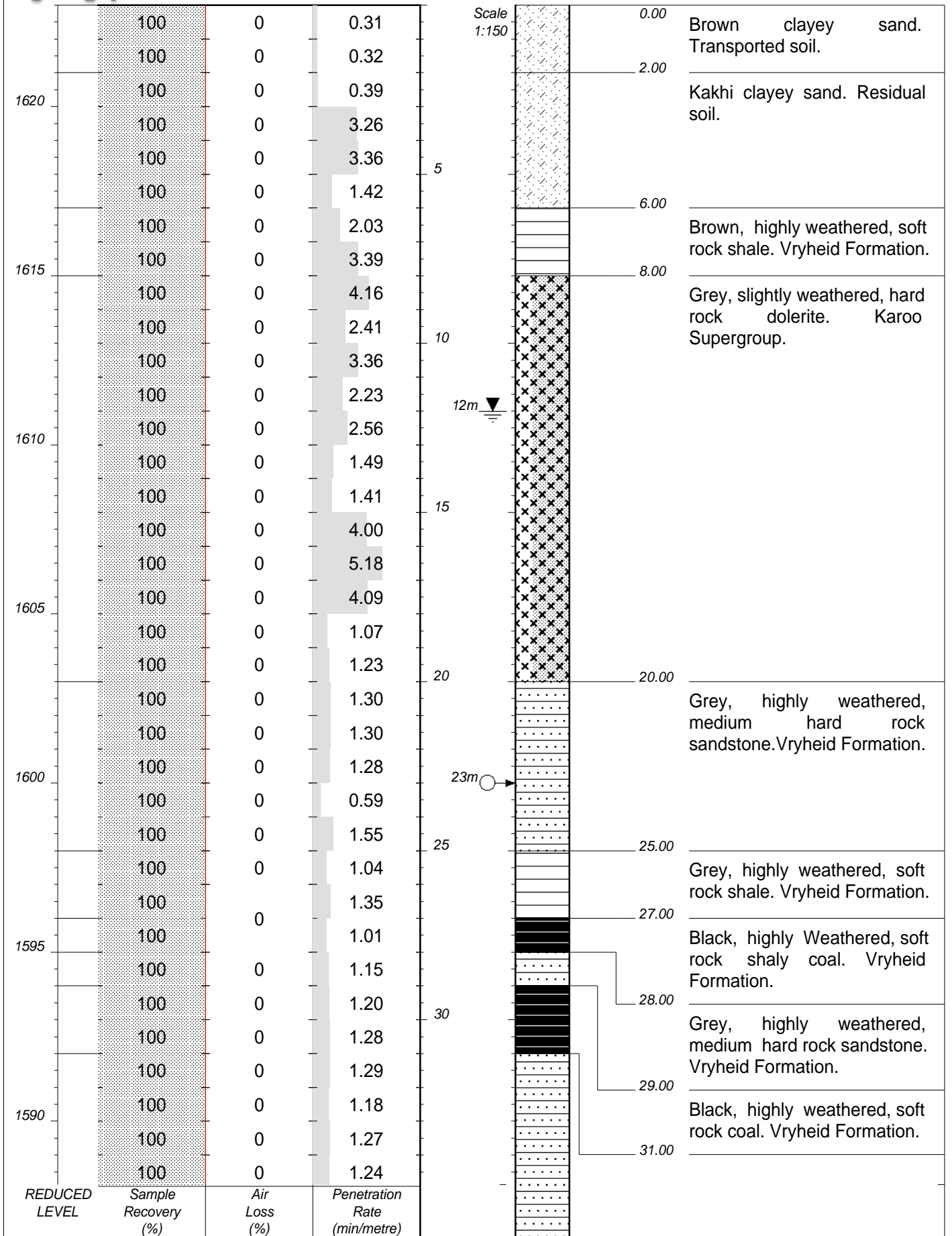
DATE : 06/12/2023 14:10
TEXT : ..ssionBoreholes\KBH22.TXT

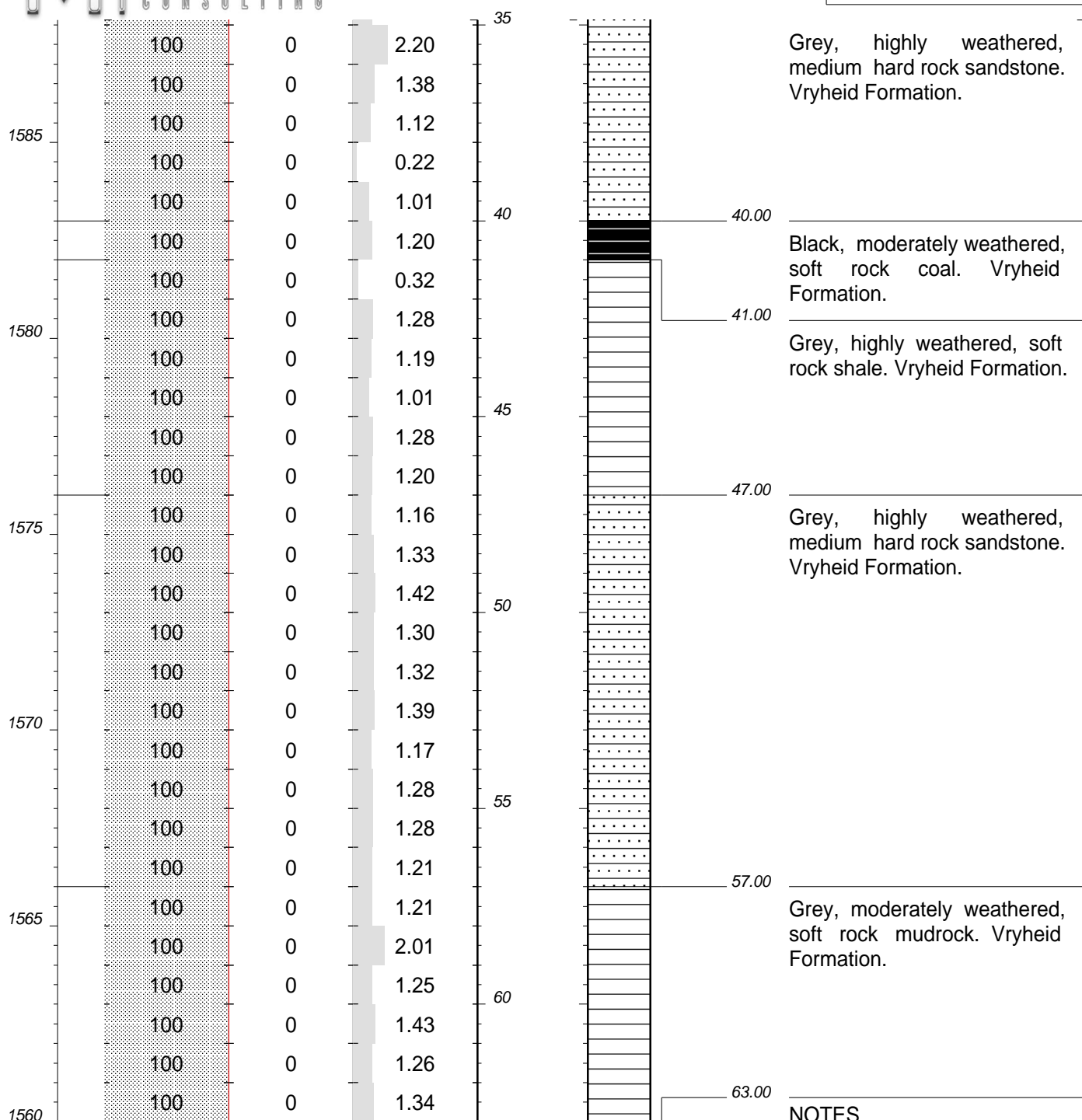
HOLE No: K-BH22











- NOTES**
- 1) Water strike at 23m.
 - 2) Groundwater table after twenty-four hours: 12m.
 - 3) Borehole terminated at 63m due to required depth.

CONTRACTOR : BAV Consulting
MACHINE : Thor 5000 Drill Rig
DRILLED BY : Zakhele Zaca
PROFIED BY : Andries Vukeya

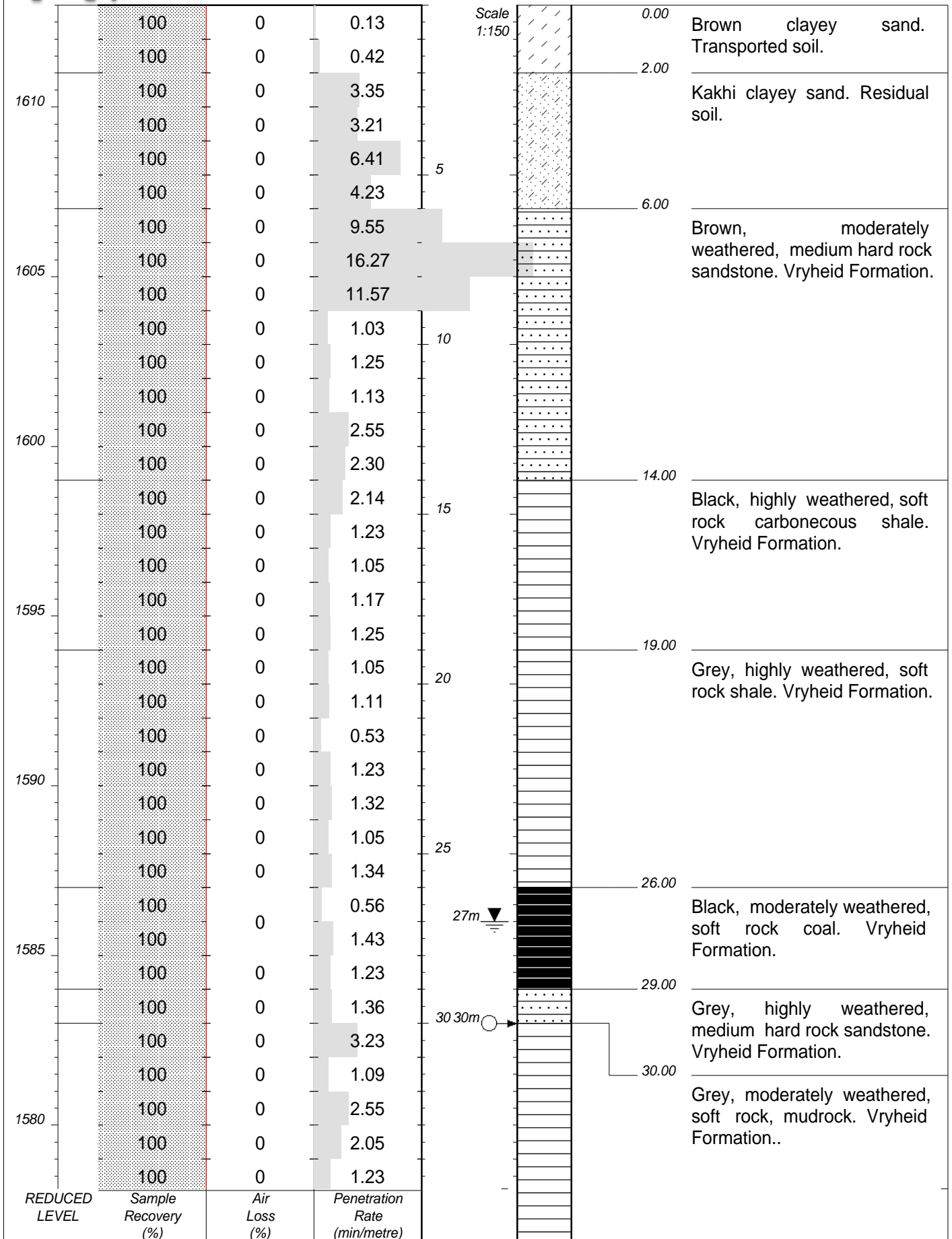
TYPE SET BY : Andries Vukeya
SETUP FILE : STANDARD.SET

INCLINATION : Vertical
DIAM : 165 mm
DATE : 14/09/2023
DATE : 14/09/2023

DATE : 06/12/2023 14:11
TEXT : ..ssionBoreholes\KBH24.TXT

ELEVATION : 1623 m
X-COORD : 29.45520°E
Y-COORD : 26.09635°S

HOLE No: K-BH24

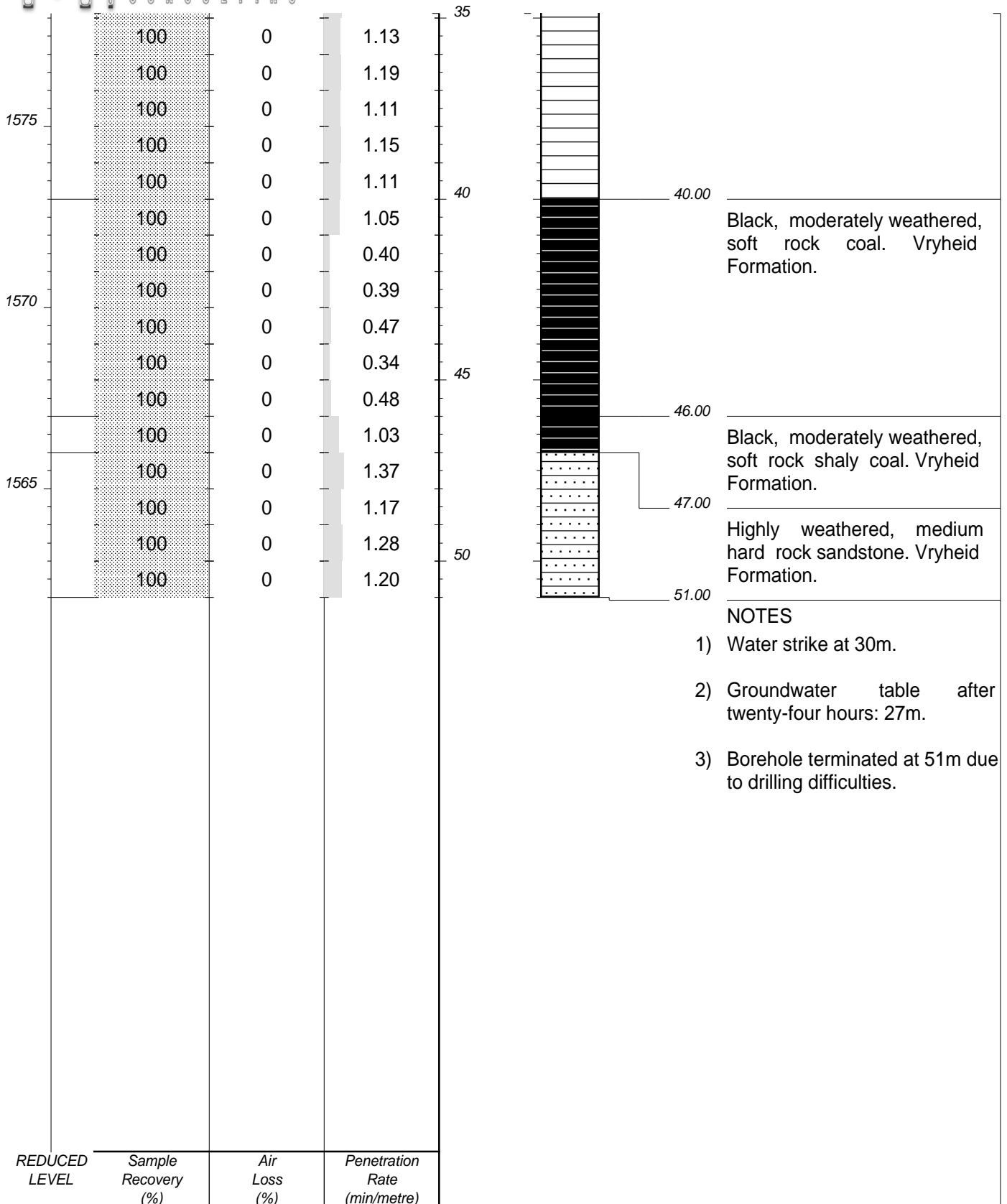




**ESKOM
PRELIMINARY GEOTECHNICAL INVESTIGATION:
KOMATI POWER STATION**

HOLE No: K-BH25
Sheet 2 of 2

JOB NUMBER: MAK152.08.23



NOTES

- 1) Water strike at 30m.
- 2) Groundwater table after twenty-four hours: 27m.
- 3) Borehole terminated at 51m due to drilling difficulties.

CONTRACTOR : BAV Consulting
MACHINE : Thor 5000 Drill Rig
DRILLED BY : Zakhele Zaca
PROFIED BY : Andries Vukeya

TYPE SET BY : Andries Vukeya
SETUP FILE : STANDARD.SET

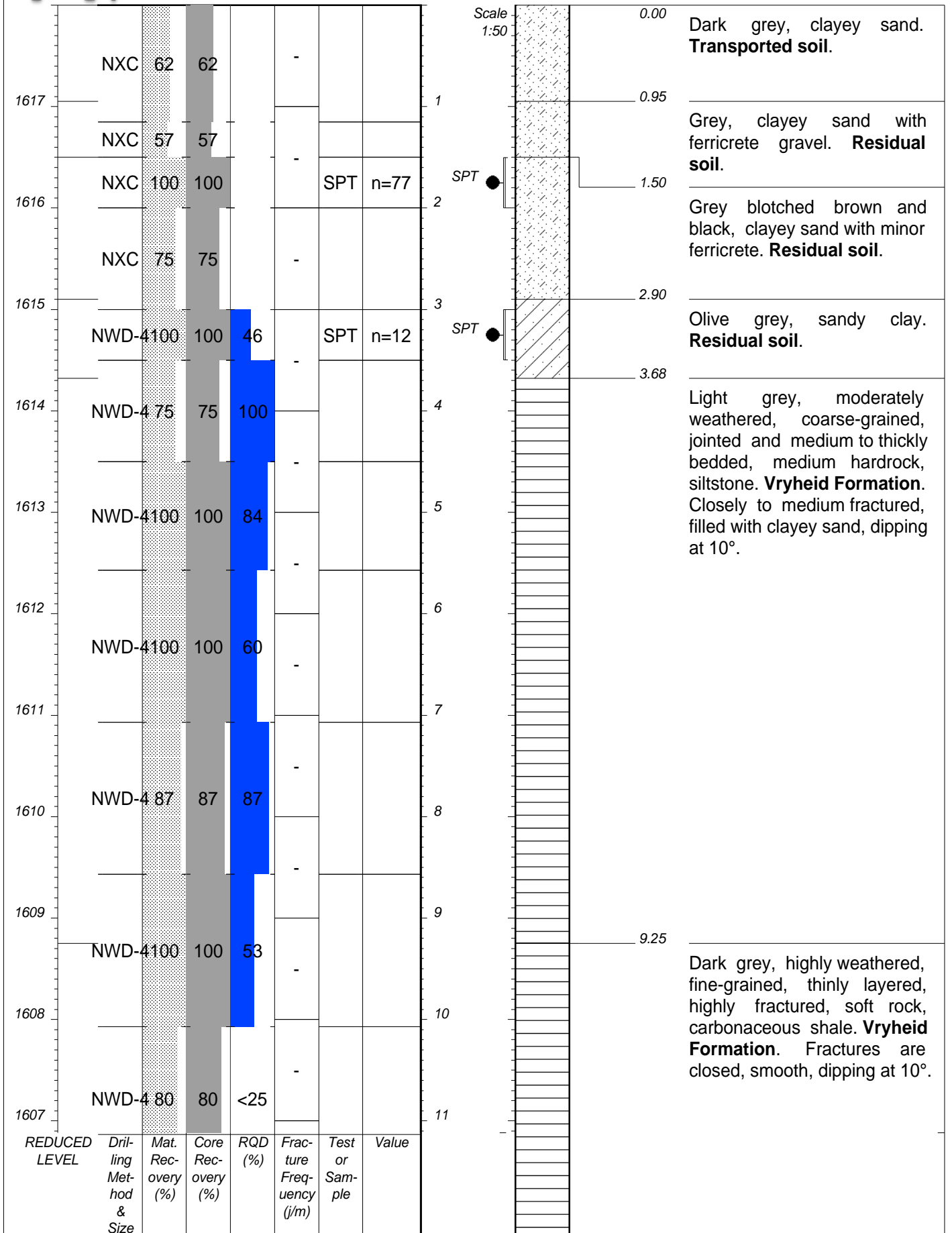
INCLINATION : Vertical
DIAM : 165 mm
DATE : 12/09/2023
DATE : 12/09/2023

DATE : 06/12/2023 14:12
TEXT : ..ssionBoreholes\KBH25.TXT

ELEVATION : 1613 m
X-COORD : 29.45604 °E
Y-COORD : 26.09251 °S

HOLE No: K-BH25

APPENDIX F: ROTARY CORE DRILLING LOGS



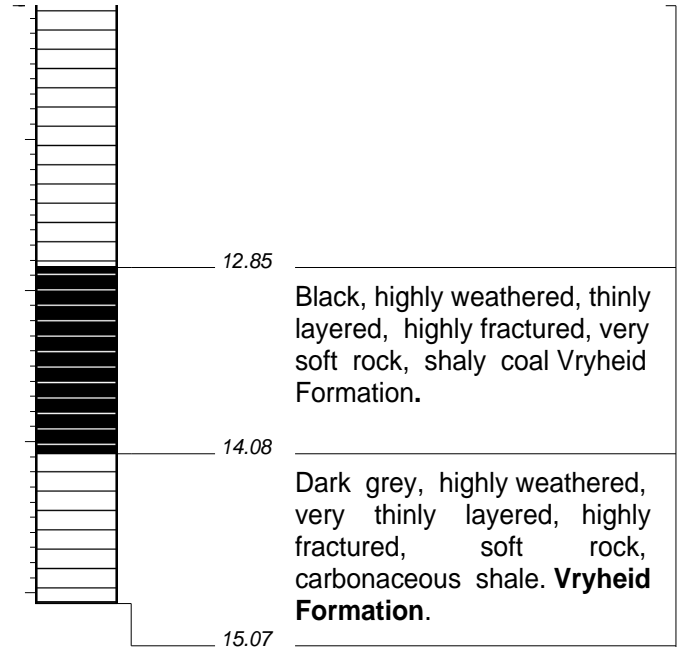


**ESKOM
PRELIMINARY GEOTECHNICAL INVESTIGATION:
KOMATI POWER STATION**

HOLE No: BH-01
Sheet 2 of 2

JOB NUMBER: MAK1520823

1606							
	NWD-4100	100	<25				
1605							
	NWD-4 83	83	<25				
1604							
	NWD-4 75	75	<25				
1603							
REDUCED LEVEL	Drilling Method & Size	Mat. Recovery (%)	Core Recovery (%)	RQD (%)	Fracture Frequency (j/m)	Test or Sample	Value



NOTES

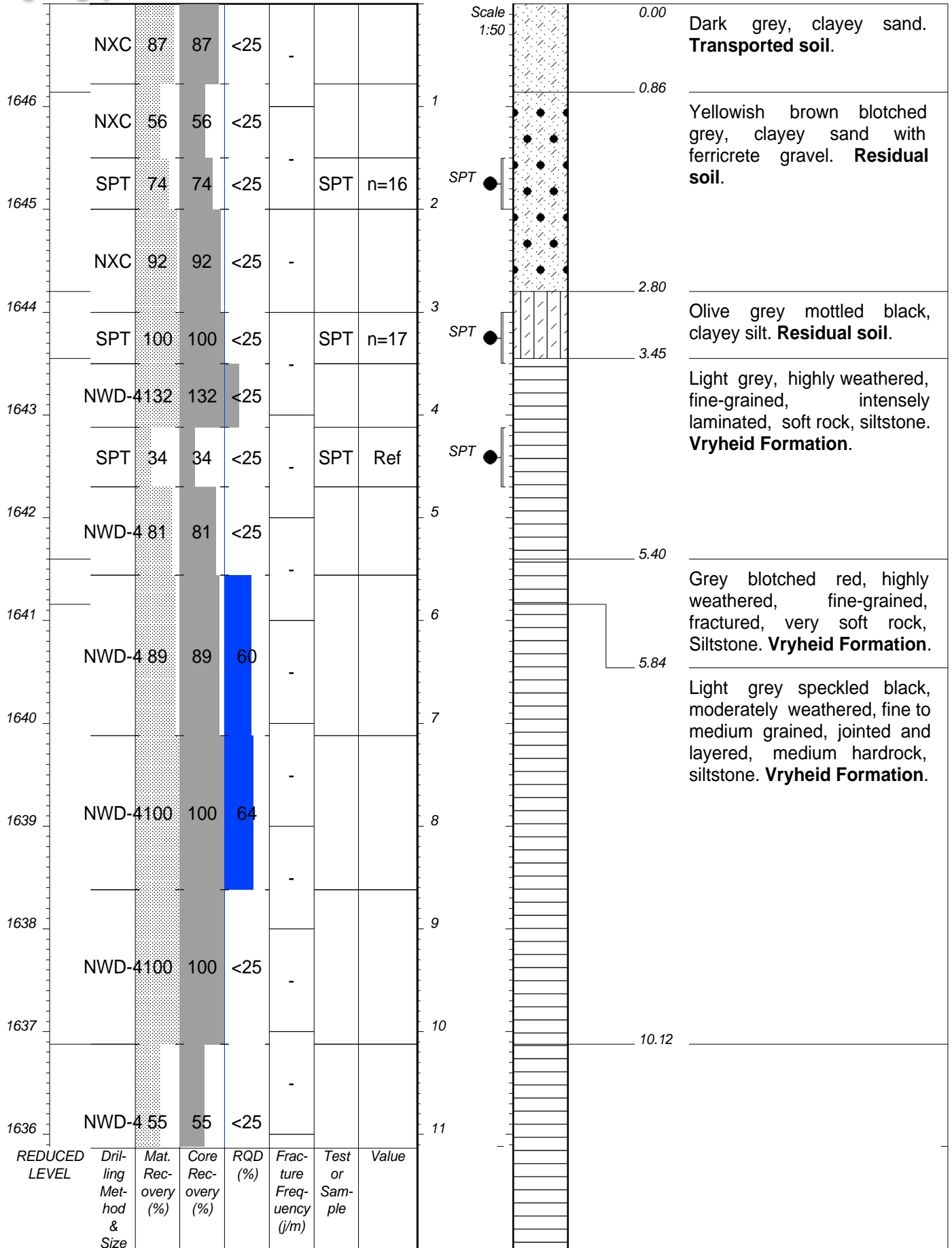
- 1) Sh = Shelby sample.
- 2) Ref = Refusal.
- 3) Disturbed SPT sample taken at 1.50--2.00 m.
- 4) Disturbed SPT sample taken at 3.00--3.50 m.
- 5) End of hole at 15.07 m.

CONTRACTOR : BAV Consulting Pty Ltd
MACHINE : ROCKDRILL TRD-300
DRILLED BY : Philip Mahlangu
PROFIED BY : Hudson Mabelane Pr.Sci.Nat
TYPE SET BY : Fumani Vukeya Cand.Sci.Nat
SETUP FILE : STANDARD.SET

INCLINATION : Vertical
DIAM : NWD-4 & NXC
DATE : 18/09/2023
DATE : 17/11/2023
DATE : 06/12/2023 14:22
TEXT : ..eholes\Boreholes\BH1.TXT

ELEVATION : 1618m
X-COORD : 26.09088°S
Y-COORD : 29.46033°E

HOLE No: BH-01
Komati Power Station



1635							
	NWD-4	43	43	<25	-		
1634							
	NWD-4	93	93	59	-		
1633							
	NWD-4	100	100	45	-		
1632							
	NWD-4	101	101	29	-		
1631							
	NWD-4	93	93	91	-		
1630							
1629							
1628							
REDUCED LEVEL	Drilling Method & Size	Mat. Recovery (%)	Core Recovery (%)	RQD (%)	Fracture Frequency (j/m)	Test or Sample	Value



Light grey blotched yellowish brown, highly weathered, fine-grained, intensely laminated and layered, soft rock, siltstone. **Vryheid Formation.**

Black, silty clay/clayey silt. **Peat.**

Light grey speckled black, moderately weathered, fine to medium grained, layered, medium hardrock, siltstone. **Vryheid Formation.**

Dark grey to black, moderately weathered, fine to medium grained, intensely laminated and layered, very soft to soft rock, carbonaceous shale interlayered with coal. **Vryheid Formation.**

Grey, moderately to slightly weathered, fine-grained, layered, medium hardrock, siltstone. **Vryheid Formation.**

NOTES

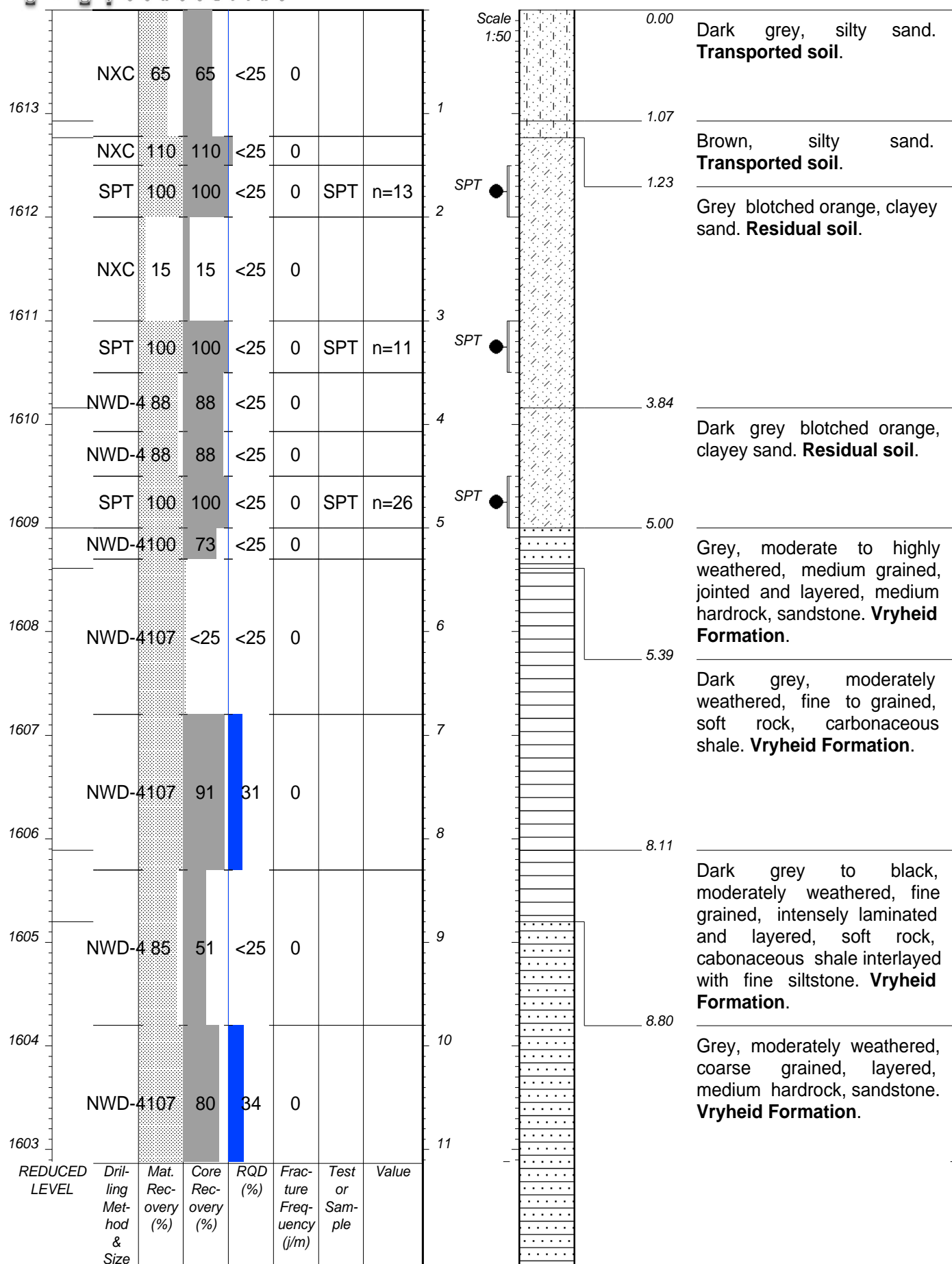
- 1) Sh = Shelby sample.
- 2) Ref = Refusal.
- 3) Disturbed SPT sample taken at 1.50--2.00 m.
- 4) Disturbed SPT sample taken at 3.00--3.50 m.
- 5) Disturbed SPT sample refused at 4.12--4.70 m.
- 6) End of hole at 19.12 m.

CONTRACTOR : BAV Consulting Pty Ltd
MACHINE : ROCKDRILL TRD-300
DRILLED BY : Philip Mahlangu
PROFIED BY : Hudson Mabelane Pr.Sci.Nat
TYPE SET BY : Fumani Vukeya Cand.Sci.Nat
SETUP FILE : STANDARD.SET

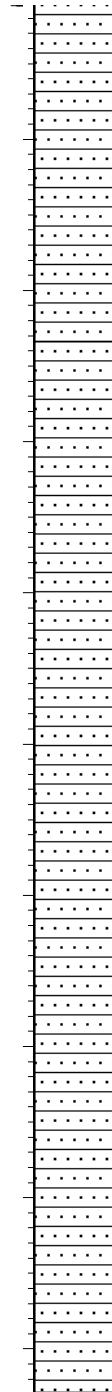
INCLINATION : Vertical
DIAM : NWD-4 & NXC
DATE : 04/10/2023
DATE : 17/11/2023
DATE : 06/12/2023 14:23
TEXT : ..eholes\Boreholes\BH2.TXT

ELEVATION : 1647m
X-COORD : 26.09243°S
Y-COORD : 29.45800°E

HOLE No: BH-02
Komati Power Station



1602	NWD-4100	80	28	0		
1601						
1600	NWD-4100	100	96	0		
1599	NWD-4 99	99	88	0		
1598						
1597	NWD-4100	100	85	0		
1596	NWD-4100	100	85	0		
1595						
1594	NWD-4100	100	82	0		
REDUCED LEVEL	Drilling Method & Size	Mat. Recovery (%)	Core Recovery (%)	RQD (%)	Fracture Frequency (j/m)	Test or Sample
						Value



13.34

Dark grey, fresh, coarse grained, layered, medium hardrock, sandstone. **Vryheid Formation.**

20.30

NOTES

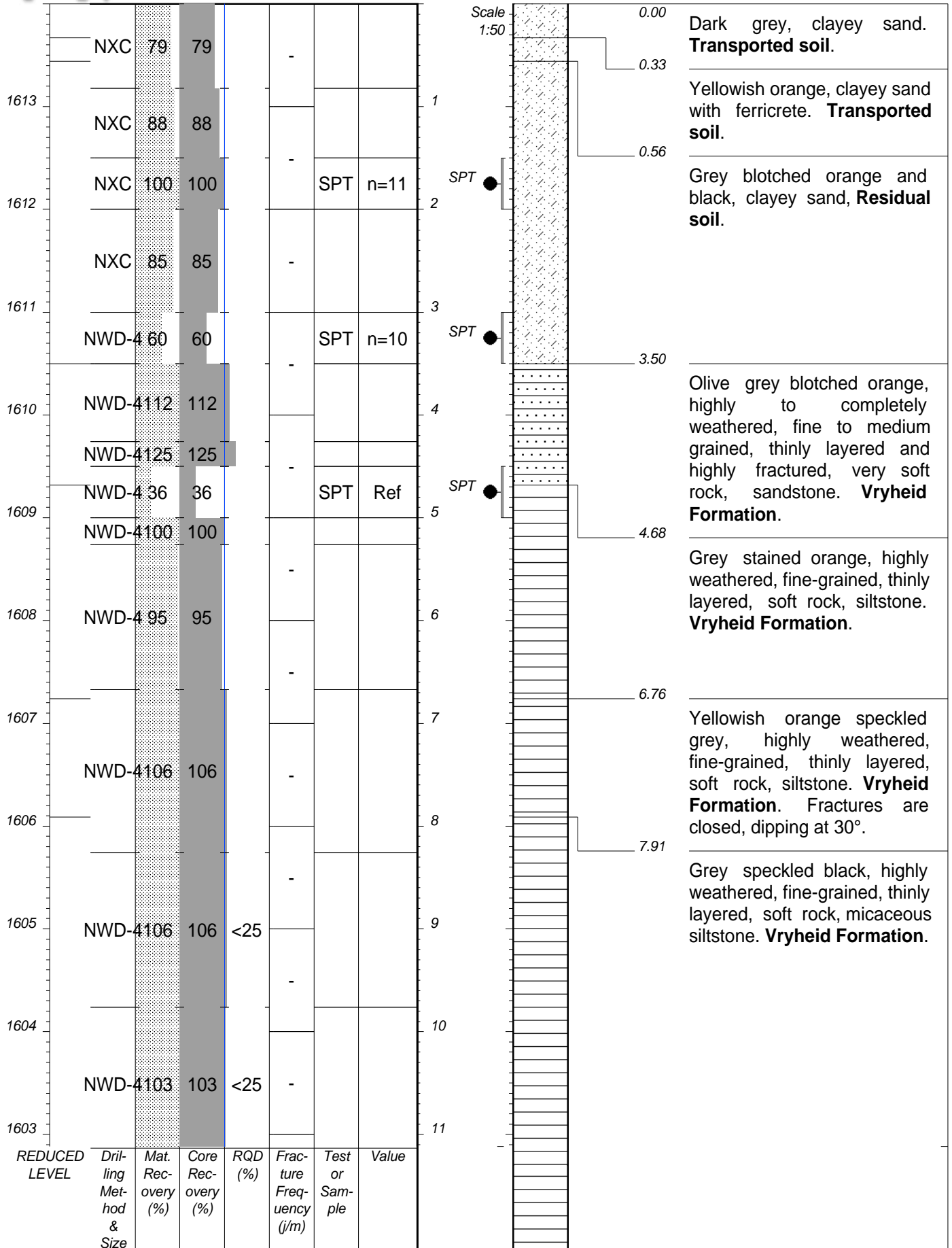
- 1) Sh = Shelby sample.
- 2) Ref = Refusal.
- 3) Disturbed SPT sample taken at 1.50--2.00 m.
- 4) Disturbed SPT sample taken at 3.00--3.50 m.
- 5) Disturbed SPT sample taken at 4.50--5.00 m.
- 6) End of hole at 20.30 m.

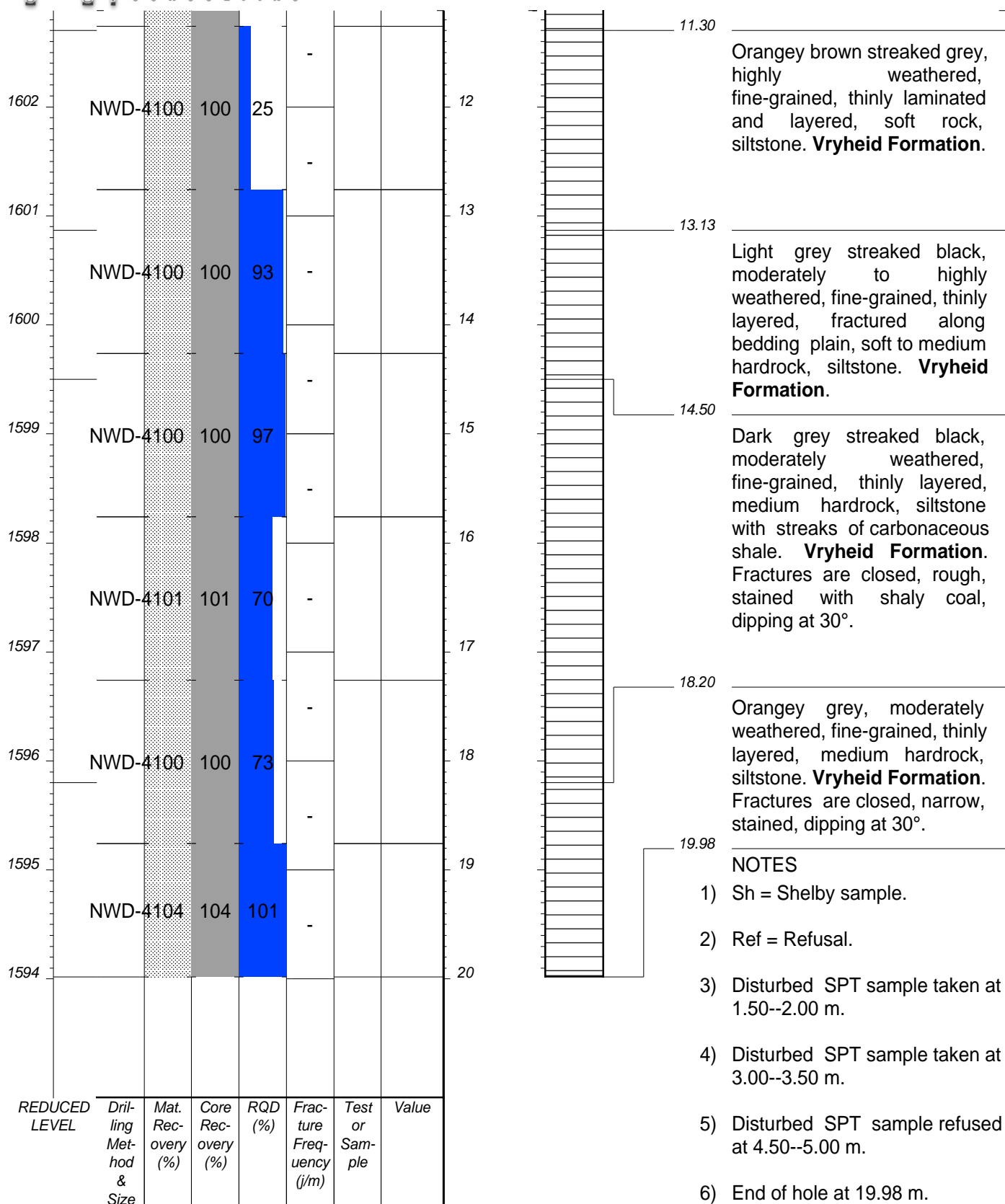
CONTRACTOR : BAV Consulting Pty Ltd
MACHINE : ROCKDRILL TRD-300
DRILLED BY : Philip Mahlangu
PROFILED BY : Andries Vukeya Cand.Sci.Nat
TYPE SET BY : Andries Vukeya Cand.Sci.Nat
SETUP FILE : STANDARD.SET

INCLINATION : Vertical
DIAM : NWD-4 & NXC
DATE : 02/10/2023
DATE : 17/11/2023
DATE : 06/12/2023 14:24
TEXT : ..eholes\Boreholes\BH3.TXT

ELEVATION : 1614m
X-COORD : 26.09292°S
Y-COORD : 29.45524°E

HOLE No: BH-03
Komati Power Station





CONTRACTOR : BAV Consulting Pty Ltd
MACHINE : ROCKDRILL TRD-300
DRILLED BY : Philip Mahlangu
PROFILED BY : Hudson Mabelane Pr.Sci.Nat

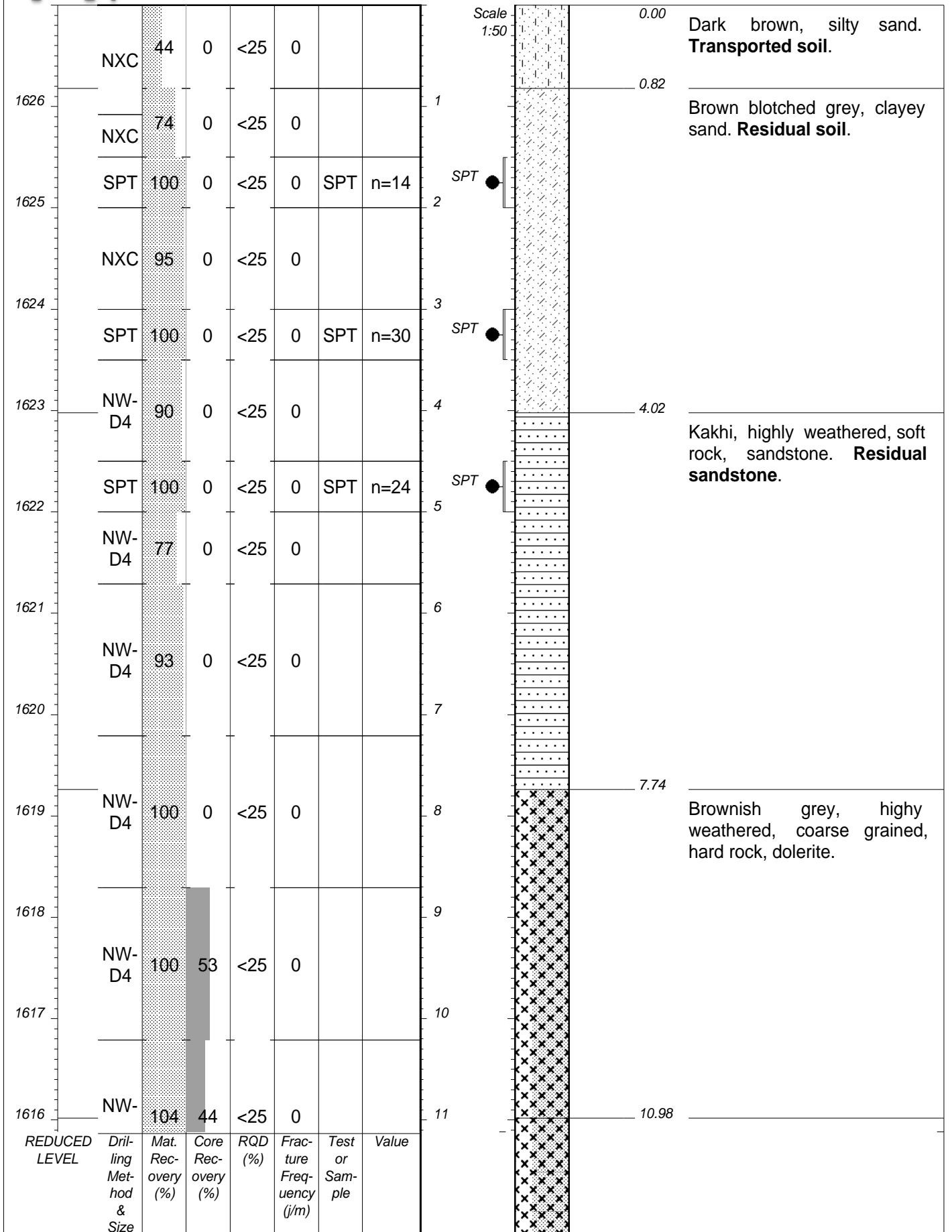
TYPE SET BY : Fumani Vukeya Cand.Sci.Nat
SETUP FILE : STANDARD.SET

INCLINATION : Vertical
DIAM : NWD-4 & NXC
DATE : 04/10/2023
DATE : 17/11/2023

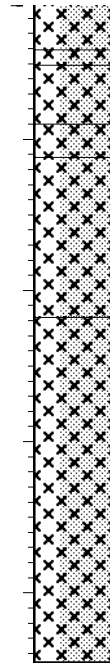
DATE : 06/12/2023 14:24
TEXT : ..eholes\Boreholes\BH4.TXT

ELEVATION : 1614m
X-COORD : 26.09311°S
Y-COORD : 29.45172°E

HOLE No: BH-04
Komati Power Station



1615	NW-D4	100	62	60	0		
1614	NW-D4	100	100	<25	0		
1613	NW-D4	73	73	<25	0		
1612	NW-D4						
REDUCED LEVEL	Drilling Method & Size	Mat. Recovery (%)	Core Recovery (%)	RQD (%)	Fracture Frequency (j/m)	Test or Sample	Value



Grey, moderately weathered, coarse grained, hard rock, dolerite.

Brownish grey, highly weathered, coarse grained, hard rock, dolerite.

Grey, moderately weathered, coarse grained, hard rock, dolerite.

Brownish grey, highly weathered, coarse grained, hard rock, dolerite.

Grey, moderately weathered, coarse grained, hard rock, dolerite.

Brownish grey, highly weathered, coarse grained, hard rock, dolerite.

NOTES

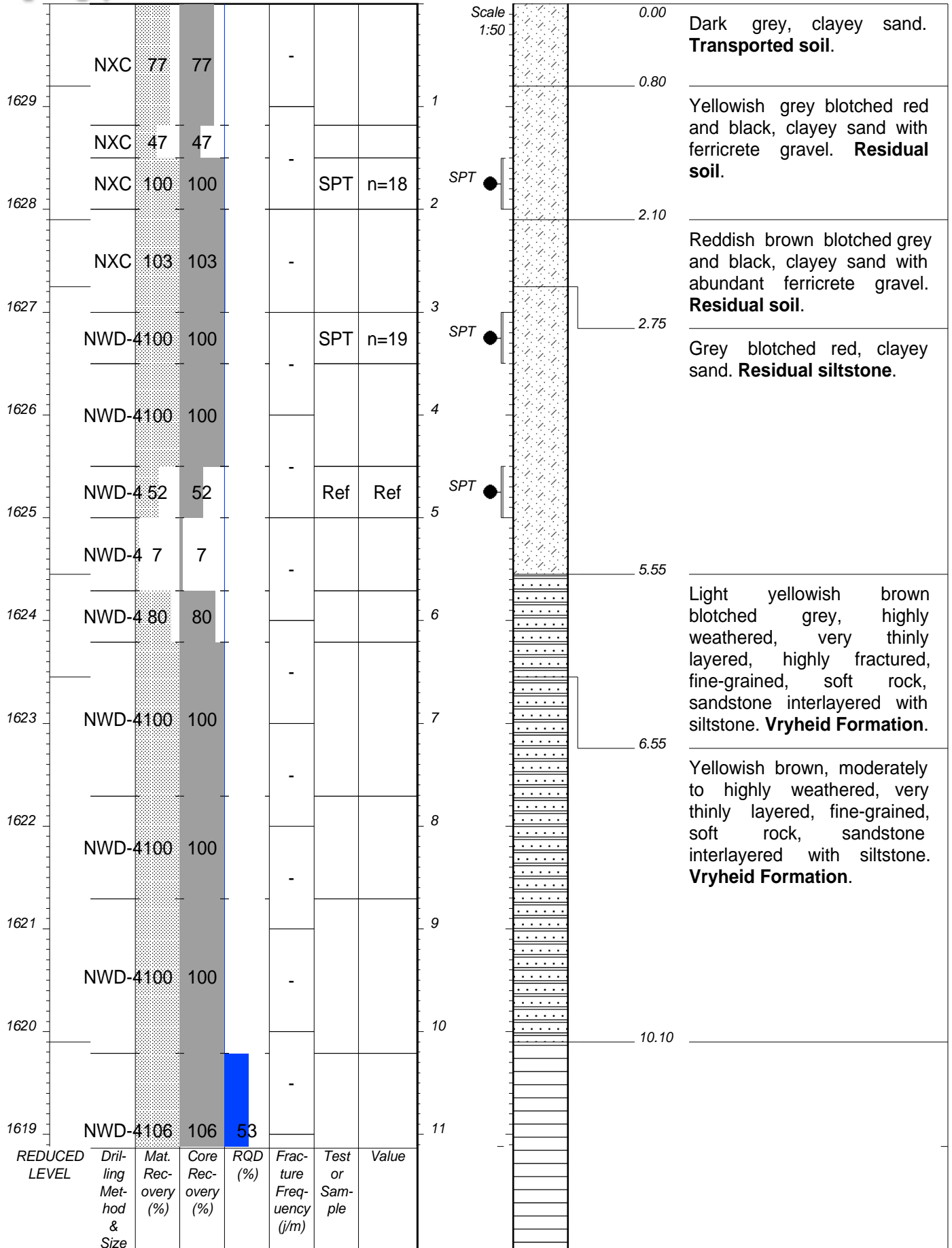
- 1) No water strike encountered.
- 2) Sh = Shelby sample.
- 3) Ref = Refusal.
- 4) Disturbed SPT sample taken at 1.50--2.00 m.
- 5) Disturbed SPT sample taken at 3.00--3.50 m.
- 6) Disturbed SPT sample taken at 4.50--5.00 m.
- 7) End of hole at 15.26 m.

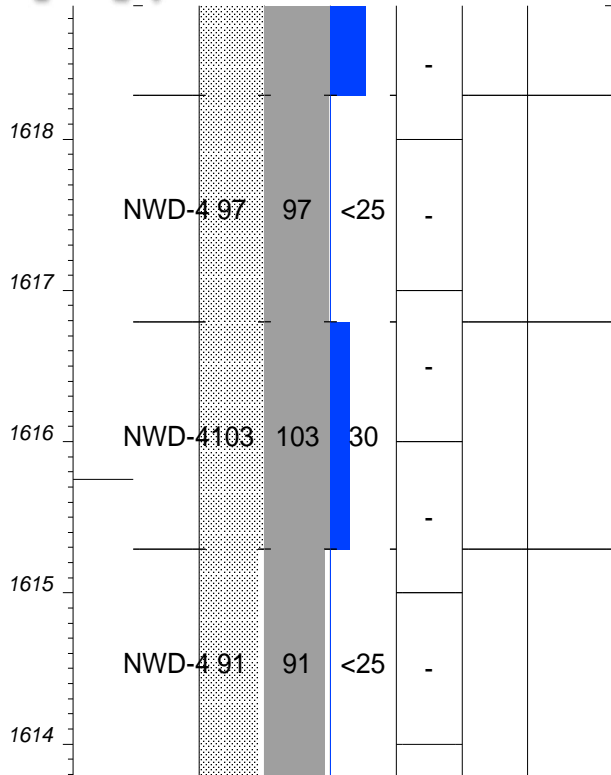
CONTRACTOR : BAV Consulting Pty Ltd
MACHINE : ROCKDRILL TRD-300
DRILLED BY : Philip Mahlangu
PROFILED BY : Andries Vukeya Cand.Sci.Nat
TYPE SET BY : Andries Vukeya Cand.Sci.Nat
SETUP FILE : STANDARD.SET

INCLINATION : Vertical
DIAM : NWD-4 & NXC
DATE : 09/10/2023
DATE : 02/12/2023
DATE : 06/12/2023 14:25
TEXT : ..eholes\Boreholes\BH5.TXT

ELEVATION : 1627m
X-COORD : 26.09509°S
Y-COORD : 29.45090°E

HOLE No: BH-05
Komati Power Station





Dark grey, highly weathered, thinly layered, fine-grained, soft rock, siltstone. **Vryheid Formation.**

Dark grey, slightly weathered, thinly layered, fine to medium grained, hard rock, dolerite. **Vryheid Formation.**

NOTES

- 1) Sh = Shelby sample.
- 2) Ref = Refusal.
- 3) Disturbed SPT sample taken at 1.50--2.00 m.
- 4) Disturbed SPT sample taken at 3.00--3.50 m.
- 5) Disturbed SPT sample refused at 4.50--5.00 m.
- 6) End of hole at 16.21 m.

REDUCED LEVEL	Drilling Method & Size	Mat. Recovery (%)	Core Recovery (%)	RQD (%)	Fracture Frequency (j/m)	Test or Sample	Value
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CONTRACTOR : BAV Consulting Pty Ltd
MACHINE : ROCKDRILL TRD-300
DRILLED BY : Philip Mahlangu
PROFILED BY : Hudson Mabelane Pr.Sci.Nat

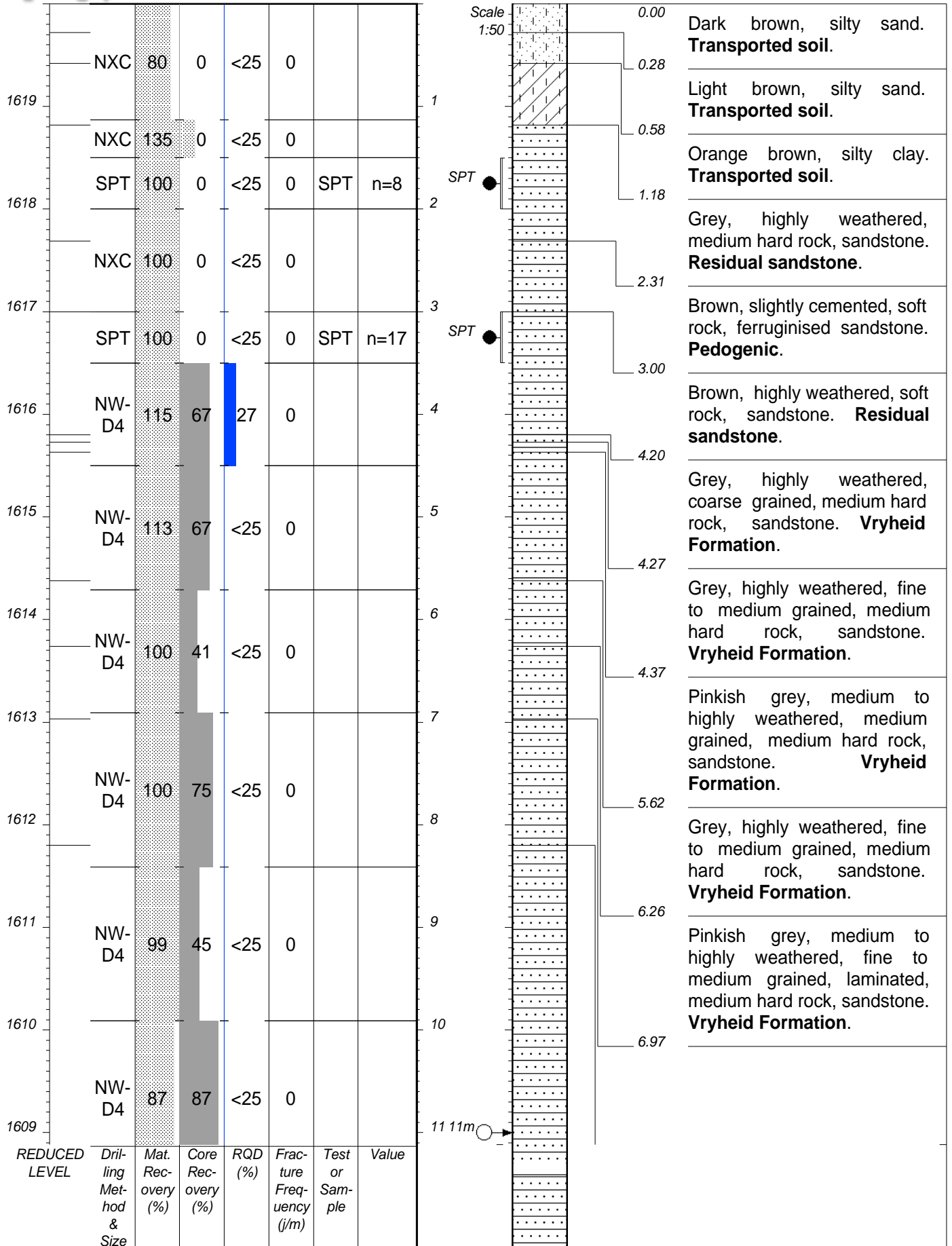
TYPE SET BY : Fumani Vukeya Cand.Sci.Nat
SETUP FILE : STANDARD.SET

INCLINATION : Vertical
DIAM : NWD-4 & NXC
DATE : 12/10/2023
DATE : 18/11/2023

DATE : 06/12/2023 14:25
TEXT : ..eholes\Boreholes\BH6.TXT

ELEVATION : 1630m
X-COORD : 26.09667°S
Y-COORD : 29.45204°E

HOLE No: BH-06
Komati Power Station



1608	NW-D4	91	91	91	0			12
1607	NW-D4	90	90	90	0			13
1606	NW-D4	107	107	100	0			14
1605	NW-D4	86	86	86	0			15

8.20	Grey, medium to highly weathered, medium grained, laminated, medium hard rock, sandstone. Vryheid Formation.
11.30	Grey, medium to highly weathered, medium grained, medium hard rock, sandstone. Vryheid Formation.
11.42	Dark grey, slightly weathered, fine grained, soft rock, shale. Vryheid Formation.
11.85	Pinkish grey, moderately weathered, medium grained, laminated, medium hard rock, sandstone. Vryheid Formation.
12.10	Grey, highly weathered, fine grained, laminated, soft to medium hard rock, sandstone. Vryheid Formation.
12.57	Grey, slightly weathered, very coarse grained, medium hard rock, sandstone. Vryheid Formation.
14.71	Grey, slightly weathered, medium grained, laminated, medium hard rock, sandstone. Vryheid Formation.
15.55	Grey, slightly weathered, medium grained, laminated, medium hard rock, sandstone. Vryheid Formation.

NOTES

- 1) Possible water strike encountered at 11m.
- 2) Sh = Shelby sample.
- 3) Ref = Refusal.
- 4) Disturbed SPT sample taken at 1.50--2.00 m.

REDUCED LEVEL	Drilling Method & Size	Mat. Recovery (%)	Core Recovery (%)	RQD (%)	Fracture Frequency (j/m)	Test or Sample	Value
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ESKOM
PRELIMINARY GEOTECHNICAL INVESTIGATION:
KOMATI POWER STATION

HOLE No: BH-07
Sheet 3 of 3

JOB NUMBER: MAK1520823

- 5) Disturbed SPT sample taken at 3.00--3.50 m.
- 6) End of hole at 15.55 m.

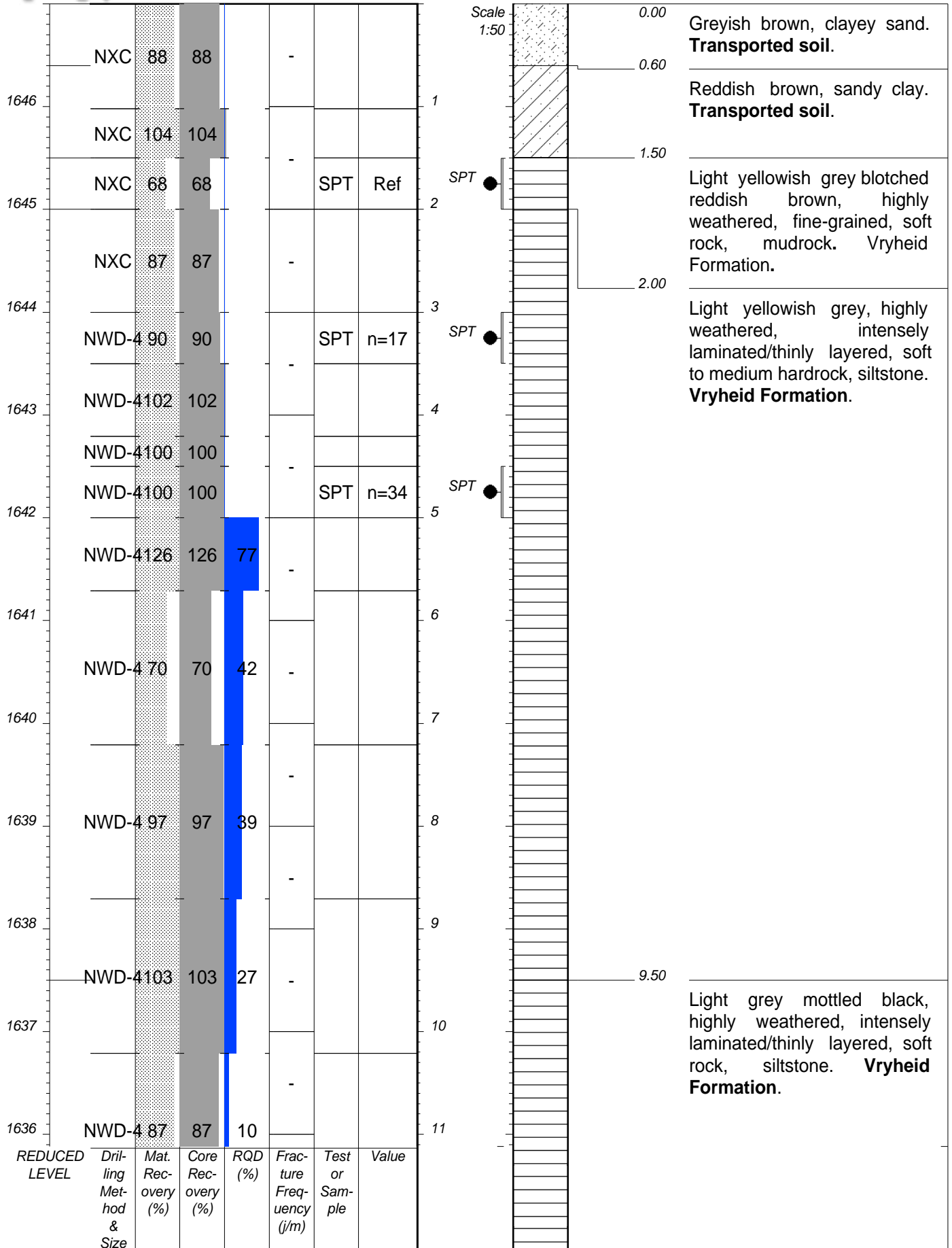
REDUCED LEVEL	Dril- ling Met- hod & Size	Mat. Rec- overy (%)	Core Rec- overy (%)	RQD (%)	Frac- ture Freq- uency (j/m)	Test or Sam- ple	Value
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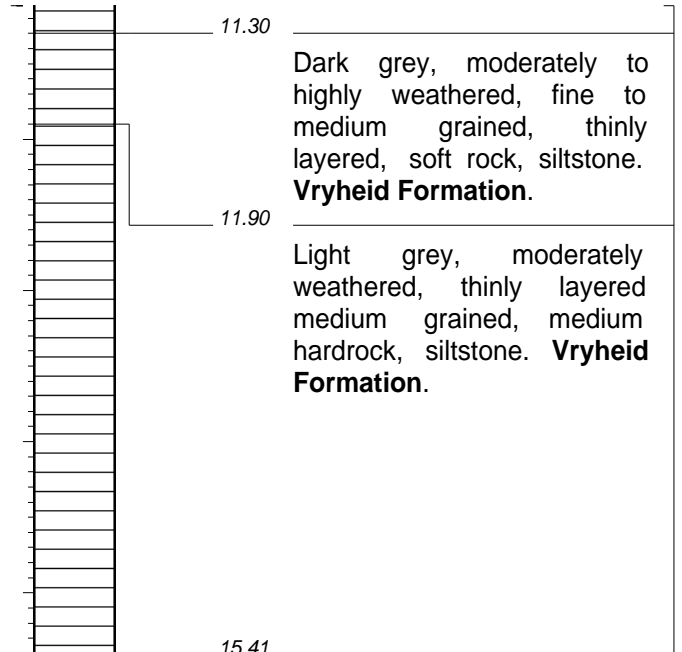
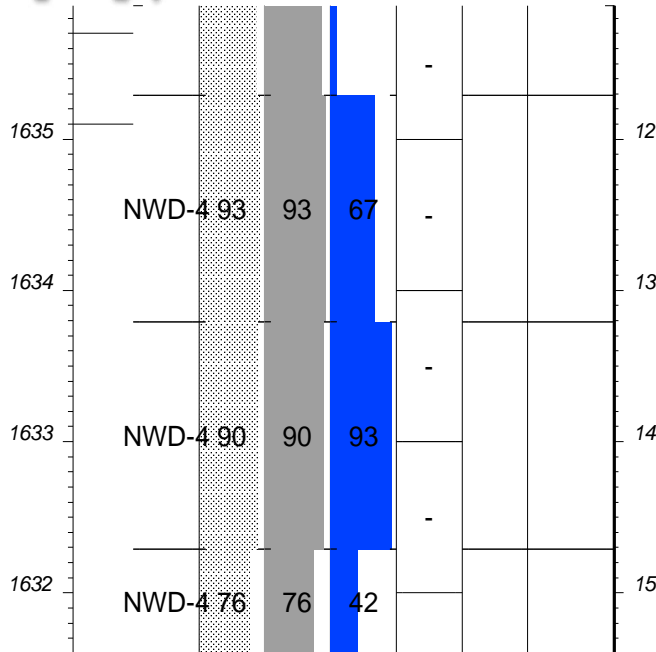
CONTRACTOR : BAV Consulting Pty Ltd
MACHINE : ROCKDRILL TRD-300
DRILLED BY : Philip Mahlangu
PROFILED BY : Andries Vukeya Cand.Sci.Nat
TYPE SET BY : Andries Vukeya Cand.Sci.Nat
SETUP FILE : STANDARD.SET

INCLINATION : Vertical
DIAM : NWD-4 & NXC
DATE : 14/10/2023
DATE : 30/11/2023
DATE : 06/12/2023 14:25
TEXT : ..eholes\Boreholes\BH7.TXT

ELEVATION : 1620m
X-COORD : 26.09992°S
Y-COORD : 29.45487°E

HOLE No: BH-07
Komati Power Station





Dark grey, moderately to highly weathered, fine to medium grained, thinly layered, soft rock, siltstone. **Vryheid Formation.**

Light grey, moderately weathered, thinly layered medium grained, medium hardrock, siltstone. **Vryheid Formation.**

NOTES

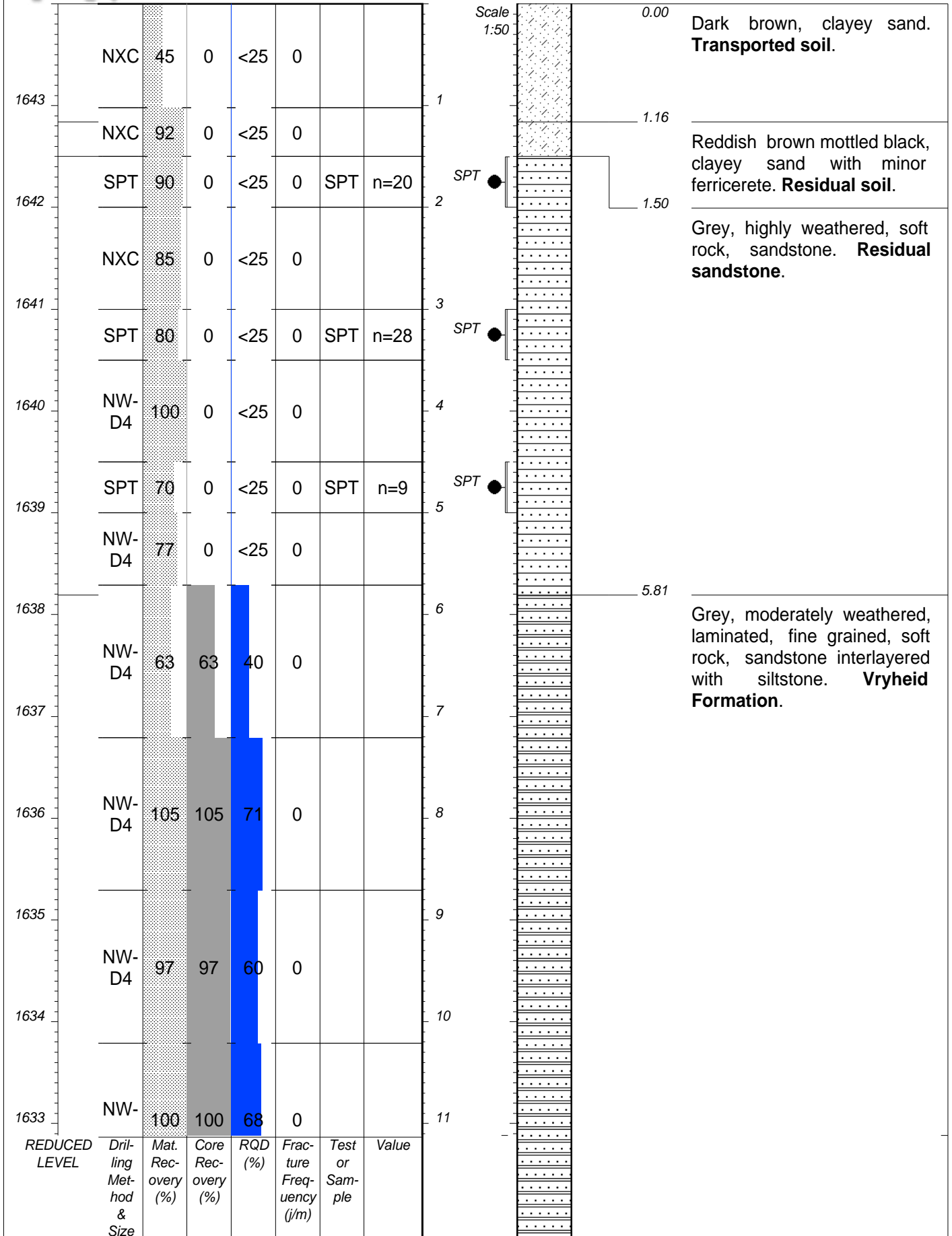
- 1) Sh = Shelby sample.
- 2) Ref = Refusal.
- 3) Disturbed SPT sample refused at 1.50--2.00 m.
- 4) Disturbed SPT sample taken at 3.00--3.50 m.
- 5) Disturbed SPT sample taken at 4.50--5.00 m.
- 6) End of hole at 15.41 m.

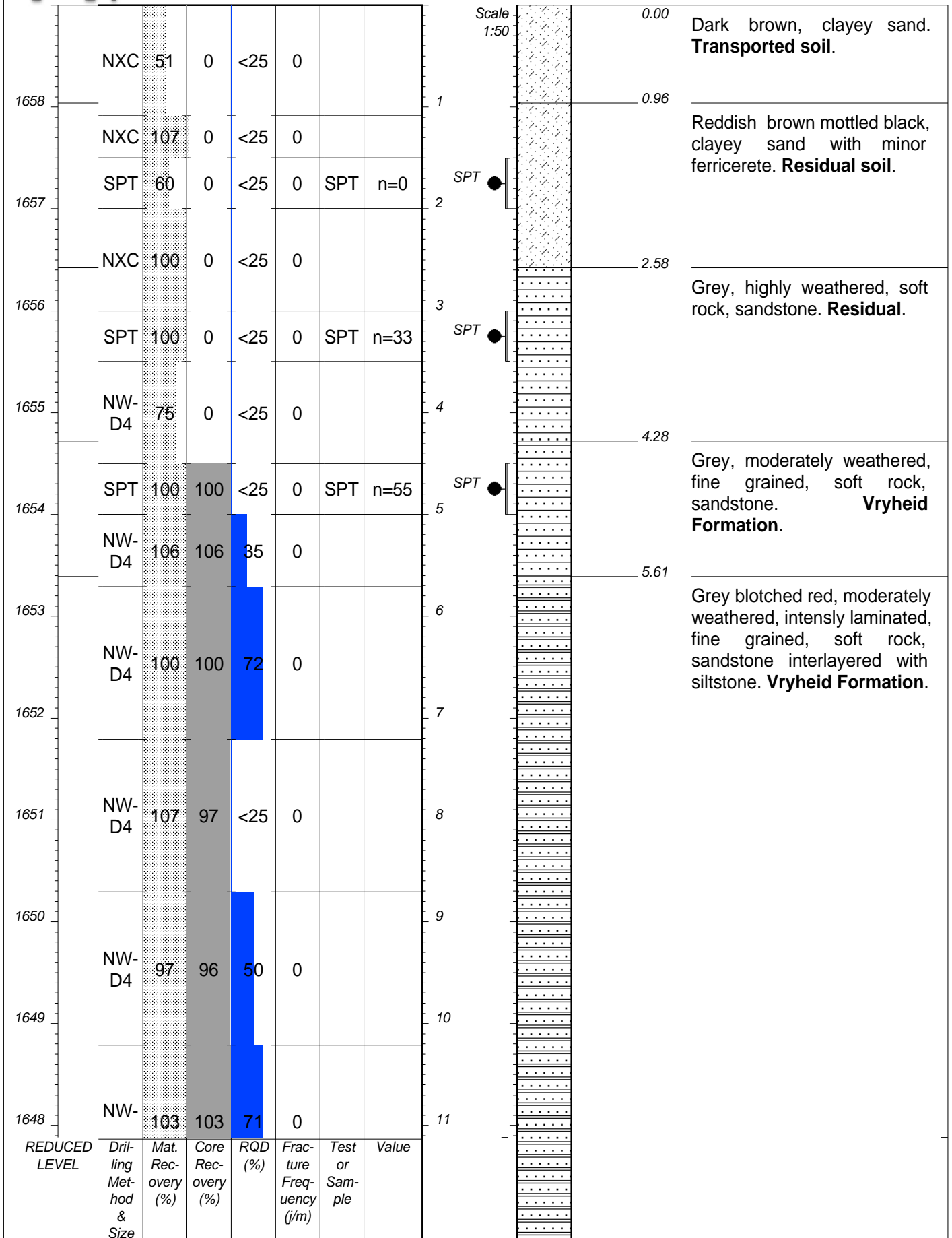
CONTRACTOR : BAV Consulting Pty Ltd
MACHINE : ROCKDRILL TRD-300
DRILLED BY : Philip Mahlangu
PROFIED BY : Hudson Mabelane Pr.Sci.Nat
TYPE SET BY : Fumani Vukeya Cand.Sci.Nat
SETUP FILE : STANDARD.SET

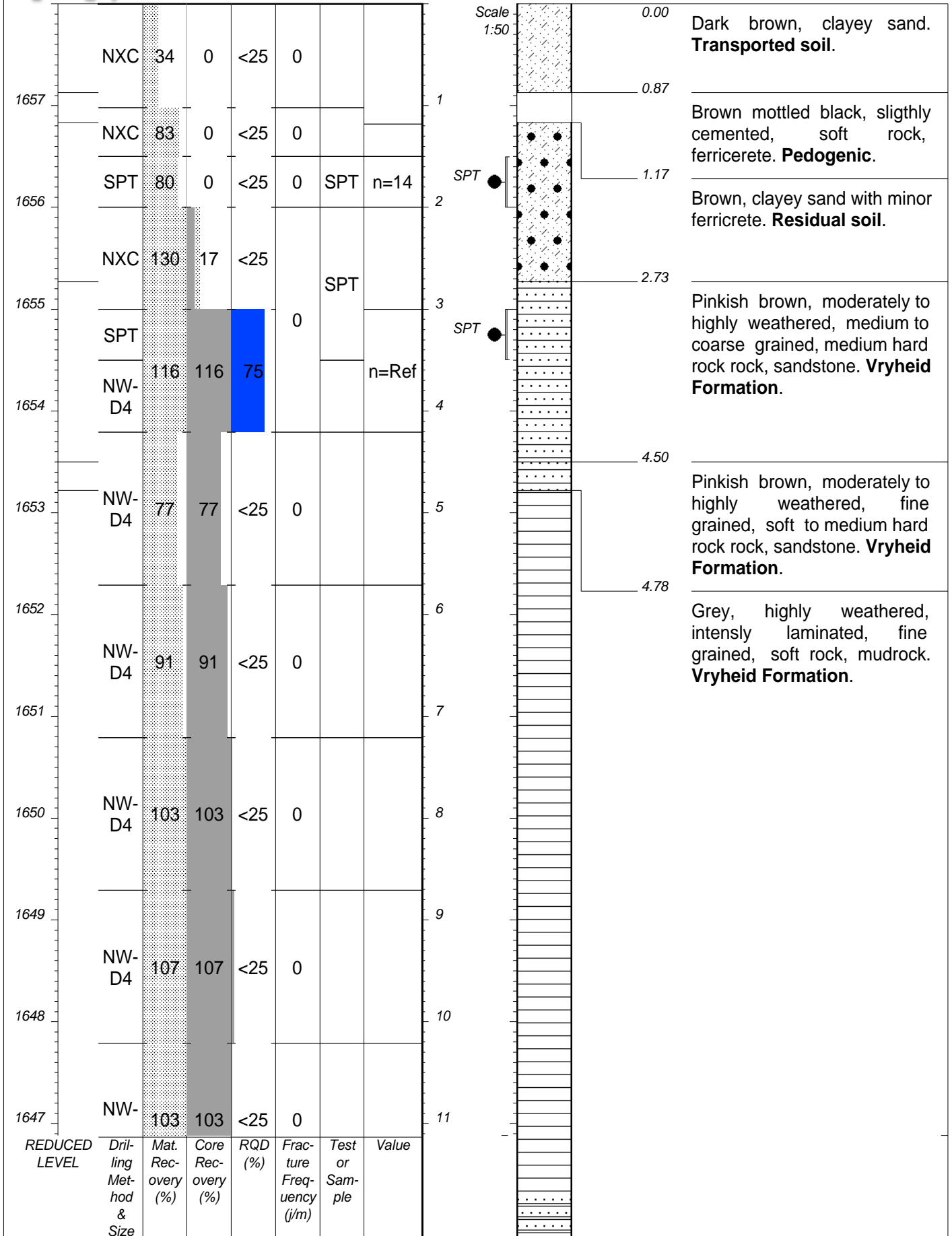
INCLINATION : Vertical
DIAM : NWD-4 & NXC
DATE : 17/10/2023
DATE : 17/11/2023
DATE : 06/12/2023 14:26
TEXT : ..eholes\Boreholes\BH8.TXT

ELEVATION : 1647m
X-COORD : 26.10565°S
Y-COORD : 29.45343°E

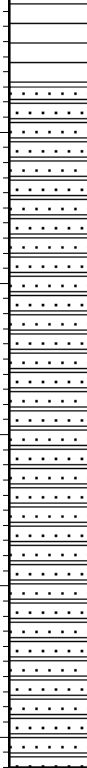
HOLE No: BH-08
Komati Power Station







REDUCED LEVEL	Drilling Method & Size	Mat. Recovery (%)	Core Recovery (%)	RQD (%)	Fracture Frequency (j/m)	Test or Sample	Value
1646	NW-D4	100	100	81	0		
1645							
1644	NW-D4	100	100	<25	0		
1643							
1642	NW-D4	103	103	<25	0		



11.70

Grey, highly weathered, intensely laminated, fine grained, soft rock, sandstone interlayered with siltstone. **Vryheid Formation.**

16.21

NOTES

- 1) No water strike encountered.
- 2) Sh = Shelby sample.
- 3) Ref = Refusal.
- 4) Disturbed SPT sample taken at 1.50--2.00 m.
- 5) Disturbed SPT sample taken at 3.00--3.50 m.
- 6) End of hole at 16.21 m.

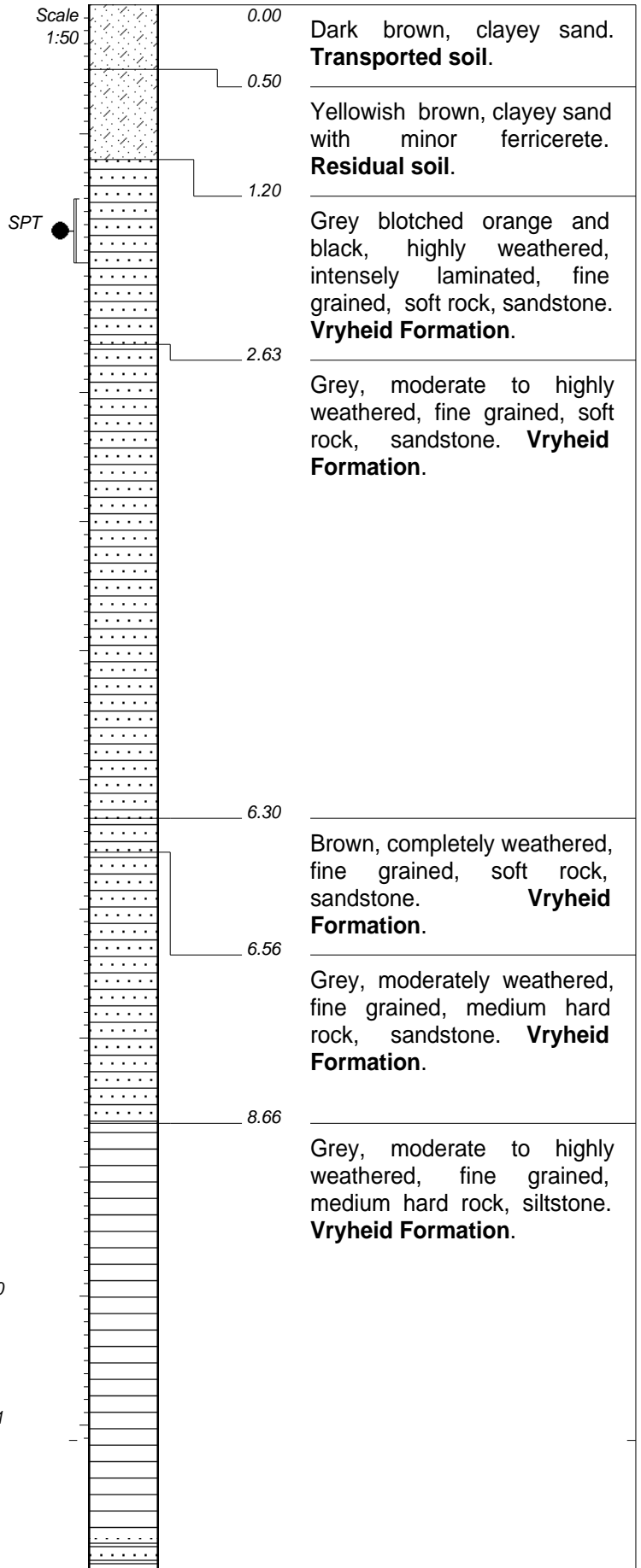
CONTRACTOR : BAV Consulting Pty Ltd
MACHINE : ROCKDRILL TRD-300
DRILLED BY : Philip Mahlangu
PROFILED BY : Andries Vukeya Cand.Sci.Nat
TYPE SET BY : Andries Vukeya Cand.Sci.Nat
SETUP FILE : STANDARD.SET

INCLINATION : Vertical
DIAM : NWD-4 & NXC
DATE : 27/10/2023
DATE : 02/12/2023
DATE : 06/12/2023 14:27
TEXT : .holes\Boreholes\BH11.TXT

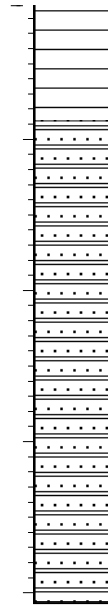
ELEVATION : 1658m
X-COORD : 26.10828°S
Y-COORD : 29.45994°E

HOLE No: BH-11
Komati Power Station

REDUCED LEVEL	Drilling Method & Size	Mat. Recovery (%)	Core Recovery (%)	RQD (%)	Fracture Frequency (j/m)	Test or Sample	Value
1659	NXC	63	0	<25	0		
1658	NXC	107	0	<25	0		
1658	SPT	100	0	<25	0	SPT	n=44
1657	NXC	75	63	<25	0		
1656	NW-D4	113	87	37	0		n=Ref
1655	NW-D4	90	87	43	0		
1654	NW-D4	99	80	58	0		
1653	NW-D4	100	100	73	0		
1651	NW-D4	100	100	38	0		
1649	NW-	103	103	47	0		



	D4						
1648							12
	NW-D4	90	90	<25	0		
1647							13
	NW-D4	105	89	<25	0		
1646							14
	NW-D4	103	64	<25	0		
1645							15



11.88

Grey, highly to completely weathered, fine grained, medium hard rock, sandstone interlayered with siltstone.
Vryheid Formation.

15.07

NOTES

- 1) No water strike encountered.
- 2) Sh = Shelby sample.
- 3) Ref = Refusal.
- 4) Disturbed SPT sample taken at 1.50--2.00 m.
- 5) End of hole at 15.07 m.

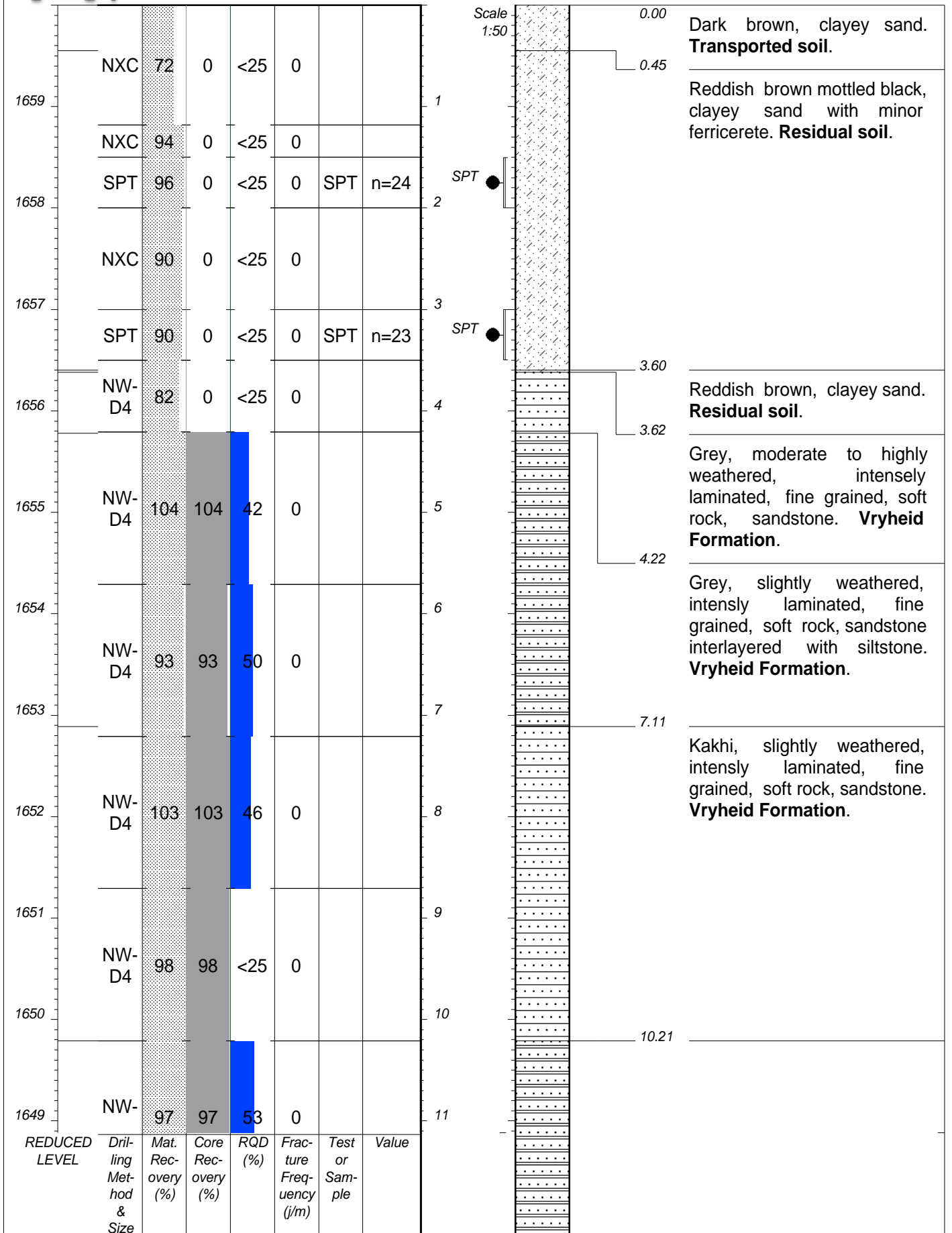
REDUCED LEVEL	Drilling Method & Size	Mat. Recovery (%)	Core Recovery (%)	RQD (%)	Fracture Frequency (j/m)	Test or Sample	Value
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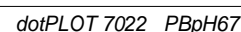
CONTRACTOR : BAV Consulting Pty Ltd
MACHINE : ROCKDRILL TRD-300
DRILLED BY : Philip Mahlangu
PROFILED BY : Andries Vukeya Cand.Sci.Nat
TYPE SET BY : Andries Vukeya Cand.Sci.Nat
SETUP FILE : STANDARD.SET

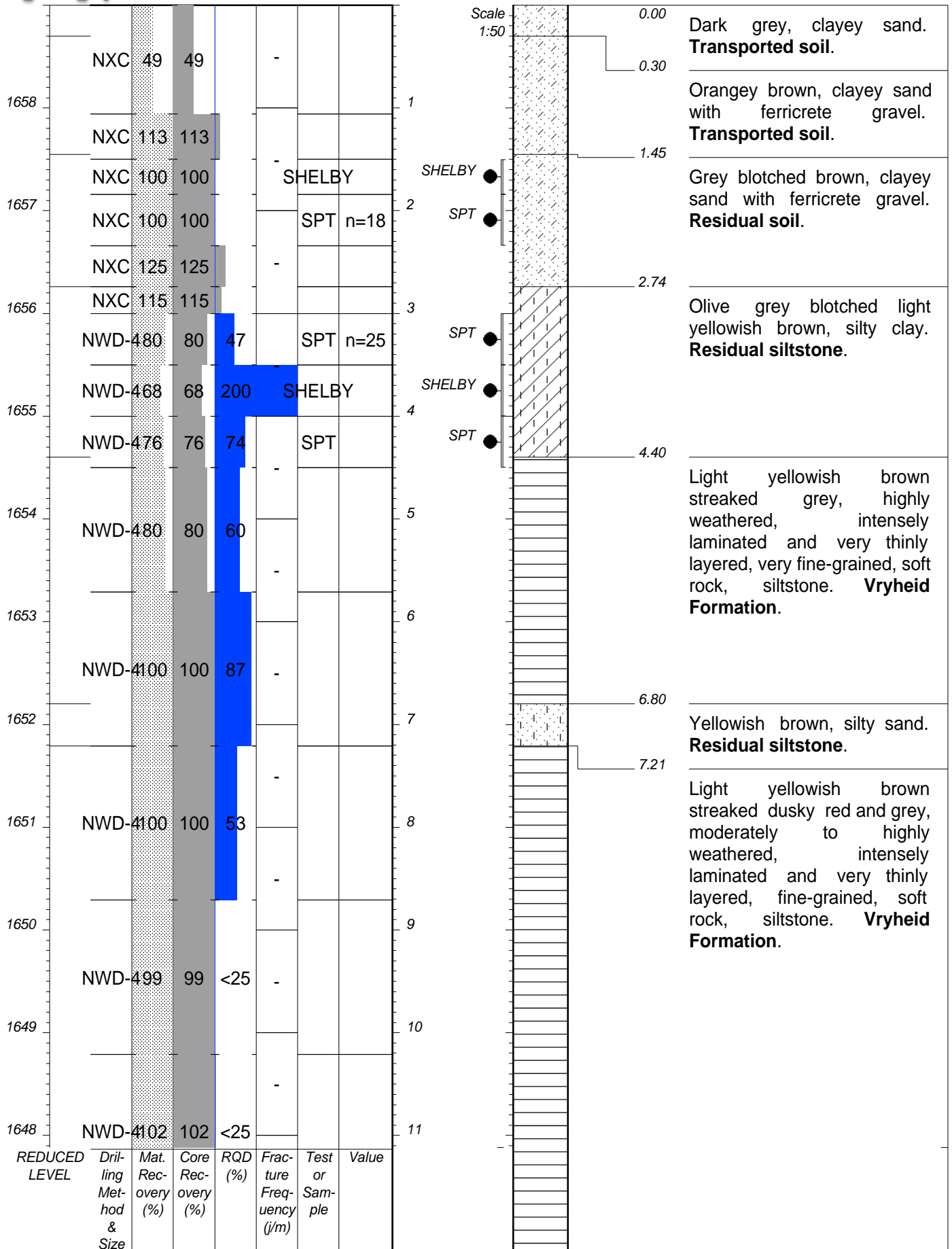
INCLINATION : Vertical
DIAM : NWD-4 & NXC
DATE : 28/12/2023
DATE : 03/12/2023
DATE : 06/12/2023 14:28
TEXT : ..holes\Boreholes\BH12.TXT

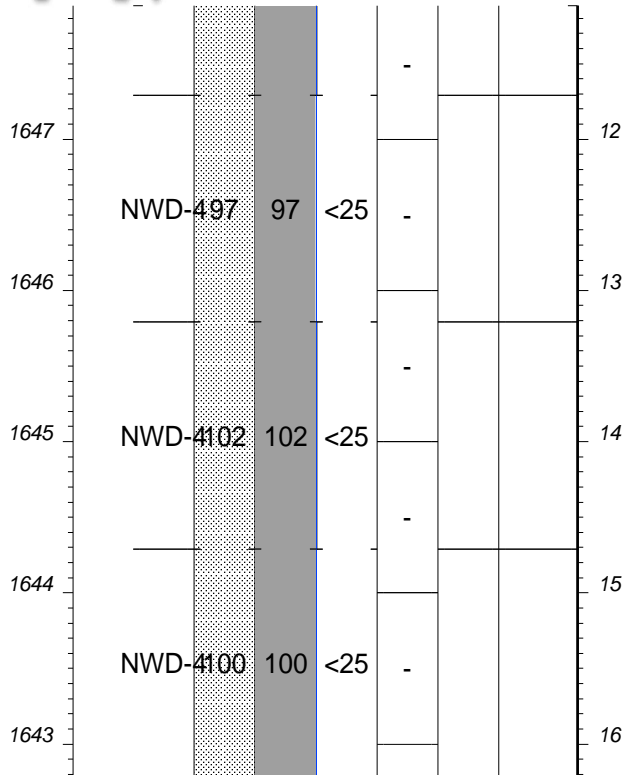
ELEVATION : 1660m
X-COORD : 26.11020°S
Y-COORD : 29.46267°E

HOLE No: BH-12
Komati Power Station









16.21

NOTES

- 1) Sh = Shelby sample.
- 2) Ref = Refusal.
- 3) Disturbed SHELBY sample taken at 1.50--1.84 m.
- 4) Disturbed SPT sample taken at 1.84--2.34 m.
- 5) Disturbed SPT sample taken at 3.00--3.50 m.
- 6) Disturbed SHELBY sample taken at 3.50--4.00 m.
- 7) Disturbed SPT sample refused at 4.00--4.50 m.
- 8) End of hole at 16.21 m.

CONTRACTOR : BAV Consulting Pty Ltd
MACHINE : ROCKDRILL TRD-300
DRILLED BY : Philip Mahlangu
PROFILED BY : Hudson Mabelane Pr.Sci.Nat

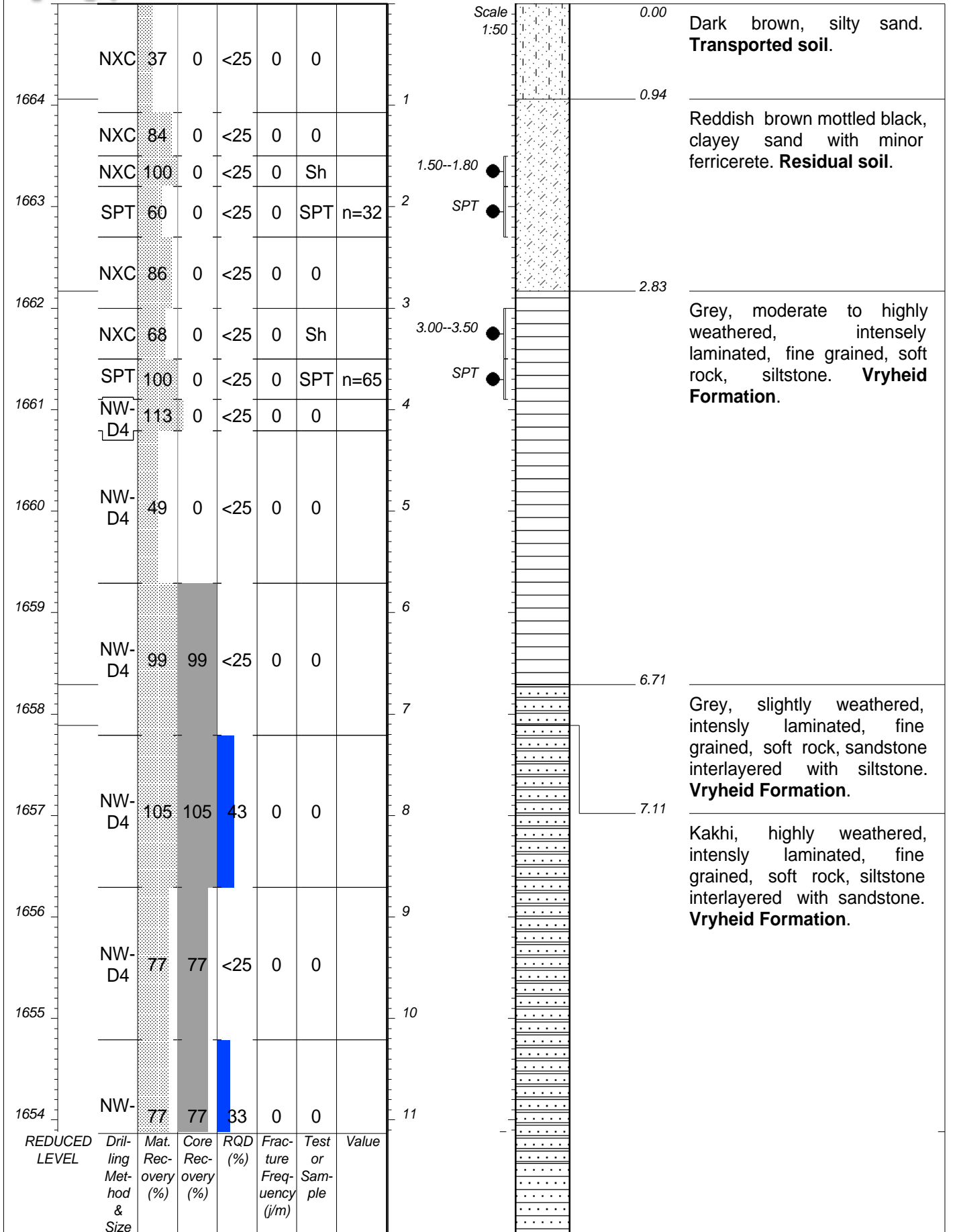
TYPE SET BY : Fumani Vukeya Cand.Sci.Nat
SETUP FILE : STANDARD.SET

INCLINATION : Vertical
DIAM : NWD-4 & NXC
DATE : 10/11/2023
DATE : 18/11/2023

DATE : 06/12/2023 14:29
TEXT : ..holes\Boreholes\BH14.TXT

ELEVATION : 1659m
X-COORD : 26.10803°S
Y-COORD : 29.46337°E

HOLE No: BH-14
Komati Power Station

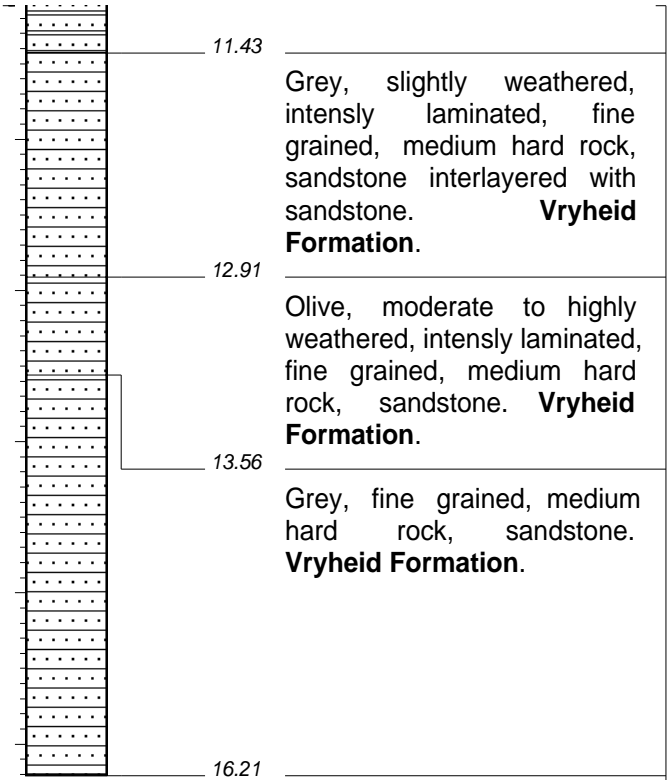
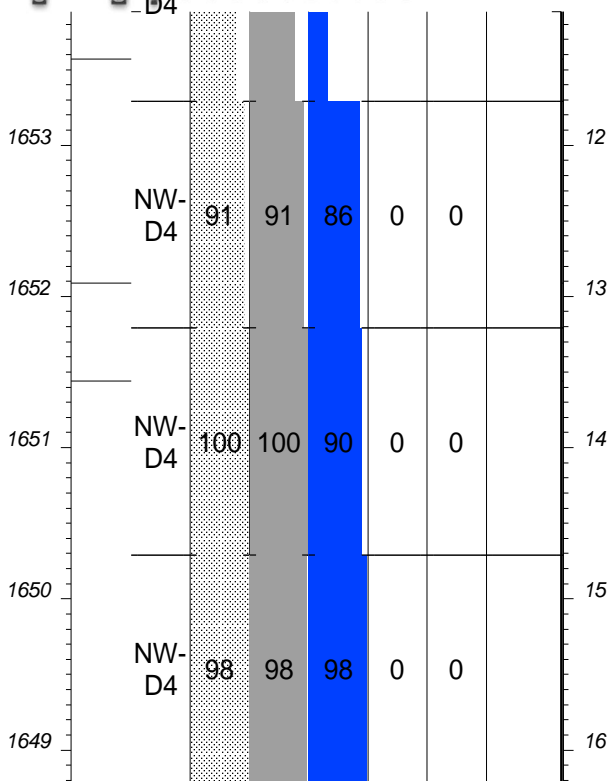




**ESKOM
PRELIMINARY GEOTECHNICAL INVESTIGATION:
KOMATI POWER STATION**

HOLE No: BH-15
Sheet 2 of 2

JOB NUMBER: MAK1520823



Grey, slightly weathered, intensely laminated, fine grained, medium hard rock, sandstone interlayered with sandstone. **Vryheid Formation.**

Olive, moderate to highly weathered, intensely laminated, fine grained, medium hard rock, sandstone. **Vryheid Formation.**

Grey, fine grained, medium hard rock, sandstone. **Vryheid Formation.**

NOTES

- 1) No water strike encountered.
- 2) Sh = Shelby sample.
- 3) Ref = Refusal.
- 4) Disturbed Sh sample taken at 1.50--1.80 m
- 5) Disturbed SPT sample taken at 1.80--2.30 m.
- 6) Disturbed Sh sample taken at 3.00--3.50 m
- 7) Disturbed SPT sample taken at 3.50--3.90 m.
- 8) End of hole at 16.21 m.

CONTRACTOR : BAV Consulting Pty Ltd
MACHINE : ROCKDRILL TRD-300
DRILLED BY : Philip Mahlangu
PROFILED BY : Andries Vukeya Cand.Sci.Nat
TYPE SET BY : Andries Vukeya Cand.Sci.Nat
SETUP FILE : STANDARD.SET

INCLINATION : Vertical
DIAM : NWD-4 & NXC
DATE : 13/11/2023
DATE : 03/12/2023

ELEVATION : 1665m
X-COORD : 26.10505°S
Y-COORD : 29.46251°E

HOLE No: BH-15
Komati Power Station